

ATTACHMENT AA

Vickery Groundwater Investigation Program

GROUNDWATER FIELD INVESTIGATION

**A Groundwater Field Investigation Program
In Support Of the Vickery Coal Project
Environmental Assessment**

FOR

WHITEHAVEN COAL LIMITED

By

Groundwater Exploration Services Pty Ltd

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1 INTRODUCTION

1.1 BACKGROUND

This document describes the hydrogeological investigation program conducted by Groundwater Exploration Services Pty Ltd in late 2011 and 2012 at the Vickery Mine Site. The program was commissioned by Whitehaven Coal Limited (Whitehaven) to gather hydrogeological and geological information, and to install groundwater monitoring bores.

The Vickery Mine Site is owned by Whitehaven and is located approximately 15 kilometres (km) south-east of Boggabri and 25 km north of Gunnedah in New South Wales (NSW) (**Figure 1.1**). Whitehaven also owns and/or manages the Tarrawonga and Rocglen Coal Mines which are currently operating and are located approximately 10 km to the north and 5 km east of the Vickery Mine Site, respectively (Figure 1). Whitehaven also continues to maintain the Canyon Mine Site (operations ceased in 2009) located to the immediate north of the Vickery Mine Site.

The Vickery Mine Site was most recently operated in the early 1990s and now consists of rehabilitated coal mining and overburden placement areas, which are currently under care and maintenance. Whitehaven is conducting exploration drilling, feasibility studies, and undertaking an environmental impact assessment for the recommencement of open cut mining operations at Vickery. The proposal is known as the Vickery Coal Project (the Project). The Project would involve the development of a 4.5 million tonne per annum (Mtpa) open cut coal mining operation with a mine life of approximately 30 years.

1.2 OVERVIEW OF THE GEOLOGY AND HYDROGEOLOGY OF THE PROJECT AREA

1.2.1 Geology

The Vickery Mine Site is located in the Gunnedah Basin, in the NSW Gunnedah Coalfield, which contains sedimentary rocks, including coal measures, of Permian and Triassic age. Regionally, there are two coal-bearing sequences in the Gunnedah Basin, namely:

- Early Permian Bellata Group (comprising the Maules Creek sub-basin and Mullaley sub-basin, separated by the Boggabri Ridge); and
- Late Permian Black Jack Group.

The Project coal resource is located within the Maules Creek sub-basin of the Early Permian Bellata Group. The target coal seams within the Maules Creek sub-basin are contained within the Maules Creek Formation. They dip towards the east and the south.

The targeted coal seams in the proposed Project open cut are divided into upper and lower groups. The upper group of seams includes:

- Gundawarra Seam;
- Kurrumbede and Welkeree Seams;
- Shannon Harbour Upper Seam;
- Shannon Harbour Lower Seam; and
- Stratford Seam.

The lower group of seams includes:

- Bluevale Upper and Lower Seams;
- Cranleigh Upper Seam; and
- Cranleigh Middle and Lower Seams.

Below the Maules Creek Formation are the Goonbri and Leard Formations, which are basal units of the Gunnedah Basin sedimentary sequence and unconformably overlie the Boggabri Volcanics.

The upper and mid slopes of the Project area generally comprise moderate relief, rounded ridges and hills which are composed of Permian-aged strata from the Maules Creek Formation. The broad valley and outflow plain areas in the lower slopes of the Project area and surrounding floodplains comprise predominantly Quaternary-aged undifferentiated colluvial and alluvial sediments. Minor undifferentiated volcanic and igneous rocks of younger age occur as isolated outcrops in the surrounding area.

The Quaternary-aged alluvial sediments comprise the (upper) Narrabri Formation and the (lower) Gunnedah Formation of the upper Namoi Valley (referred to herein as the Upper Namoi Alluvium).

1.2.2 Hydrogeology

Previous groundwater investigations and monitoring data indicate that the Project area supports two groundwater systems:

- **Porous Rock Groundwater System** - including the coal measures of the Maules Creek Formation; and
- **Alluvial Groundwater System** – associated with the low-lying flood plains and Upper Namoi Alluvium.

Recharge to the groundwater systems occurs from rainfall and runoff infiltration, lateral groundwater flow and some leakage from surface water sources (e.g. Namoi River). Groundwater levels are sustained by rainfall infiltration, however they are controlled by topography, geology and surface water levels in local drainages. Local groundwater tends to mound beneath hills, with ultimate discharge to distant drainages (via subsurface through-flow) and loss by evapotranspiration through rock outcrops and vegetation where the water table is near the ground surface (generally 2 metres [m] to 3 m below ground level). However, given the typical depth to water (10 to 14 m to the south and west of the Project area), evapotranspiration is considered to be an unlikely occurrence in the vicinity of the Project area and adjacent alluvium (Heritage Computing, 2012).

1.3 SCOPE OF WORK

The scope of work involved design and supervision of a hydrogeological investigation program which included the following components:

- Installation of three vibrating wire piezometers and five standpipes within the Maules Creek Formation within the proposed Project open cut extent.
- Drilling and geological logging of thirty four shallow investigation drillholes within the Upper Namoi Alluvium and weathered Maules Creek Formation strata within, and to the south of, the proposed Project open cut.
- Conversion of four of the above shallow investigation holes to standpipe bores.
- A pumping test at one of the drillholes to the south of the proposed open cut.

- Drilling and geological logging of a shallow investigation drillhole within the Upper Namoi Alluvium to the west of the Western Emplacement.
- Monitoring of groundwater levels from installed bores.
- Hydraulic testing and monitoring of the installed monitoring bores.
- Hydraulic testing of selected drill hole core from the Permian-aged Maules Creek Formation.

2 HYDROGEOLOGICAL INVESTIGATION PROGRAMME

2.1 MAULES CREEK FORMATION PIEZOMETERS

2.1.1 Maules Creek Formation Standpipes

Five new standpipes were installed in existing exploration drillholes in the Maules Creek Formation in December 2011. The locations of the standpipe piezometers are shown in **Figure 2.1**. All five were located within the proposed Project open cut. Each bore was backfilled to target depth and bentonite seals were placed below and above the targeted and screened test intervals. **Table 2.1** provides a summary of the installation details for each standpipe. Further details are provided in **Appendix A**, including the geological logs for the exploration holes and NSW Office of Water (NOW) test bore licences.

Table 2.1
Summary of the Maules Creek Formation Standpipes

Bore	Coordinates		Date Completed	Screened Interval	Aquifer Screened	NOW Licence Number
	Easting	Northing		(m bgl)		
VKY3034	232519	6593822	20/12/2011	118 - 136 (126 - 132)*	Shannon Harbour Seam	90BL256014
VKY3035	232703	6593356	20/12/2011	69 - 79 (72 - 75)*	Kurrumbede - Shannon Harbour Interburden	90BL256013
VKY3036	233120	6592921	21/12/2011	107 - 117 (114 - 111)*	Shannon Harbour - Stratford Interburden	90BL256015
VKY3042	232543	6592598	21/12/2011	134 - 158 (137 - 140/152 -155)*	Stratford Seam	90BL256016
VKY3043	233250	6590533	22/12/2011	225 - 245 (246 - 237)*	Bluevale - Cranleigh Interburden	90BL256011

* Screened interval: 118 - 136 (126 - 132) [Gravel Pack interval (Screen Interval)].
m bgl = metres below ground level.

2.1.2 Maules Creek Formation Vibrating Wire Piezometers

Three multi-level vibrating wire piezometers were installed in the Maules Creek Formation in February 2012. The vibrating wire piezometers were installed in existing exploration holes located within the proposed Project open cut. Each piezometer was installed with transducers targeting coal seams and interburden units to monitor groundwater pressures within the Maules Creek Formation coal measures. The piezometers were located in the Shannon Harbour, Stratford, Kurrumbede, Bluevale and Cranleigh coal seams and also within selected interburden units. **Table 2.2** provides a summary of the installation details for each of the multi-level vibrating wire piezometers. Further details are provided in **Appendix B**, including the geological logs for the original exploration holes and NOW test bore licences. The locations of the three new vibrating wire piezometers are shown in **Figure 2.1**.

Table 2.2
Summary of the Maules Creek Formation Vibrating Wire Piezometers

Bore	Coordinates		Date Completed	Screened Interval	Aquifer Screened	NOW Licence Number
	Easting	Northing		(m bgl)		
VKY3033	232366	6594263	19/02/2012	38.0	Kurrumbede Overburden	90BL256012
				51.0	Kurrumbede Seam	
				70.0	Kurrumbede - Shannon Harbour Interburden	
				90.0	Shannon Harbour Interburden	
				115.0	Stratford Upper Seam	
				140.0	Stratford - Bluevale Interburden	
				170.0	Bluevale - Cranleigh Interburden	
				190.0	Cranleigh Mid Seam	
VKY3041	233397	6591348	18/02/2012	38.0	TRA - GDW Interburden	90BL256010
				51.0	TRA - GDW Interburden	
				70.0	GDW-Kurrumbede Interburden	
				95.0	Kurrumbede - Shannon Harbour Interburden	
				115.0	Kurrumbede - Shannon Harbour Interburden	
				140.0	Shannon Harbour Interburden	
				165.0	Shannon Harbour Interburden	
				190.0	Stratford - Bluevale Interburden.	
VKY3053	230098	6593816	12/02/2012	30.0	Kurrumbede - Shannon Harbour Interburden	90BL256009
				50.0	Stratford - Bluevale Interburden	
				68.0	Bluevale Mid Seam	
				75.0	Bluevale - Cranleigh Interburden	
				89.0	Volcanic Basement	

2.2 UPPER NAMOI ALLUVIUM INVESTIGATION HOLES

Thirty three shallow exploration holes were drilled along four north-south trending transects and one east-west trending transect during the period January to March 2012.

An additional hole was drilled on the southern end of Transect 2 in August 2012 and was used to conduct a pumping test (Section 2.7).

The holes were drilled into the Maules Creek Formation bedrock where possible. The depths of the drilled holes ranged from 13 to 42 m.

Temporary casing was installed in the holes and they were geophysically logged where possible prior to being cemented and rehabilitated in April 2012. Due to the method of drilling (air rotary/hammer), hole instabilities and caving meant that holes rapidly deteriorated and not all were geophysically logged to full depth.

The purpose of these investigation holes was to more accurately delineate the boundary between the weathered Permian-aged Maules Creek Formation (on the upper slopes) and the Quaternary-aged Upper Namoi Alluvium (further down the slope and onto the floodplain). The interpretation of the boundary was primarily based on the geological logs, geophysical logs, and recorded groundwater levels. Consideration was also given to the topography and location of the holes relative to the existing floodplain and adjoining slopes.

The location of the investigation holes and the interpreted boundary between the Maules Creek Formation and Upper Namoi Alluvium in the vicinity of the proposed Project open cut is shown on **Figure 2.2**.

Further detail of the Upper Namoi Alluvium boundary interpretation for each of the transects is provided below and in **Appendix C**, including the geological and geophysical logs.

2.2.1 Transect 1

Transect 1 is the most westerly transect line and closest to Blue Vale Road (**Figure 2.2**). Just to the north of Transect 1 at the corner of Shannon Harbour Road and Blue Vale Road, weathered Maules Creek Formation conglomerate outcrops and is clearly visible on the ground and in aerial photographs. This is the only area of in-situ rock outcrop in the vicinity of the five transects.

The topography along Transect 1 slopes gradually and uniformly from north to south between TR23 and TR5. The southern end of Transect 1 (i.e. between TR5 and TR3) is almost flat and is located on the floodplain.

Figure 2.3 is the interpreted cross section along Transect 1 based on the seven investigation holes.

Drilling in the northern part of Transect 1 (i.e. TR23 and TR24) revealed shallow (i.e. less than 3 m deep) unsaturated conglomerate of the Maules Creek Formation. The conglomerate transitioned from weathered to fresh rock within the upper 5 m at holes TR23, TR24 and TR25. South of TR25, the depth to the conglomerate basement rock progressively increased (i.e. up to approximately 20 m in hole TR3), which indicates that beneath the floodplain is a deepening channel structure which has been filled by alluvial materials of the Upper Namoi Alluvium.

The groundwater levels along Transect 1 were consistently measured at approximately 246 m Australian Height Datum (AHD).

A general trend of increased angularity in gravel particles and overall reduction in the amount of clay was observed north of TR5 (i.e. on the lower slopes). The gravels north of TR5 on the sloping topography were interpreted to be more colluvial in nature, when compared with the alluvial gravels on the floodplain south of TR5.

In the area between holes TR3 and TR5 the first 8 to 20 m of material beneath the surface consists of interbedded alluvium and colluvium. The interpreted boundary of the Upper Namoi Alluvium has been conservatively assumed to be at TR5, which represents the up-slope and unsaturated edge of this zone of interbedded alluvium and colluvium (refer to **Figure 2.2** and **Figure 2.3**).

Figure 2.4 shows chip tray photographs and composite graphical logs which include density trace of selected investigation holes along Transect 1. Highlighted is the apparent relative density showing a transition zone which was typical of the profile seen in most holes. The transition was assessed to be due to in-situ weathered conglomerates and sandstone typical of the local area.

2.2.2 Transect 2

Transect 2 shows very similar features to Transect 1, although the distinction between the floodplain and lower slope is less clear cut and it occurs slightly further north (i.e. approximately TR12). As per Transect 1, the basement conglomerate of the Maules Creek Formation is located relatively close to the surface and generally follows the topography between holes TR20 and TR13.

The recorded groundwater levels along Transect 2 were the same as Transect 1 (i.e. approximately 246 m AHD).

The zone of interbedded alluvium and colluvium occurs between drill holes TR7 and TR12.

Figure 2.5 is the interpreted cross section along Transect 2 based on the six investigation holes.

Figure 2.6 shows chip tray photographs and composite graphical logs which include density trace of selected investigation holes along Transect 2. The transition zone showing a relative density change is again highlighted.

2.2.3 Transect 3

Transect 3 is located near the eastern edge of the proposed Project open cut (**Figure 2.1**).

Figure 2.7 is the interpreted cross section along Transect 3 based on the nine investigation holes.

The topography along Transect 3 slopes gradually and uniformly from north to south (i.e. between TR22 and TR8). Unlike Transect 1 (and to a lesser extent Transect 2) there is no obvious flat floodplain.

The characteristics of the strata intersected by the drill holes on Transect 3 were very similar to Transects 1 and 2. The basement conglomerate of the Maules Creek Formation generally follows the topography, and occurs at a depth of 5 to 10 m below the surface between holes TR22 and TR11. Further to the south, the depth to the basement strata increases notably to approximately 20 m at TR8.

The zone of interbedded alluvium and colluvium occurs between drill holes TR8 and TR16.

Figure 2.8 shows chip tray photographs and composite graphical logs which include density trace of selected investigation holes along Transect 3.

2.2.4 Transect 4

Transect 4 is located west of the proposed Project open cut and to the south of the proposed Eastern Emplacement area (**Figure 2.1**).

Figure 2.9 is the interpreted cross section along Transect 3 based on the four investigation holes.

The topography along Transect 4 slopes gradually from north to south between TR31 and TR27. To the south of TR31, the topography is almost flat and is located on the floodplain.

Drilling in the northern part of Transect 4 (i.e. TR29 and TR27) showed relatively shallow (i.e. less than 5 m deep) unsaturated conglomerate and sandstone of the Maules Creek Formation. The conglomerate transitioned from weathered to fresh rock within the upper 8 to 10 m in holes TR27, TR28 and TR29. South of TR29, the depth to the conglomerate basement rock progressively increased (i.e. up to approximately 20 m in hole TR30).

The zone of interbedded alluvium and colluvium occurs between drill holes TR30 and TR29.

Figure 2.10 shows chip tray photographs and composite graphical logs which include density trace of selected investigation holes along Transect 4. It shows that there are less unconsolidated present with Maules Creek Formation sandstone and conglomerate evident at shallow depths. The deepening channel structure associated with the floodplain appears in the southern margin of the transect.

2.2.5 Transect 5

Transect 5 runs perpendicular to transects 1 to 4 (i.e. east to west orientation) and is located to the southwest of the proposed Project open cut (**Figure 2.2**).

Figure 2.11 is the interpreted cross section along Transect 5 based on the five investigation holes. Drilling along this transect revealed shallow colluvial soils and weathered sandstone of the Maules Creek Formation up to a depth of 10 m from the surface. No alluvial material associated with the Upper Namoi Alluvium was intersected. Water level monitored in TR35 indicates that standing water level (approximately 247 m AHD) is well below the base of the soil and weathered Maules Creek Formation sandstone.

Figure 2.12 shows chip tray photographs and composite graphical logs which include density trace of selected investigation holes along Transect 5. It shows that the transition in density of the shallow stratigraphic profile is not as clear as seen in other transect lines and that the top of weathered Maules Creek Formation strata is undulating. The density of investigation bores is not great enough to refine the undulating surface but does indicate that there are likely drainage/erosion palaeochannels in this area.

2.3 UPPER NAMOI ALLUVIUM/WEATHERED PERMIAN STRATA PIEZOMETERS

Standpipe piezometers were installed in four of the new investigation holes that were drilled in the vicinity of the southern edge of the proposed Project open cut near the boundary of the Upper Namoi Alluvium and the weathered Permian-aged Maules Creek Formation strata.

The piezometers were cased with 50 millimetre (mm) PVC including a 3 m section of factory-slotted screen. The screens were designed to be placed in the lowermost section of the Upper Namoi Alluvium or weathered Permian-aged Maules Creek Formation strata.

The location of the standpipe piezometers are shown on **Figure 2.1**. **Table 2.3** provides a summary of the installation details for the standpipe piezometers. Further details of the piezometer installation are included in **Appendix C**, including geological logs, geophysical logs and NOW test bore licences.

Table 2.3
Upper Namoi Alluvium Weathered Permian Strata Piezometers

Bore	Coordinates		Date Completed	Screened Interval	Aquifer Screened	NOW Licence Number
	Easting	Northing		(m bgl)		
TR7	232931	6589763	16/02/2012	10 - 18 (15 - 18)	Weathered Conglomerate	90BL256017
TR18	233435	6590015	12/01/2012	12 - 22 (19 - 22)	Weathered Conglomerate	90BL256018
TR26	232438	6590095	13/02/2012	10 - 18 (15 - 18)	Weathered Conglomerate	90BL256019
TR35	233848	6590279	13/02/2012	19 - 24 (21 - 24)	Weathered Permian Sandstone	90BL256020

Screened interval : 10 - 18 (15 - 18) [Gravel Pack interval (Screen Interval)].

2.4 GROUNDWATER MONITORING

2.4.1 Groundwater levels

Groundwater levels are presently recorded daily in all vibrating wire and standpipe bores installed within Maules Creek Formation strata. Within the Upper Namoi Alluvium investigation area, two of the four installed standpipes have downhole loggers monitoring at daily intervals. A summary of the current groundwater monitoring piezometers and groundwater levels recorded in May 2012 are presented in **Table 2.4**.

Hydrographs and hydrostatic head profiles for multi seam vibrating wire piezometers and hydrographs for standpipes piezometers are provide in **Figures 2.13 to 2.15**.

The hydrostatic head profile is used to gauge the quality of data sets and to explore the head gradients which may be apparent. Generally, under pre-mining conditions, pressures plot close to the 45° "hydrostatic line", while a slight shift away from the line reveals that there is a head gradient throughout the profile. This may not be the case in areas already affected by mining stresses, in elevated terrain where perched systems prevail or where natural artesian conditions exist.

Figure 2.13 shows hydrographs and hydrostatic head profiles for VKY3033 which has eight vibrating wire transducers installed. The hydrostatic profile shows a reasonable linear trend. The increased levels in the upper most piezometer indicate some perching in the weathered strata. Hydrographs for various intervals are stable except for the transducer at 170 m depth which shows some variation since installation.

Figure 2.14 shows hydrographs and hydrostatic head profiles for VKY3041 which also has eight vibrating wire transducers installed. Hydrostatic profile is reasonable uniform however groundwater pressures are slightly lower than the 45 degree hydrostatic line. Hydrographs indicate relatively stable groundwater levels although the temporal groundwater pressures within the lower piezometers show some irregularity. No explanation is evident for the variations seen.

**Table 2.4
Current Groundwater Monitoring Piezometers**

Bore	Coordinates		Ground Level (m AHD)	Drilled Depth (m bgl)	Screened Interval (m bgl)	Aquifer Screened	Current Status	Water Level May 2012	
	Eastings	Northing						m bgl	m AHD
VKY3034	232519	6593822	287.36	200	136 - 118 (126 - 132)	Shannon Harbour Seam	Standpipe Piezometer	42.51	244.85
VKY3035	232703	6593356	290.81	214	69 - 79 (72 - 75)	Kurrumbede - Shannon Harbour Interburden	Standpipe Piezometer	44.29	246.52
VKY3036	233120	6592921	297.89	225	117 – 107 (114 - 111)	Shannon Harbour - Stratford Interburden	Standpipe Piezometer	48.17	249.72
VKY3042	232543	6592598	292.02	219.2	134 - 158 (137 -140/152 -155)	Stratford Seam	Standpipe Piezometer	42.67	249.35
VKY3043	233250	6590533	263.6	245	245 - 225 (246 - 237)	Bluevale - Cranleigh Interburden	Standpipe Piezometer	16.53	247.07
VKY3033	232366	6594263	285.57	258	38.0	Kurrumbede Overburden	Vibrating Wire Piezometer	29.46	256.11
					51.0	Kurrumbede Seam	Vibrating Wire Piezometer	36.37	249.20
					70.0	Kurrumbede - Stratford Interburden	Vibrating Wire Piezometer	39.98	245.59
					90.0	Stratford Interburden	Vibrating Wire Piezometer	43.78	241.79
					115.0	Stratford Upper Seam	Vibrating Wire Piezometer	45.42	240.15
					140.0	Stratford - Bluevale Interburden	Vibrating Wire Piezometer	47.72	237.85
					170.0	Bluevale - Cranleigh Interburden	Vibrating Wire Piezometer	48.05	237.52
					190.0	Cranleigh Mid Seam	Vibrating Wire Piezometer	49.67	235.90

Table 2.4
Current Groundwater Monitoring Piezometers (continued)

Bore	Coordinates		Ground Level (m AHD)	Drilled Depth (m bgl)	Screened Interval (m bgl)	Aquifer Screened	Current Status	Water Level May 2012	
	Eastings	Northing						m bgl	m AHD
VKY3041	233397	6591348	291.91	338	38.0	TRA - GDW Interburden	Vibrating Wire Piezometer	36.02	255.89
					51.0	TRA - GDW Interburden	Vibrating Wire Piezometer	34.57	257.34
					70.0	GDW-Kurrumbede Interburden	Vibrating Wire Piezometer	38.14	253.77
					95.0	Kurrumbede - Shannon Harbour Interburden	Vibrating Wire Piezometer	42.48	249.43
					115.0	Kurrumbede - Shannon Harbour Interburden	Vibrating Wire Piezometer	43.56	248.35
					140.0	Shannon Harbour Interburden	Vibrating Wire Piezometer	47.32	244.59
					165.0	Shannon Harbour Interburden	Vibrating Wire Piezometer	41.29	250.62
					190.0	Stratford - Bluevale Interburden	Vibrating Wire Piezometer	39.71	252.2
VKY3053	230098	6593816	264.57	90	30.0	Kurrumbede - Shannon Harbour Interburden	Vibrating Wire Piezometer	15.15	249.42
					50.0	Stratford - Bluevale Interburden	Vibrating Wire Piezometer	18.60	245.97
					68.0	Bluevale Mid Seam	Vibrating Wire Piezometer	18.57	2456.00
					75.0	Bluevale - Cranleigh Interburden	Vibrating Wire Piezometer	19.07	245.50
					89.0	Volcanic Basement	Vibrating Wire Piezometer	41.18	223.39
TR7	232931	6589763	255.17	19	15 - 18	Weathered Conglomerate	Standpipe Piezometer	9.72	245.45
TR18	233435	6590015	239.89	23	19 - 22	Weathered Conglomerate	Standpipe Piezometer	12.98	245.91
TR26	232438	6590095	258.18	19	15 - 18	Weathered Conglomerate	Standpipe Piezometer	12.41	245.77
TR35	233848	6590279	264.59	25	21 - 24	Weathered Permian Sandstone	Standpipe Piezometer	18.25	246.34

Figure 2.15 shows hydrographs and hydrostatic head profiles for VKY3033 which has five vibrating wire transducers installed. The upper 4 transducers are installed within the Maules Creek Formation coal measures while the bottom transducer is installed within the basement volcanics. The hydrostatic profile shows coal measure stratigraphy correlate well with a slight perching in the weathered profile. However, the pressure indicated in the basement is significantly lower than that within the overlying coal measures. This appears unusual and may indicate equipment failure.

Figure 2.16 shows hydrographs for all recently installed standpipe piezometers and includes TR7 and TR18 which are screened within weathered Maules Creek Formation conglomerate. Pressures are stable except for VKY3035. The rising water level is due to recovery of the bore following purging undertaken for groundwater quality sampling.

Groundwater levels in the southern end of the investigation area which focused on delineating the relationship between Maules Creek Formation conglomerate/alluvium/colluvium are approximately 10 m below ground level in the northern most extend of the floodplain. **Figures 2.17** show saturated thickness of the interpreted alluvium and highly weathered Maules Creek Formation conglomerate/sandstones. That is the difference between the interpreted base of alluvium and standing groundwater level contours. **Figures 2.18** show saturated thickness of the interpreted highly weathered Maules Creek Formation conglomerate/sandstones. This figure indicates that there is an embayment of alluvium and weathered basement strata to the north in the vicinity of Transect 3.

2.4.2 Groundwater Quality

Water quality data obtained from newly installed piezometers is summarised in **Table 2.5** and **Table 2.6**. Where groundwater samples were taken, each piezometer was purged in accordance with AS/NZS 5667 (Standards Australia, 1998) and water samples were collected for field analysis of pH and electrical conductivity (EC), and for laboratory testing of a comprehensive suite of analytes which includes:

- pH, electrical conductivity (EC) and total dissolved solids (TDS);
- major cations and anions; and
- dissolved metals (As, B, Cd, Cr, Cu, Fe, Ni, Pb, Mn, Se, Zn, Hg).

The laboratory analysis was undertaken by ALS Environmental, a NATA-accredited laboratory based in Sydney.

Assessments of groundwater and surface quality can be useful in understanding conceptual hydrogeology, particularly in relation to EC and Piper diagram plots. Different strata horizons can demonstrate differing amounts of salinity, which tend to be low in areas of high recharge or connectivity with surface waters. Piper plots provide an assessment of the recharge-discharge processes, and also allow a comparison of water samples derived from different environments within the hydrological cycle. Recently-recharged water tends to plot closer to the left-hand apex of the diamond field in the Piper diagram, and waters further from the source of recharge closer to the right-hand side.

Figure 2.19 illustrates a piper diagram Piper Diagram based on the March 2012 results which shows that groundwater is generally of a Sodium Chloride type. There is a bicarbonate component seen in the Permian strata but this is noticeably absent from highly saline groundwater found within the shallow weathered Permian conglomerates at the southern end of CL316 suggesting that these water samples are generally not accepting recharge.

Table 2.5
Groundwater Quality – Major Ions

Bore (Registered Bore/Licence Number)	Lithology	Date	pH	EC (μ S/cm)	TDS (mg/L)	Ca (mg/L)	Mg (mg/L)	Na (mg/L)	K (mg/L)	Cl (mg/L)	HCO ₃ (mg/L)	SO ₄ (mg/L)
VKY3034 (90BL256014)	MCF/Coal	16 Mar 2012	8.1	4040	2430	80	57	855	14	756	807	268
		Aug 2012	7.9	3770	2703	79	58	800	12	668	826	260
VKY3035 (90BL256013)	MCF	16 Mar 2012	7.6	2980	1790	64	44	633	11	606	644	88
		Aug 2012	7.8	3150	2203	55	43	645	10	596	742	112
VKY3036 (90BL256015)	MCF	16 Mar 2012	7.5	5080	2970	109	95	963	13	1180	706	235
		Aug 2012	7.8	5350	3446	114	104	1020	13	1160	767	268
VKY3042 (90BL256016)	MCF/Coal	16 Mar 2012	7.8	4810	2810	126	125	829	17	1150	714	156
		Aug 2012	7.7	5290	3536	171	210	803	22	1260	800	270
VKY3043 (90BL256011)	MCF	16 Mar 2012	8.1	2540	1550	12	4	663	7	331	909	8
		Aug 2012	8.3	3030	2391	6	4	817	6	396	1160	2
TR7 (90BL256017)	MCF/Regolith	16 Mar 2012	7.3	15900	12500	305	411	2870	20	5250	703	512
		Aug 2012	7.2	14700	9231	261	356	2600	18	4770	739	487
TR18 (90BL256018)	MCF/Regolith	16 Mar 2012	7.3	13500	8690	262	370	2490	16	4330	649	656
		Aug 2012	7.3	13600	8293	232	359	2440	15	4380	722	145
TR26 (90BL256019)	MCF/Regolith	16 Mar 2012	7.4	4640	2720	105	104	829	10	1040	844	117
		Aug 2012	7.5	4950	3297	108	115	926	10	1070	923	145
TR35 (90BL256020)	MCF/Regolith	16 Mar 2012	7.4	13200	9300	323	400	2190	21	4340	710	564
		Aug 2012	7.3	15400	9940	322	468	2630	20	5020	742	738

MCF = Maules Creek Formation.

**Table 2.6
Groundwater Quality – Metals**

Registered Bore	Date	Alumin.	Arsenic	Cad.	Chrom.	Copper	Lead	Mang.	Nickel	Selen.	Silver	Zinc	Boron	Iron	Merc.	Nitrite as N	Nitrate as N	Nitrite + Nitrate as N
		mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L	mg/L
VKY3034	16/03/2012	<0.01	0.005	<0.0001	<0.005	0.004	<0.001	0.283	0.002	<0.01	<0.001	0.008	0.07	<0.05	<0.0001	0.02	0.05	0.07
VKY 3035	16/03/2012	<0.01	0.007	<0.0001	<0.005	0.005	0.006	0.744	0.004	<0.01	<0.001	0.046	<0.05	<0.05	<0.0001	<0.01	0.03	0.03
VKY 3036	16/03/2012	<0.01	0.002	<0.0001	<0.005	0.004	<0.001	0.058	0.004	<0.01	<0.001	0.023	0.06	<0.05	<0.0001	0.02	0.09	0.11
VKY 3042	16/03/2012	<0.01	0.001	<0.0001	<0.005	0.004	0.004	0.786	0.003	<0.01	<0.001	0.028	0.07	0.3	<0.0001	0.03	0.07	0.1
VKY 3043	16/03/2012	0.05	0.01	<0.0001	<0.005	0.002	<0.001	<0.001	0.001	<0.01	<0.001	<0.005	0.08	<0.05	<0.0001	<0.01	<0.01	<0.01
TR7	16/03/2012	<0.01	<0.001	<0.0001	<0.005	0.006	<0.001	0.724	0.004	<0.01	<0.001	0.006	0.06	<0.05	<0.0001	<0.01	0.09	0.09
TR18	16/03/2012	<0.01	<0.001	<0.0001	<0.005	0.006	<0.001	0.498	0.004	<0.01	<0.001	0.006	0.07	<0.05	0.0002	<0.01	0.09	0.09
TR26	16/03/2012	<0.01	<0.001	<0.0001	<0.005	0.003	<0.001	0.022	0.002	<0.01	<0.001	<0.005	0.07	<0.05	<0.0001	0.03	1.08	1.11
TR35	16/03/2012	<0.01	<0.001	<0.0001	<0.005	0.008	<0.001	0.484	0.005	<0.01	<0.001	0.016	0.06	<0.05	<0.0001	<0.01	<0.01	<0.01

In summary, the sampling round conducted in March 2012 resulted in the following findings:

- The water quality in the Permian-aged coal measures of the Maules Creek Formation range from 2,540 to 5,080 $\mu\text{S}/\text{cm}$.
- The water quality in weathered Permian conglomerates located in the southern margins of CL316 are more saline with EC's ranging from 4,640 to 15,900 $\mu\text{S}/\text{cm}$ which suggests that there is little interconnection with groundwater from the Namoi River floodplain.
- Most samples are slightly alkaline, with pH values ranging from 7.2 to 8.
- No dissolved metals exceedances were observed.
- The low dissolved iron suggests the likely absence of pyrite in the coal and/or interburden strata, and in conjunction with the mostly alkaline pH, suggests that the mine waters are unlikely to have acid generating potential.

2.5 HYDRAULIC TESTING

Falling head slug tests were carried out during March 2012 on five (VKY3034, VKY3035, VKY3036, VKY3042 and VKY3043) of the standpipe piezometers installed within the Maules Creek Formation to obtain estimates hydraulic conductivity for selected stratigraphic horizons. The procedure involved adding a slug of water to each piezometer/bore and then recording water-level recovery back to a static level using a downhole pressure transducer.

Low flow constant rate and slug tests were conducted within the four (TR7, TR18, TR26 and TR35) standpipes screened within weathered Permian conglomerate underlying the alluvium and colluvium found within the Upper Namoi Alluvium transect drilling program on the southern margins of the proposed Project open cut.

The slug test data were analysed using the Bouwer-Rice method (Bouwer and Rice, 1976) for the tests on unconsolidated sediments (alluvium and colluvium), and the Hvorslev Method (Hvorslev, 1951) for tests on the hard rock units confined, which are suitable for providing 'near well' estimates of aquifer hydraulic conductivity (K).

The limitation in this method is that disturbance to the formation caused during drilling can result in elevated K values in the immediate vicinity of the borehole. Hence, where there was sufficient depth of water in the bore, a Constant Rate Test (CRT) was also conducted using a low capacity sampling pump. The drawdown and pumping rate data obtained from a CRT can provide the basis for estimating a broader range of hydraulic parameters than is achievable from falling head tests.

The constant rate test data were analysed using the Cooper-Jacob method (Cooper and Jacob, 1946) to determine a value of transmissivity, from which a value of average hydraulic conductivity (permeability) can be calculated. A summary of the derived hydraulic conductivity values is presented in **Table 2.7**. Data plots and solutions derived for each CRT test are included for reference in **Appendix D**.

Table 2.7
Summary of Results from Hydraulic Testing Program

Bore	Aquifer Screened	Screened Interval*	Test Date	Type of Test	Duration (min)	Pumping rate	Transmissivity, T	Hydraulic Conductivity, K	Comments
		(m bgl)				(m ³ /day)	(m ² /day)	(m/day)	
VKY3034	SH seam	136 - 118 (126 - 132)	15/01/2012	Slug	-	-	2.90×10^{-3}	2.07×10^{-4}	
VKY3035	Ku-SH Interburden	69 - 79 (72 - 75)	15/01/2012	Slug	-	-	3.35×10^{-3}	3.35×10^{-4}	
VKY3036	SH – Str Interburden	117 - 107 (114 - 111)	15/01/2012	Slug	-	-	7.27×10^{-3}	7.27×10^{-4}	
VKY3042	Stratford Seam	134 - 158 (137 - 140/152 - 155)	15/01/2012	Slug	-	-	4.34×10^{-2}	1.81×10^{-4}	
VKY3043	Bluevale – Cranleigh Seam	245 - 225 (246 - 237)	23/01/2012	Slug	-	-	-	-	Seal Failure
TR7	Weathered Permian Conglomerate	10 - 18 (15 - 18)	19/01/2012	CRT (Recovery)	60	13	6.49	0.81	
TR18	Weathered Permian Conglomerate	12 - 22 (19 - 22)	16/01/2012	CRT (Recovery)	60	13	1.29	1.6×10^{-1}	
TR26	Weathered Permian Conglomerate	10 - 18 (15 - 18)	23/03/2012	Slug	-	-	3.28	4.1×10^{-1}	
TR35	Weathered Permian Sandstone	19 - 24 (21 - 24)	25/03/2012	Slug	-	-	3.0×10^{-1}	5.0×10^{-2}	

* Screened interval - 10 - 18 (15 - 18) [Gravel Pack interval (Screen Interval)].

2.6 LABORATORY CORE PERMEABILITY TESTING

Core from four drill holes were sampled to gain representative lithologies from major interburden units. Boreholes from which core samples were taken include VKY002, VKY006, VKY010, VKY017 and VKY020. The locations of these holes are shown in **Figure 2.1**. The core samples were laboratory tested to determine vertical and horizontal hydraulic conductivity. Testing for vertical permeability was taken perpendicular to the bedding planes, while horizontal permeability was taken parallel to the bedding planes. During the testing process one horizontal and two vertical samples failed under the test regime.

A summary of the core test results is provided in **Table 2.8**. These results can be regarded as lower limits for use in model calibration, as cores do not capture the bulk fractured characteristics of a formation.

The results of core permeability testing did not show a noticeable decrease in permeability with depth for the coal seams. Despite this, decreasing permeability with depth is expected with greater cover depth and/or remoteness from outcrop and near-surface effects of weathering. **Figure 2.20** shows depth and hydraulic conductivity results for horizontal and vertical tests respectively.

Table 2.8
Core Permeability Test Results

Horizontal Hydraulic Conductivity (m/d)					
Arithmetic Mean	Harmonic Mean	Number of Samples	Max	Min	Formation
4.9×10^{-6}	1.65×10^{-6}	11	2.22×10^{-5}	4.9×10^{-7}	Tralee - Stratford Seam - Interburden
1.76×10^{-5}	9.33×10^{-7}	3	3.09×10^{-5}	3.16×10^{-7}	Maules Creek Formation - Interburden
4.04×10^{-5}	4.19×10^{-7}	13	4.35×10^{-4}	6.36×10^{-8}	Bluevale - Cranliegh Seam - Interburden
2.41×10^{-6}	4.03×10^{-7}	2	4.28×10^{-5}	5.4×10^{-7}	Boggabri Volcanics
Vertical Hydraulic Conductivity (m/d)					
Arithmetic Mean	Harmonic Mean	Number of Samples	Max	Min	Formation
2.3×10^{-6}	5.82×10^{-7}	11	1.19×10^{-5}	2.01×10^{-7}	Tralee - Stratford Seam - Interburden
1.8×10^{-5}	7.21×10^{-6}	3	3.64×10^{-5}	3.12×10^{-6}	Maules Creek Formation - Interburden
4.72×10^{-6}	4.19×10^{-7}	12	2.76×10^{-5}	1.03×10^{-7}	Bluevale - Cranliegh Seam - Interburden
4.03×10^{-6}	4.03×10^{-7}	1	4.03×10^{-6}	4.03×10^{-6}	Boggabri Volcanics

2.7 PUMPING TEST

Groundwater pumping tests were planned in two areas in the vicinity of the Project in order to investigate the potential yield of the weathered materials at the boundary between the Upper Namoi Alluvium and Maules Creek Formation. The first area was located to the south of the proposed cut where Transects 1 to 5 were drilled (**Section 2.2**), and the second area was located to the west of the proposed Western Emplacement.

2.7.1 Bore Location and Construction

Groundwater monitoring bore licences (i.e. 90BL256079 and 90BL256080) were obtained from the NOW in August 2012 prior to installation of both bores. A copy of the licences is contained in **Appendix E**.

The bores were assigned unique identifying numbers (i.e. VKY3092 and VNW385) by Whitehaven. VKY3092 was drilled on 29 August 2012, and VNW385 was drilled on 30 August 2012. Both were drilled by Mannion Drilling using rotary air techniques. **Table 2.9** summarises the bore installation details.

Table 2.9
Installation Details

Bore	Easting (MGA)*	Northing (MGA)	Drilled Depth (m)	Screen (m bgl)	SWL** (m bgl)	NOW License Number
VNW385	228420	6594936	18	12-18	>18 (Dry)	90BL256079
VKY3092	232913	6589765	17	11 – 17	9.98	90BL256080

* MGA = Map Grid Australia.

** SWL = Standing Water Level.

Figure 2.2 shows the location of VKY3092 and VNW385.

VKY3092 was drilled through the colluvium soils, Upper Namoi Alluvium sediments and a short depth into the underlying weathered Permian-aged Maules Creek Formation sedimentary strata. Intersection with the weathered Permian strata was necessary to attain adequate available drawdown for the pump test. VKY3092 was constructed adjacent to standpipe piezometer TR7, which was also screened across alluvium/colluvium and weathered *in situ* Permian conglomerate.

VKY3092 was drilled at a diameter of 165 mm which accommodated 125 mm diameter Class 12 PVC. The production zone consisted of 125 mm diameter PVC, slotted across a 12 m interval at a depth of 12 to 17 m. The screen consisted of vertical slots of 2 mm aperture. The gravel pack extended from 2 to 17 m. A bentonite plug was included at depths of 0 – 2 m.

VNW385 was drilled to a depth of 18 m but did not intersect any groundwater. As a result a pumping test was not able to be undertaken at this location.

2.7.2 Determination of Aquifer Properties at VKY3092

Methods

The pump test was initially intended to be undertaken for a period of 24 hours and was designed to assess whether or not the aquifer could yield at a nominal rate of 5 litres per second (5 L/s). At the initial pump rate of approximately 2 L/s, the bore was drained in less than 10 minutes immediately indicating a yield of 5 L/s was not available.

The pump rate was then reduced and rate varied until a near stable drawdown was attained. The test was run for a total of 12 hours although an apparent valve clogging has effectively limited the test to 8 hours with an average discharge rate of 0.2 L/s (17.3 m³/d). Variable flow in the early part of the pump test demonstrates the difficulty in attaining a static drawdown under very low yield conditions. Extracted groundwater was discharged to a HDPE lined sump which was installed for drilling purposes but not used.

The hydrogeological assessment utilises Constant Rate Test (CRT) data obtained from the VKY3092 pump test and observation data at TR7.

A suite of published solutions (Kruseman and de Ridder, 1991) have been used to analyse the CRT data from TA122. Automated curve matching was undertaken using Aqtesolv for Windows. During analysis of drawdown and recovery data, it became apparent the curve best fitted a confined or leaky confined model despite being from shallow levels. This could be explained to a degree but the intercalated nature of clays and clayey gravels within the lower part of the alluvium/colluvium sequence and the heavy clay cover in the upper 5 m of the profile. In addition, it has been noted that during excavation of shallow test pits in this area, groundwater seepage into the pits occurred from discrete horizons. It is thought that this is the case through the shallow stratigraphic profile comprised of intercalated alluvium and colluvium. Below this within the weathered Permian conglomerate is expected that groundwater flow is likely to be fracture driven rather than from the rock matrix.

The following methods were used in the analysis:

- Barker (1988) curve fitting method, which is derived for confined aquifers and used using drawdown and recovery for both the pumped bore VKY3092 and adjacent observation bore TR7.
- Hantush (1935) curve fitting method, which is derived for leaky confined aquifers.

Assumptions common to all of these methods are that discharge is constant and that the well where data collection takes place fully penetrates the aquifer.

Analysis

A summary of the hydraulic parameters derived from the CRT are summarised in **Table 2.10** and the calculations from which these are derived are presented in **Figures 2.21 to 2.23**.

Aquifer parameters of transmissivity (T) and aquifer Storage (S) is assessed from the level data within the pumping bore. Aquifer transmissivity is a function of permeability (K) multiplied by aquifer thickness (b) was derived from a combination of methods. An aquifer thickness of 10m was assumed as this was the maximum available drawdown available. These provided results that were in close agreement and indicated an aquifer transmissivity in the range of 0.15 to 0.4 m²/d (K = 0.015 to 0.04 m/d). Aquifer storage properties from the distance drawdown method also suggest a confined nature with a specific Storage value of 1.3×10^{-7} .

Table 2.10
Summary of Aquifer Properties

Method	T (m ² /sec)	T (m ² /day)	K (m/d)
Barker (Confined)	4.4×10^{-6}	0.38	0.038
Hantush with Aquitard Storage (Leaky Confined)	1.1×10^{-6}	0.10	0.010
Barker (Distance Drawdown TR7)	2.2×10^{-6}	0.19	0.019

Figure 2.21: Curve Matching Solution (Barker Confined)

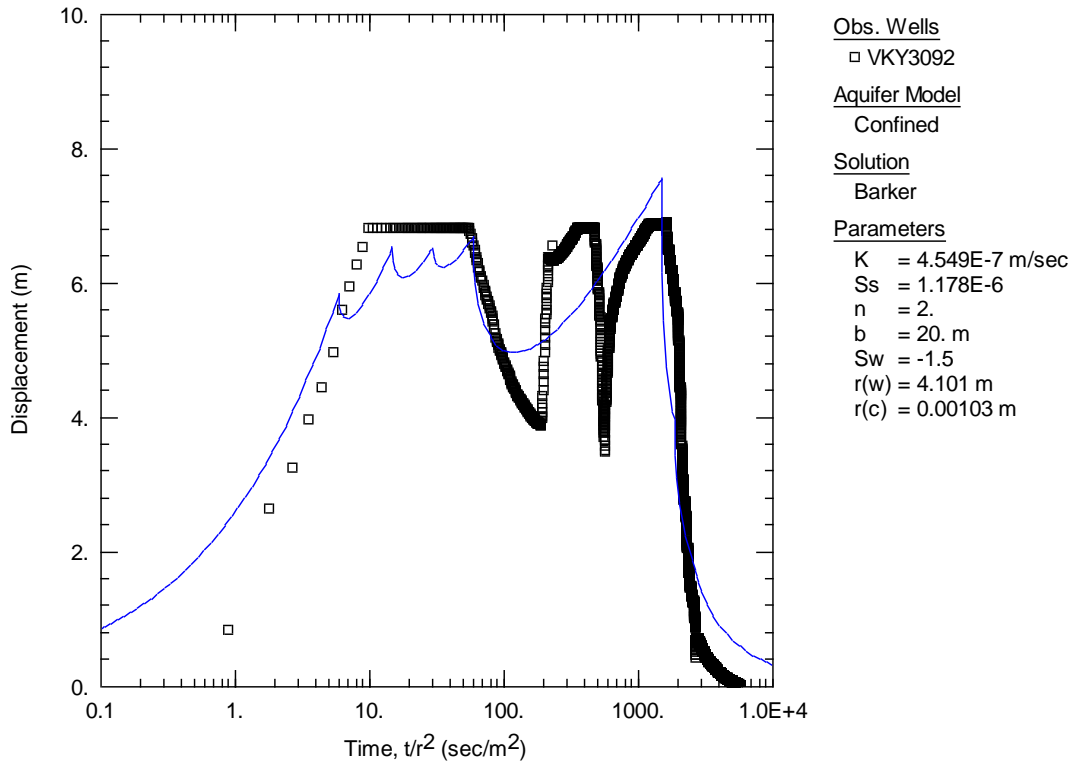


Figure 2.22: Curve Matching Solution (Hantush Leaky)

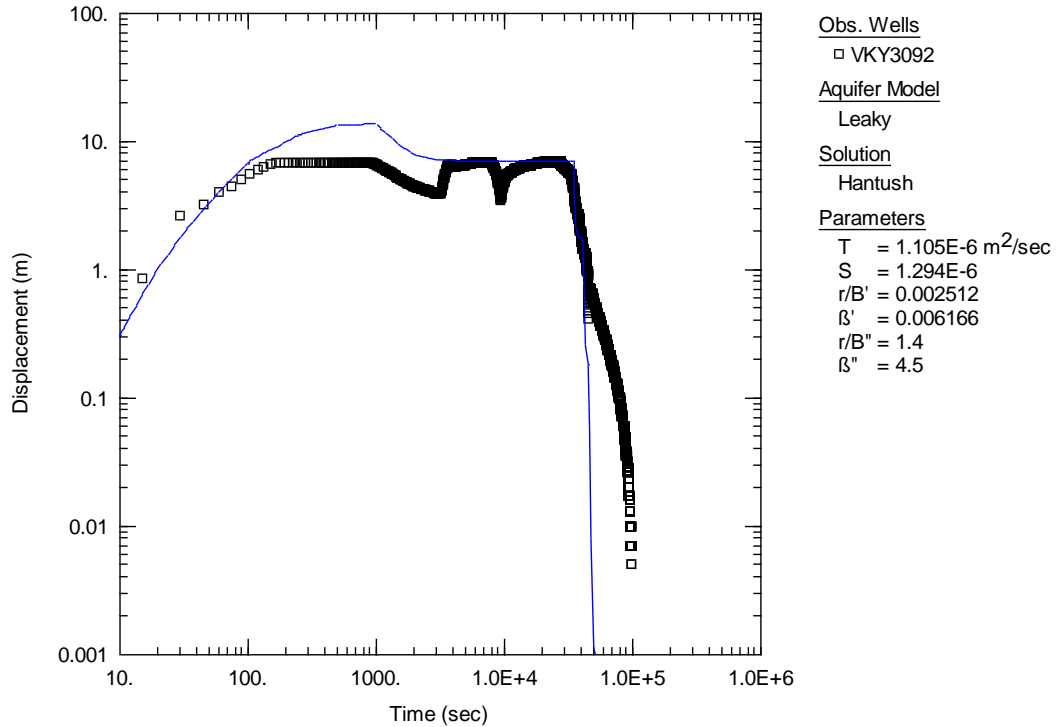
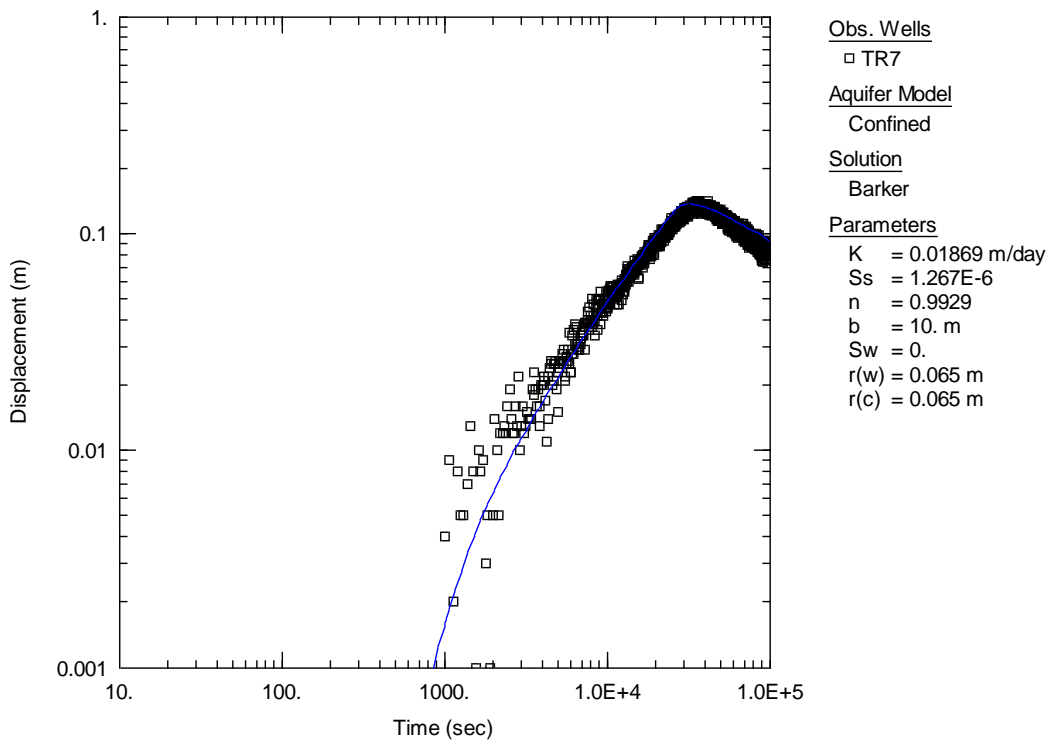


Figure 2.23: Curve Matching Solution (Distance Drawdown TR7 Confined Barker)



Groundwater Quality

Water quality samples were obtained from VKY3092 during the test at periods 1 hour after the start of the pump test, midway (6 hours) and at the end (12 hours) of the pump test. Field water quality data including pH and EC is summarised in **Table 2.11**. It shows that groundwater extracted has a very high level of salinity. Groundwater samples were taken in accordance with AS/NZS 5667 (Standards Australia, 1998) and water samples were sent for laboratory testing of a comprehensive suite of analytes which included:

- pH, EC and TDS;
- major cations and anions; and
- dissolved metals (As, B, Cd, Cr, Cu, Fe, Ni, Pb, Mn, Se, Zn, Hg).

**Table 2.11
 Summary of Field Water Quality Testing**

Time (hr)	pH	EC (µS/cm)	Lab sample ID
1	7.65	18600	TA122A
6	7.67	18600	TA122B
12	7.86	17300	TA122C

The laboratory analysis was undertaken by ALS Environmental, a NATA-accredited laboratory based in Sydney and results are contained in **Appendix F**.

Sustainable Bore Yield

The results show that the alluvium/colluvium contained in the area of flood plains to the south of the proposed Vickery open cut has very limited potential yield and that the bore tested (VKY3092) is not be able to sustain a pumping rate of above 0.25 L/s over the long term. Pumping at this rate resulted drawdown of approximately 8 m over the duration of the pump test. In addition, the groundwater levels did not reach equilibrium at the tested rate, and therefore pumping for extended periods of time may lead to drawdowns approaching maximum available drawdown.

To assess the sustainable yield, the maximum transmissivity value from the various analysis methods used above ($T=0.4 \text{ m}^2/\text{day}$) was applied to the Theis Analytical solution for distance drawdown. **Table 2.12** shows drawdown rates for pumping at the low rates of 2.5 and 5 m^3/day (0.06 and 0.12 L/s) over periods of 1 month, 1 year and 10 years. Maximum available drawdown for this bore is approximately 10 m. Longer term drawdown response suggest that the maximum sustainable yield for VKY3092 is in the order rates at less than 2.5 m^3/day .

**Table 2.12
Predicted Drawdown for Varying Pump Rate and Duration**

Pump Rate (m^3/day)	Time (days)	Drawdown (m)
2.5	30	10.5
2.5	365	11.8
2.5	3650	12.9
5	30	20.9
5	365	23.5
5	3650	25.9

FIGURES

Figure 1.1: Project Location

Figure 2.1: Location of Monitoring Bores

Figure 2.2: Transect Locations

Figure 2.3: Cross-section at Transect 1

Figure 2.4: Transect 1 Graphic Logs

Figure 2.5: Cross-section at Transect 2

Figure 2.6 Transect 2 Graphic Logs

Figure 2.7: Cross-section at Transect 3

Figure 2.8: Transect 3 Graphic Logs

Figure 2.9: Cross-section at Transect 4

Figure 2.10: Transect 4 Graphic Logs

Figure 2.11: Cross-section at Transect 5

Figure 2.12: Transect 5 Graphic Logs

Figure 2.13: Hydrograph and Hydrostatic Head Profile for VKY3033

Figure 2.14: Hydrograph and Hydrostatic Head Profile for VKY3041

Figure 2.15: Hydrograph and Hydrostatic Head Profile for VKY3053

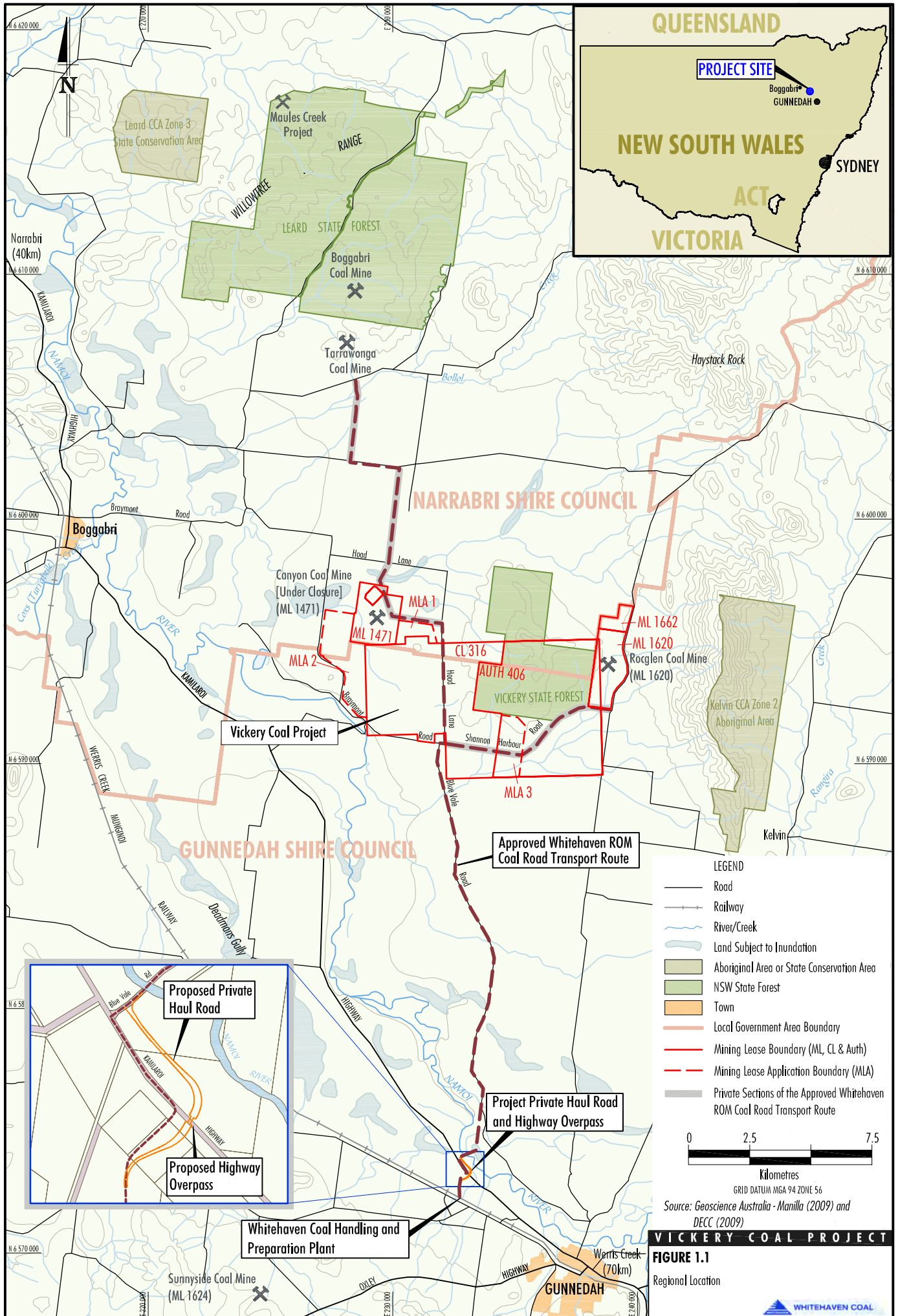
Figure 2.16: Hydrograph for Installed Standpipes

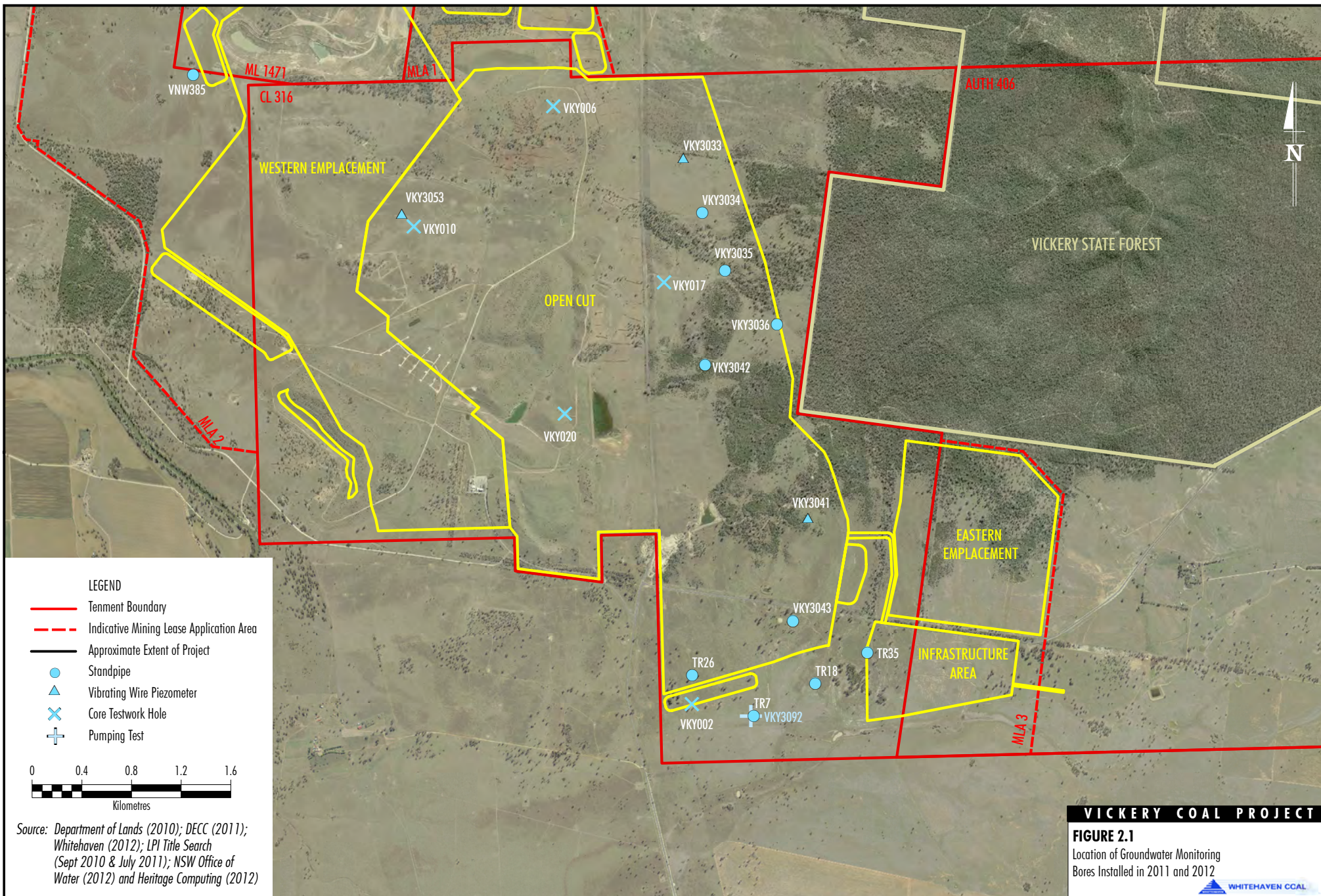
Figure 2.17: Extent of Saturated Alluvium

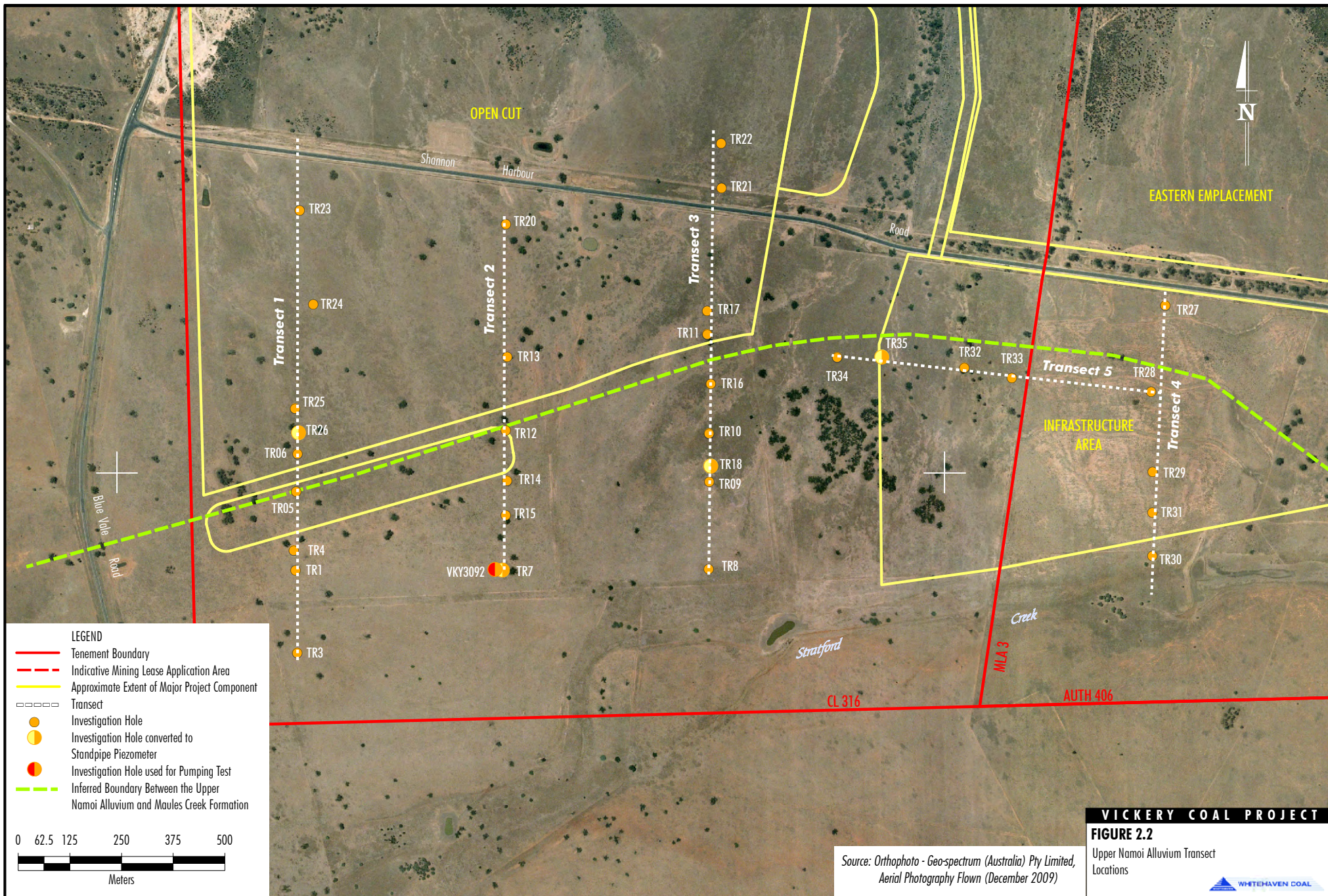
Figure 2.18 Extent of Saturated Highly Weathered Permian Conglomerate/Sandstone

Figure 2.19: Piper diagram

Figure 2.20: Core Test Data - Hydraulic Conductivity vs. Depth







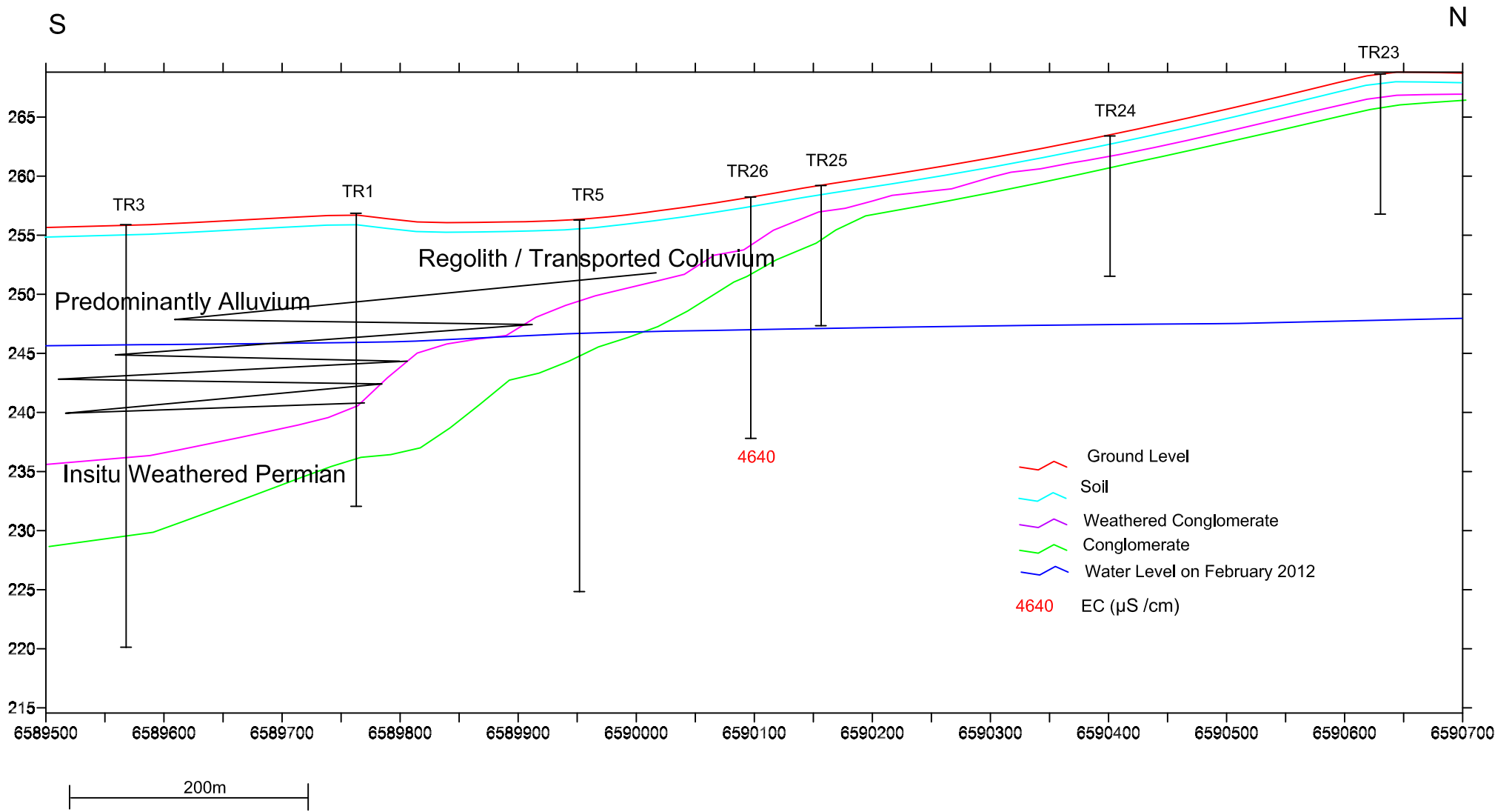
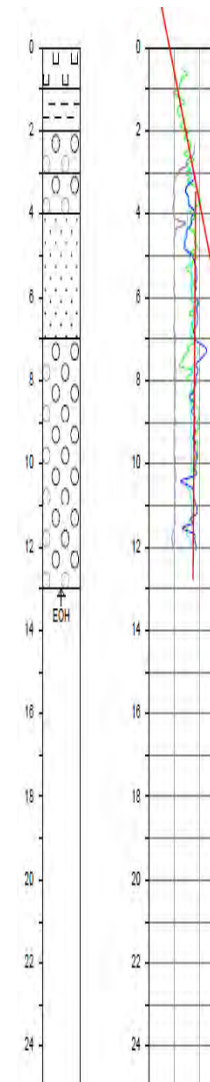
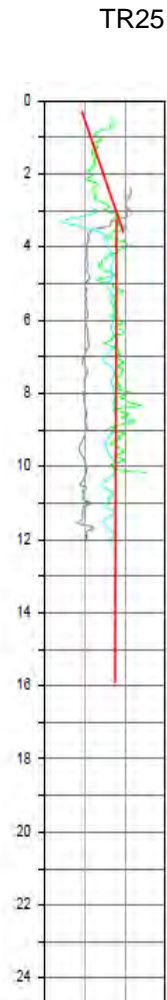
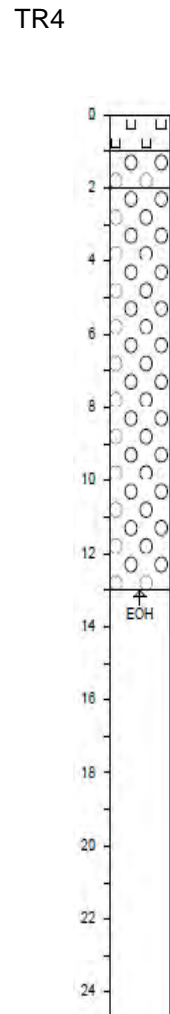
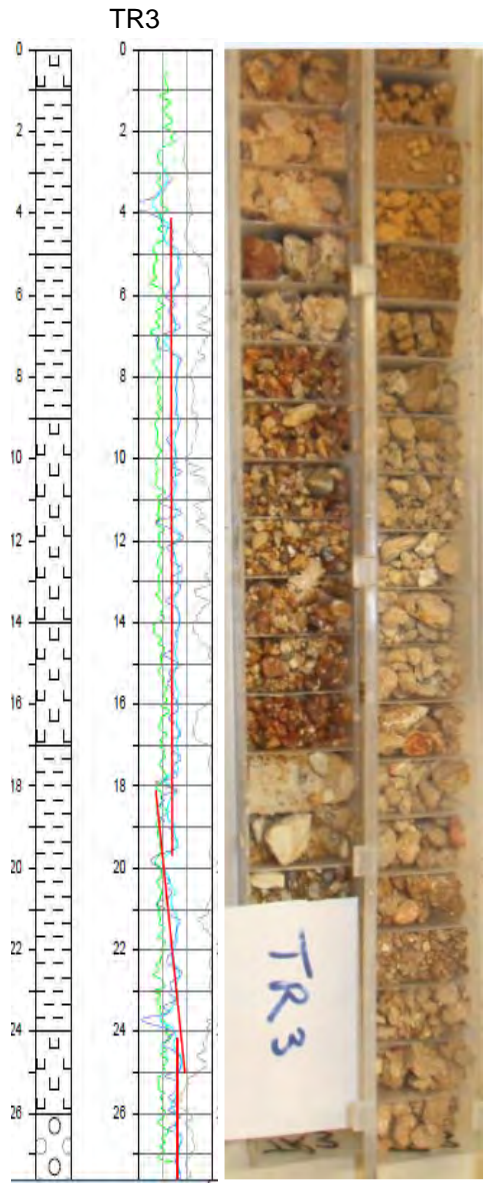


Figure 2.3 Cross-section at Transect 1



Date	20/02/2012	Scale
Initials	AF	Project
Drawing Number		Revision A

Whitehaven Coal Pty Ltd

Transect 3 Graphic Logs

Figure 2.4

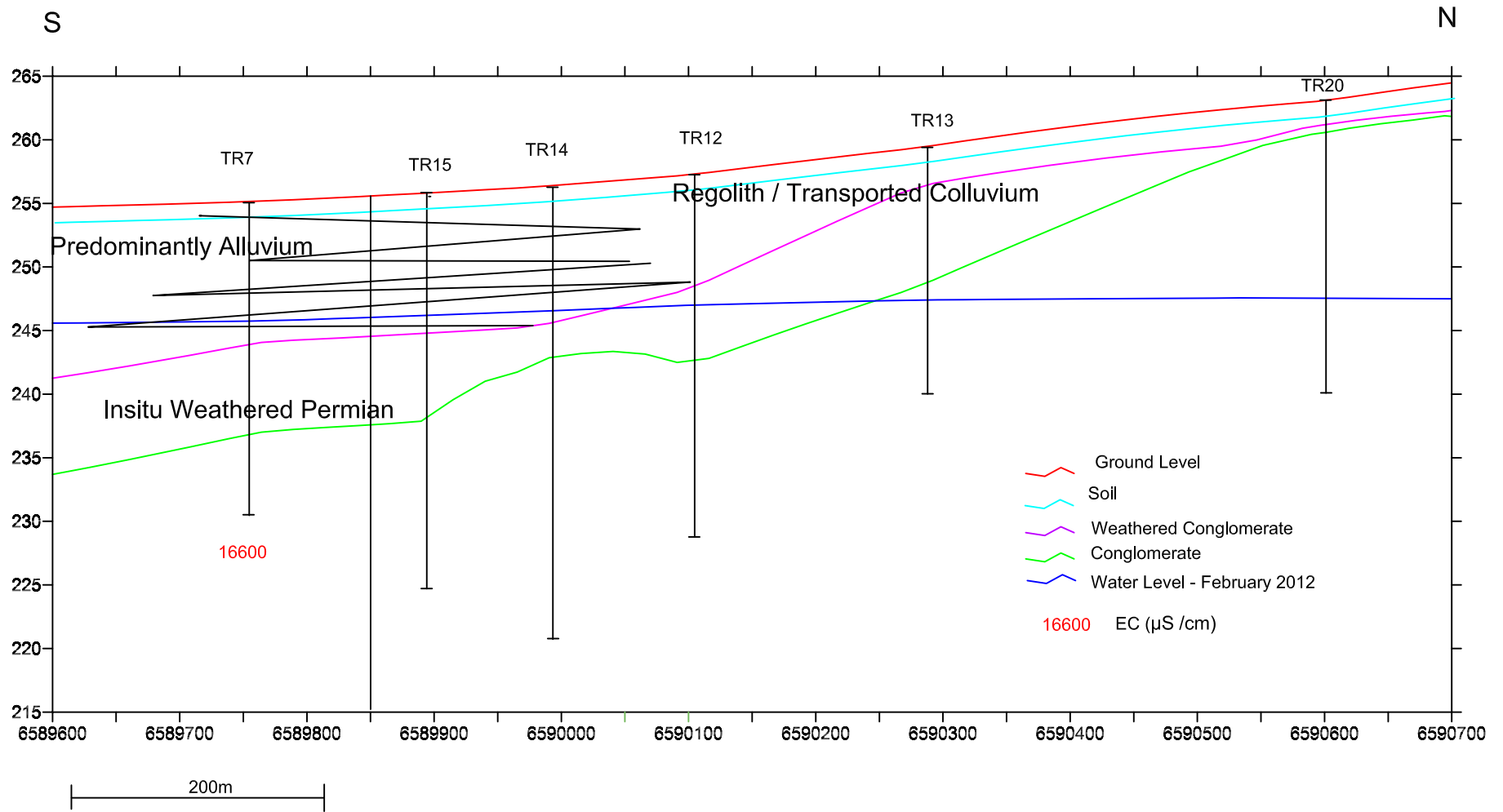
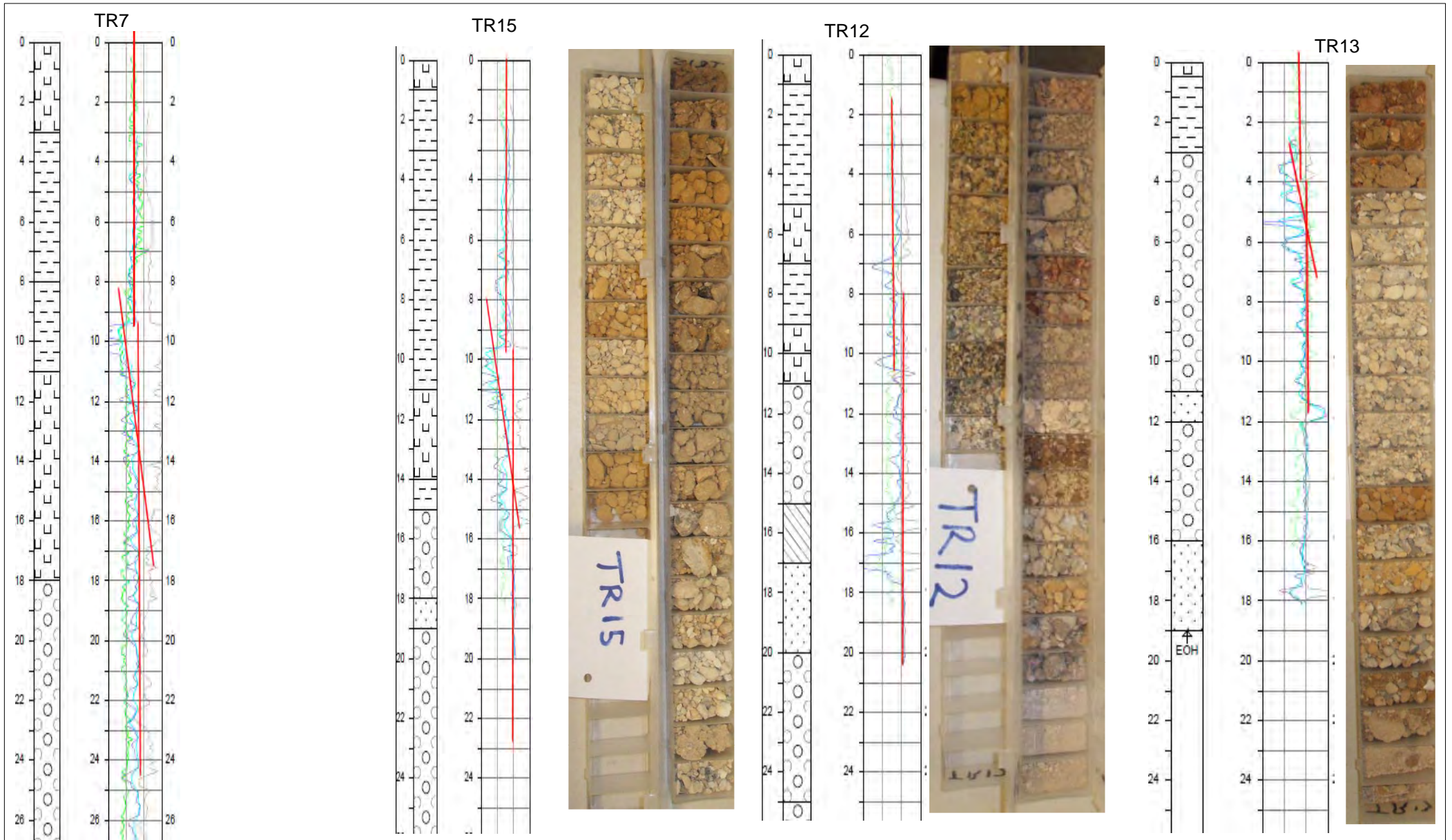


Figure 2.5 Cross-section at Transect 2



Date	20/02/2012	Scale
Initials	AF	Project
Drawing Number		Revision A

Whitehaven Coal Pty Ltd

Transect 2 Graphic Logs

Figure 2.6

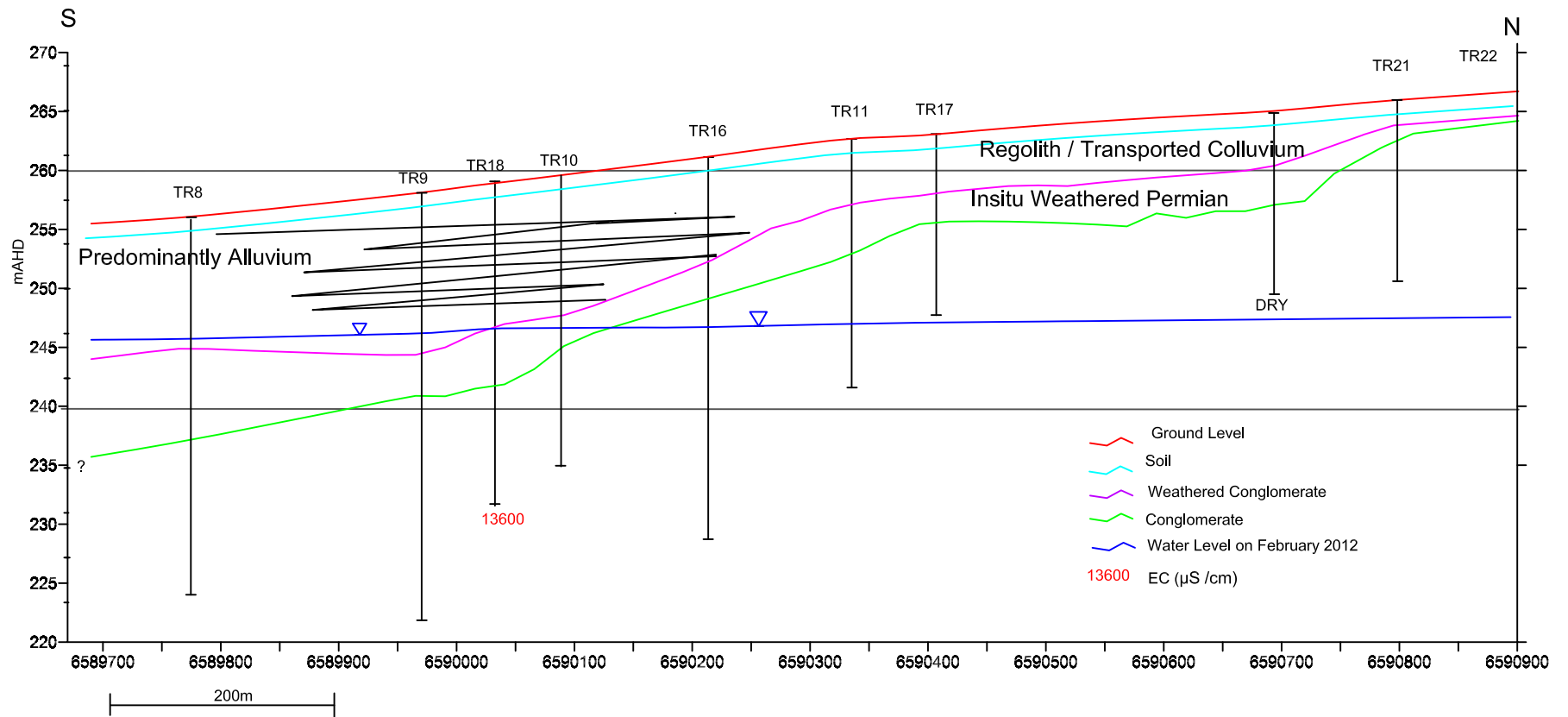
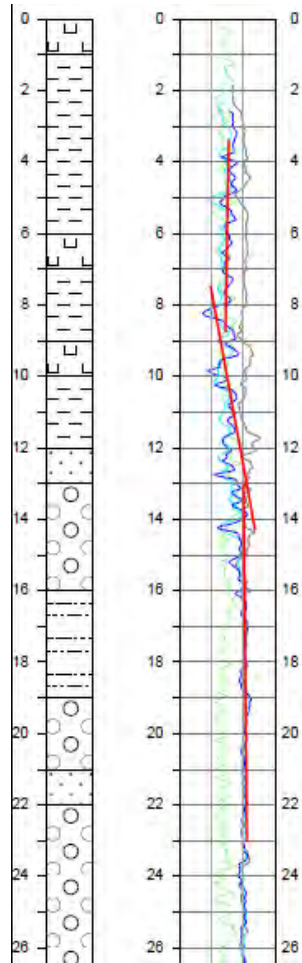


Figure 2.7 Cross-section at Transect 3

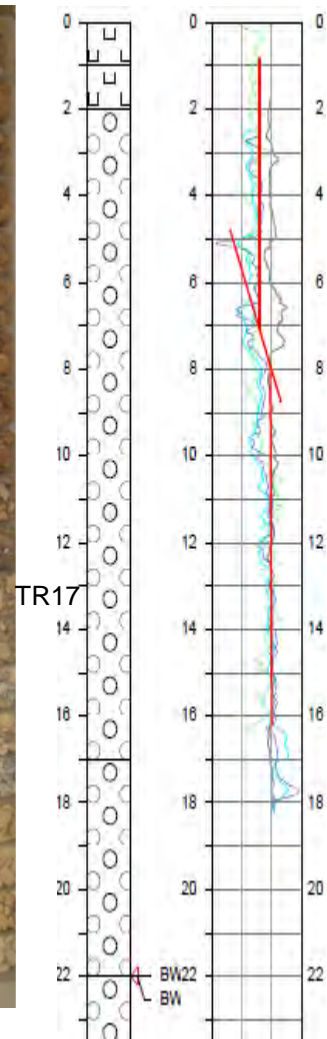
TR18



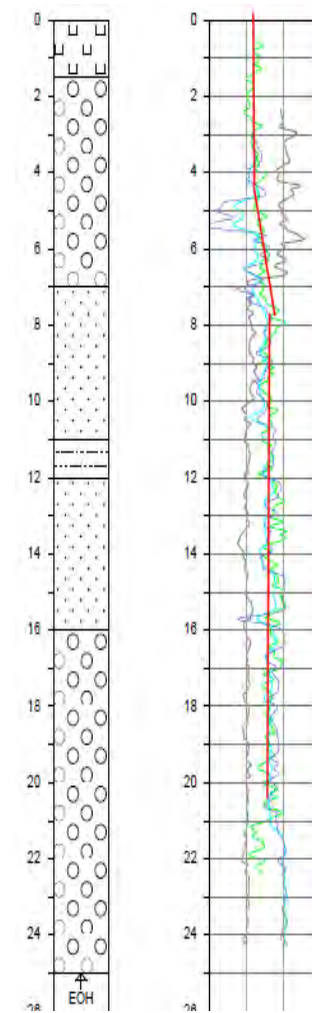
TR16



TR17



TR22



Date	20/02/2012	Scale
Initials	AF	Project
Drawing Number		Revision A

Whitehaven Coal Pty Ltd

Transect 3 Graphic Logs

Figure 2.8

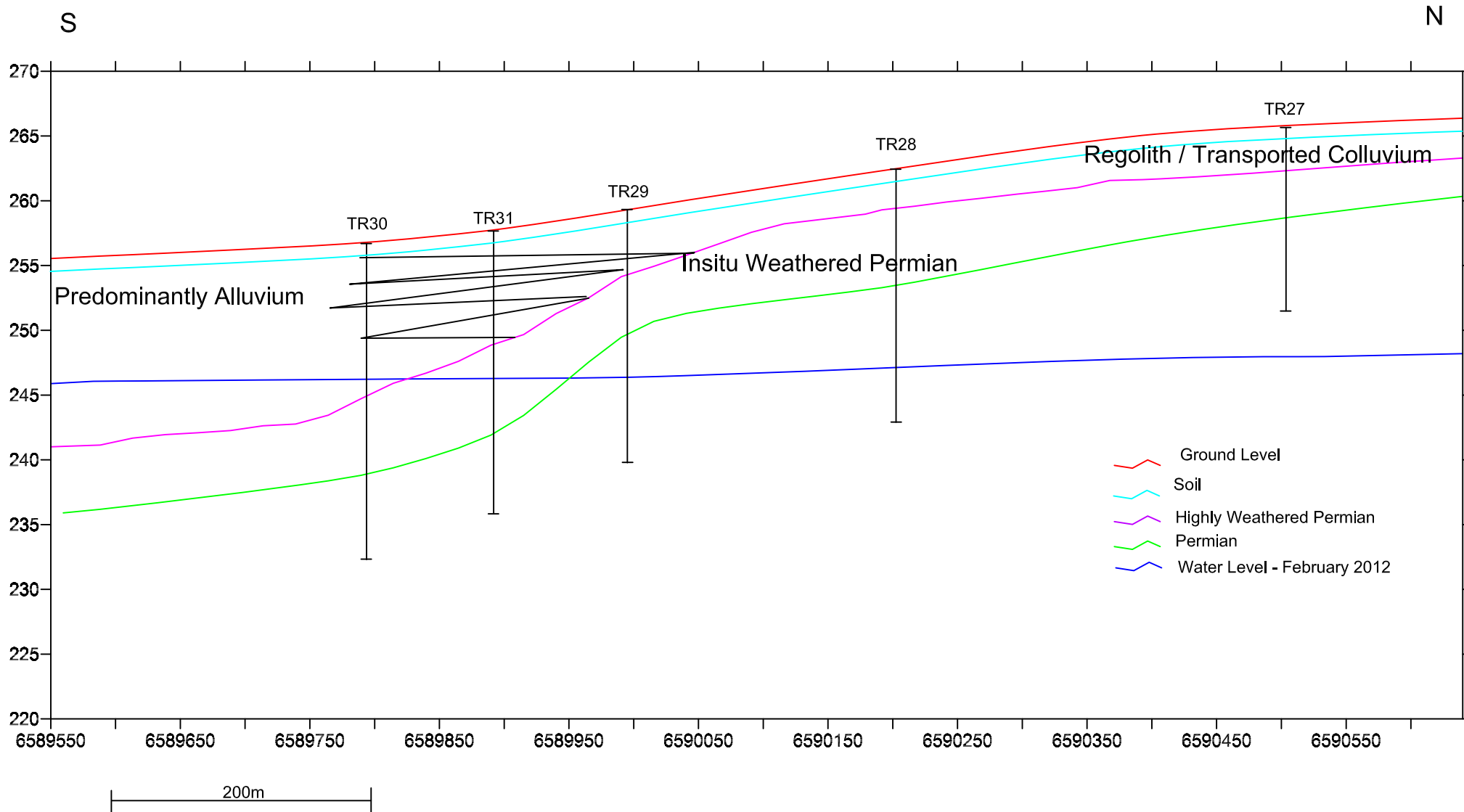
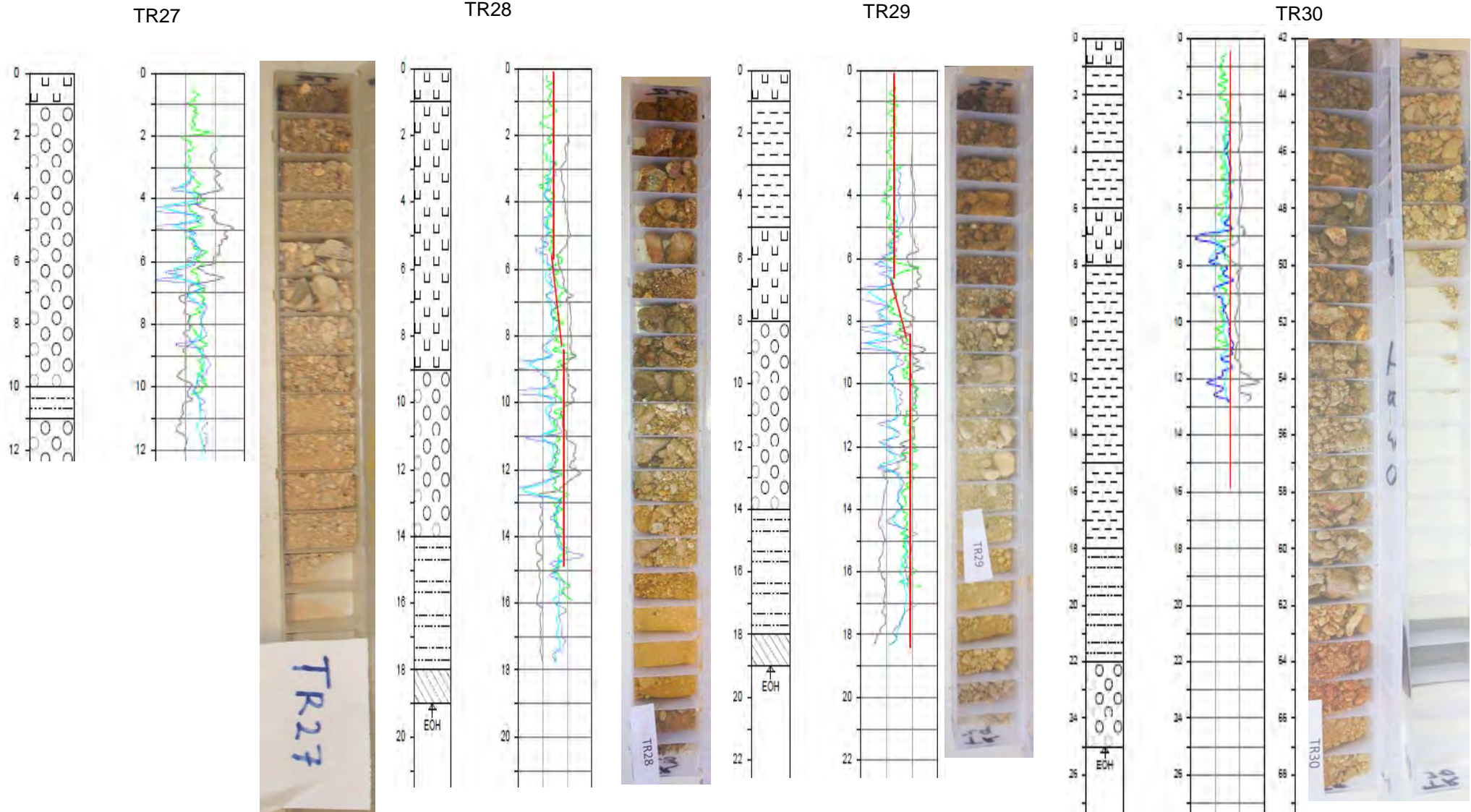


Figure 2.9 Cross-section at Transect 4



Date	20/02/2012	Scale
Initials	AF	Project
Drawing Number		Revision A

Whitehaven Coal Pty Ltd

Transect 4 Graphic Logs

Figure 2.10

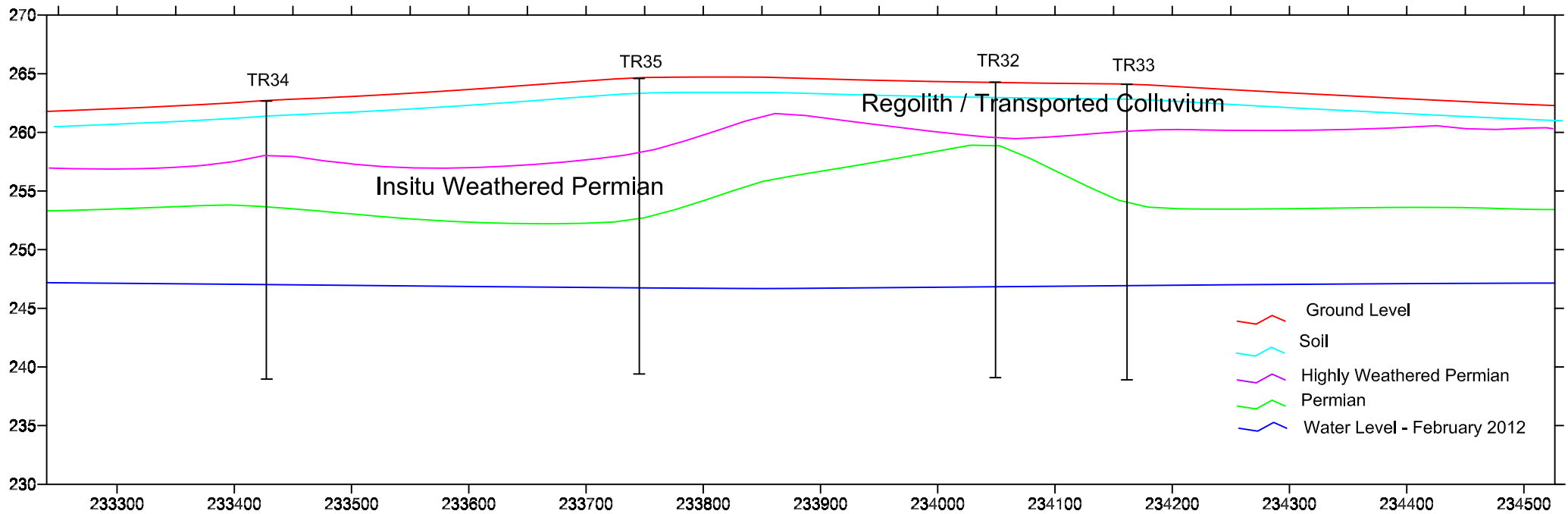


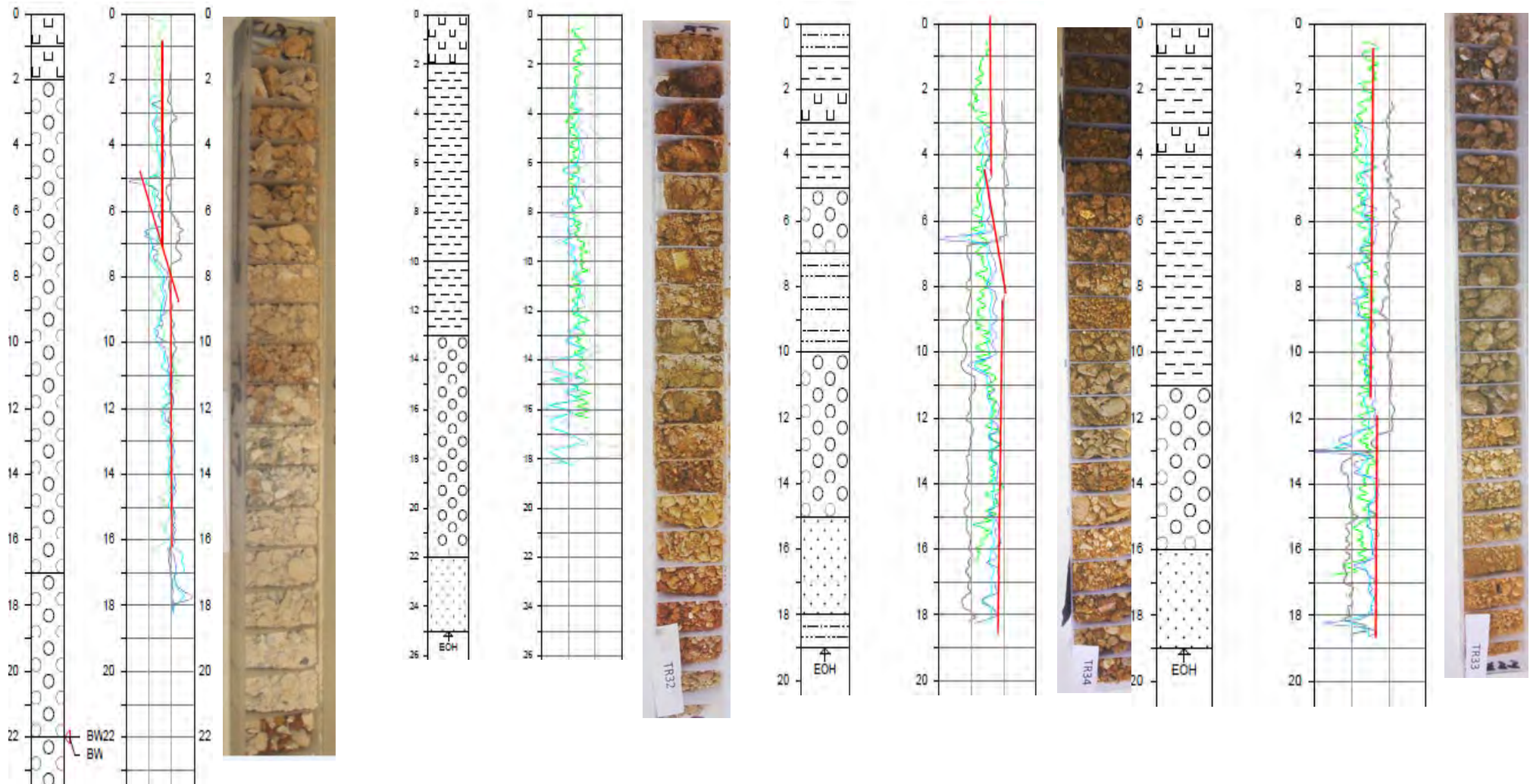
Figure 2.11 Cross-section at Transect 5

TR17

TR34

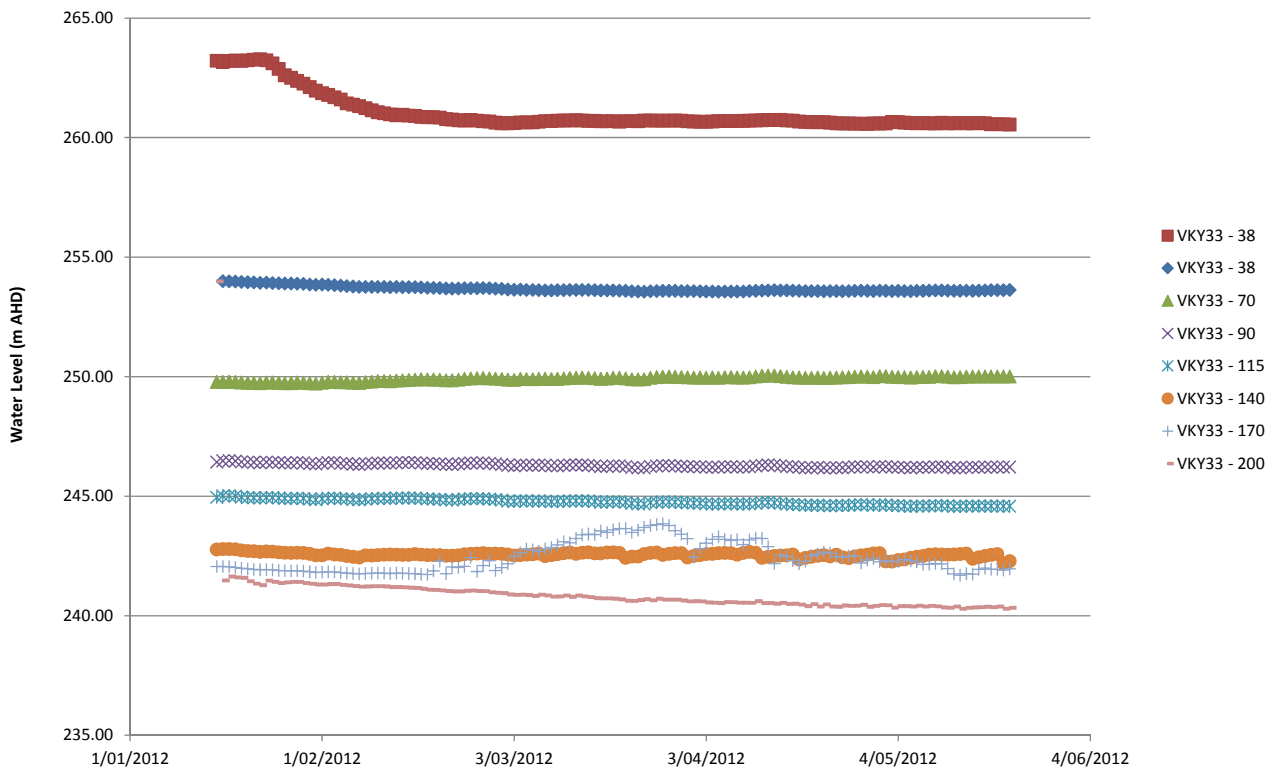
TR32

TR33

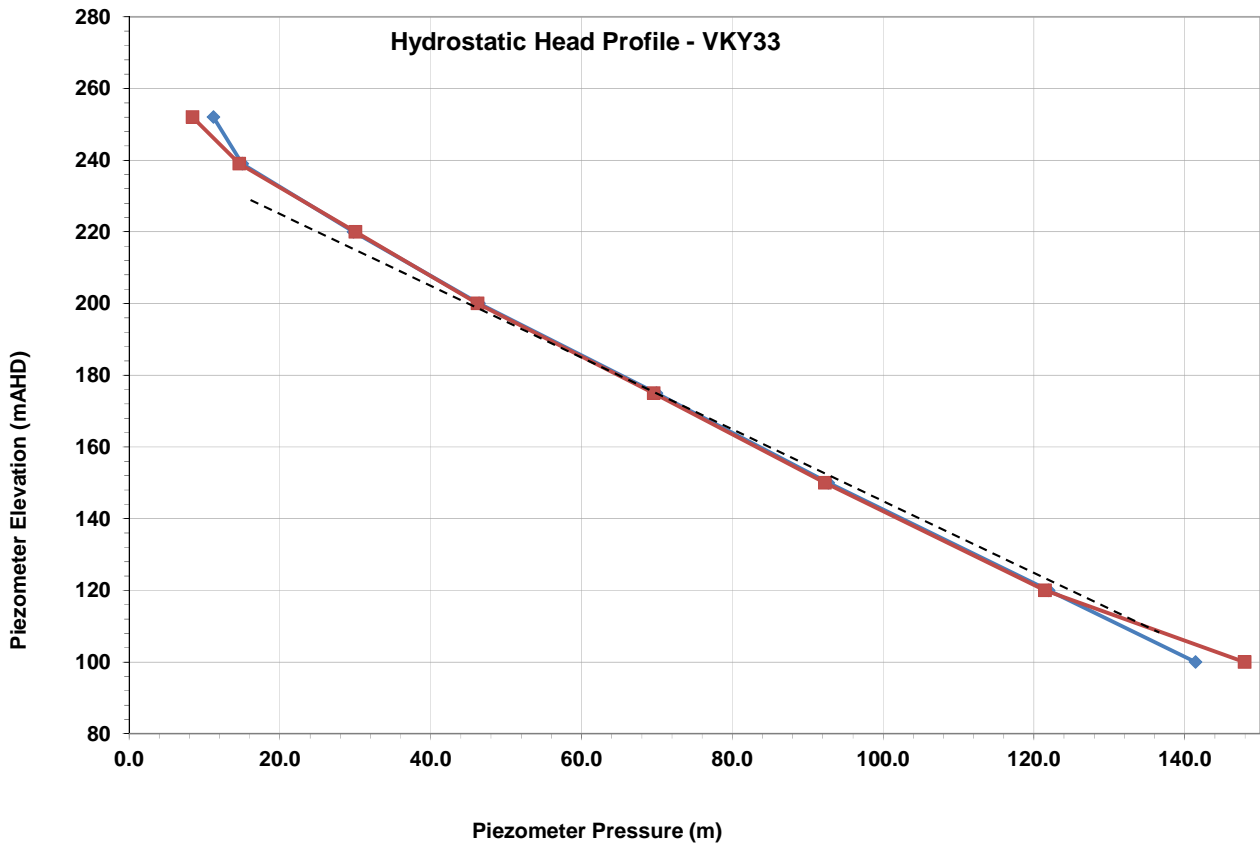


Date	20/02/2012	Scale	Whitehaven Coal Pty Ltd
Initials	AF	Project	
Drawing Number		Revision A	
			Transect 5 Graphic Logs
			Figure 2.12

VKY5053 Hydrograph



Hydrostatic Head Profile - VKY33



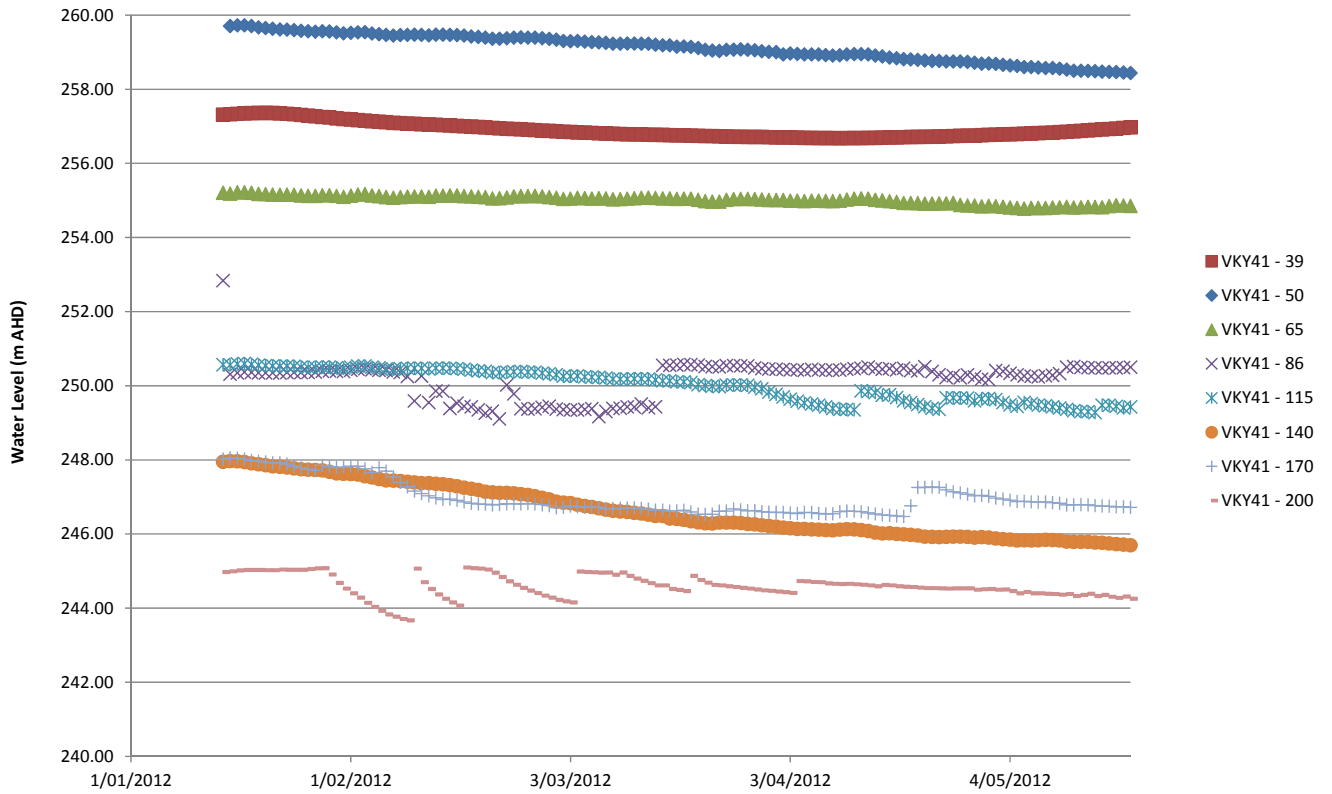
Date	22 May 2012	Scale	As Shown
Drawn by	AF	Checked by	Project
Drawing Number		Revision	0

Whitehaven Coal Pty Ltd

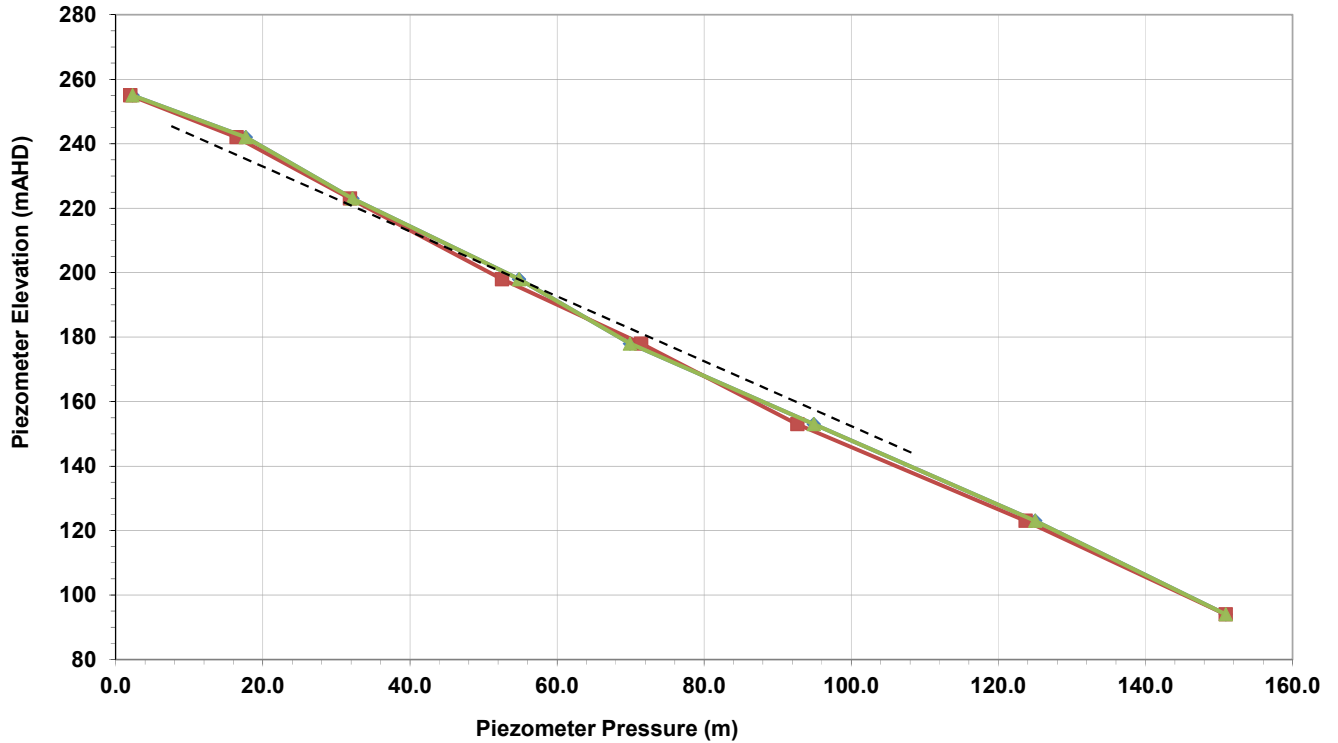
Hydrograph and Hydrostatic Head Profile for VKY3033

Figure 2.13

VKY41 Hydrograph

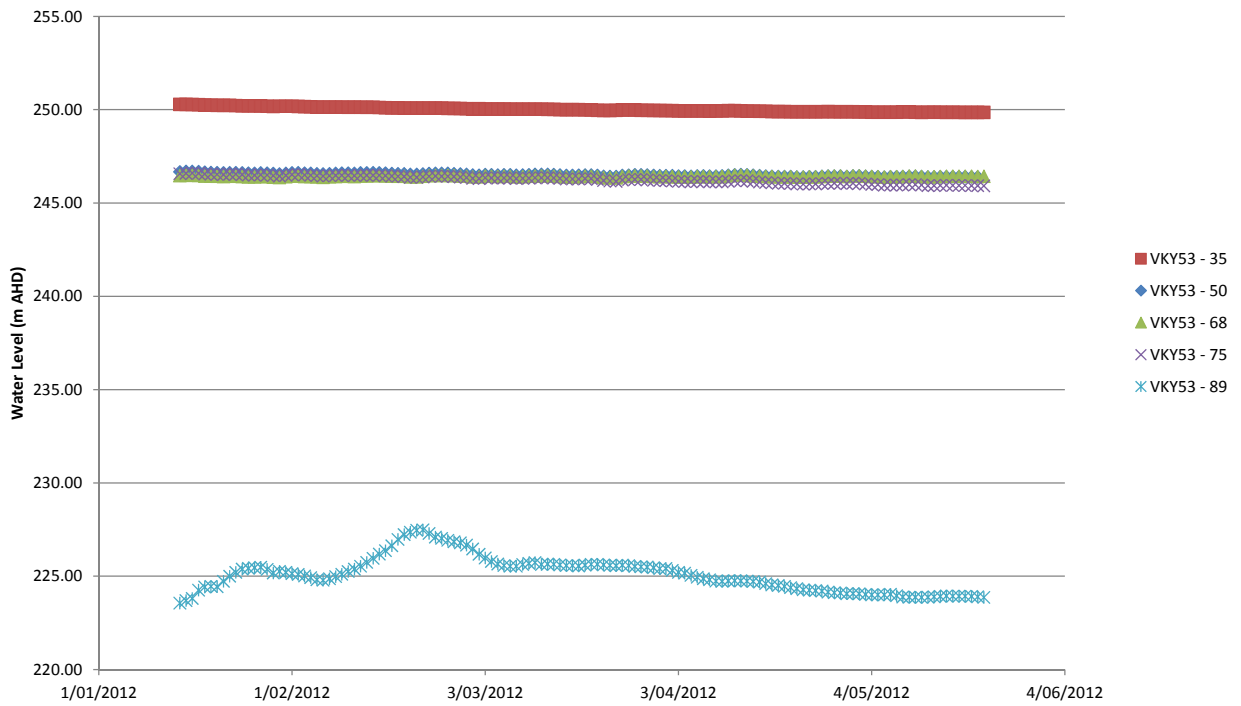


Hydrostatic Head Profile - VKY41

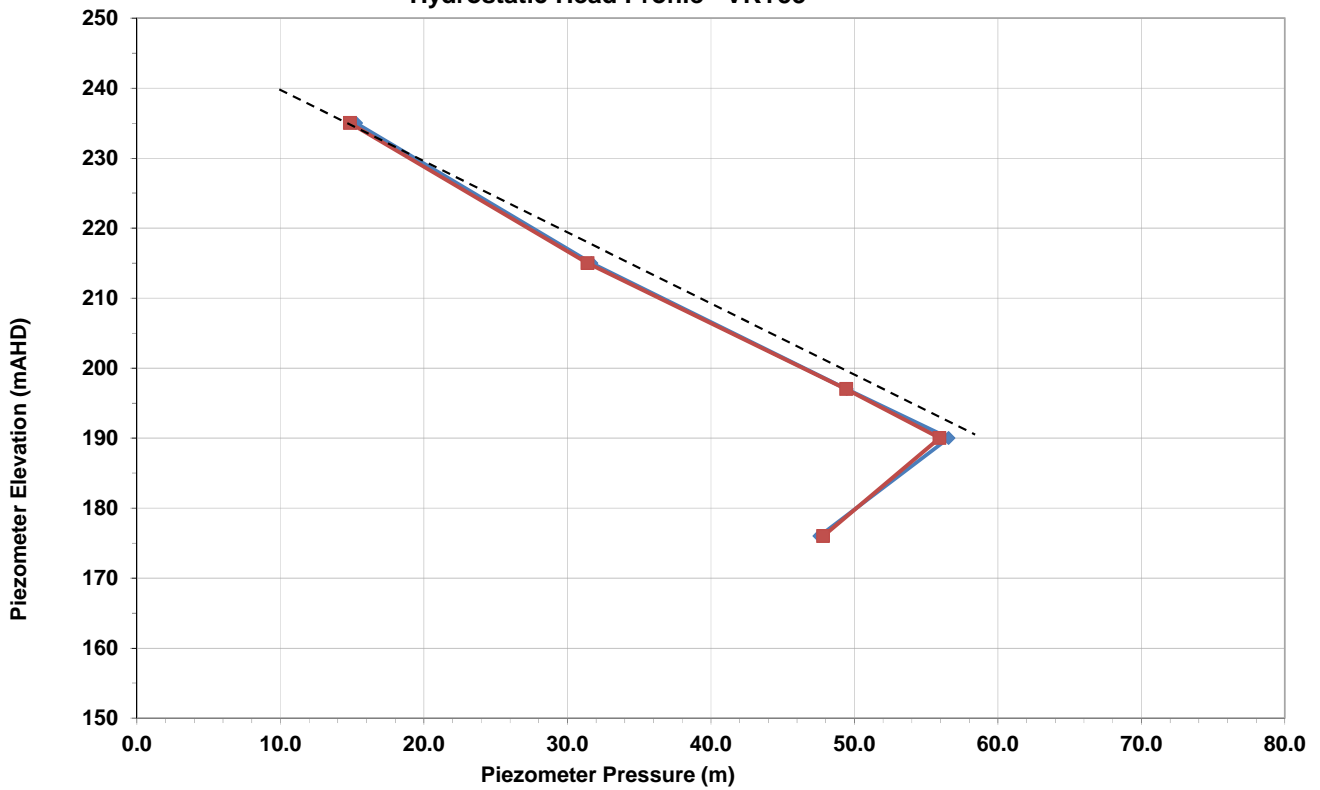


Date	22 May 2012	Scale	As Shown	Whitehaven Coal Pty Ltd Hydrograph and Hydrostatic Head Profile for VKY3041 Figure 2.14
Drawn by	AF	Checked by		
Drawing Number		Project		
		Revision	0	

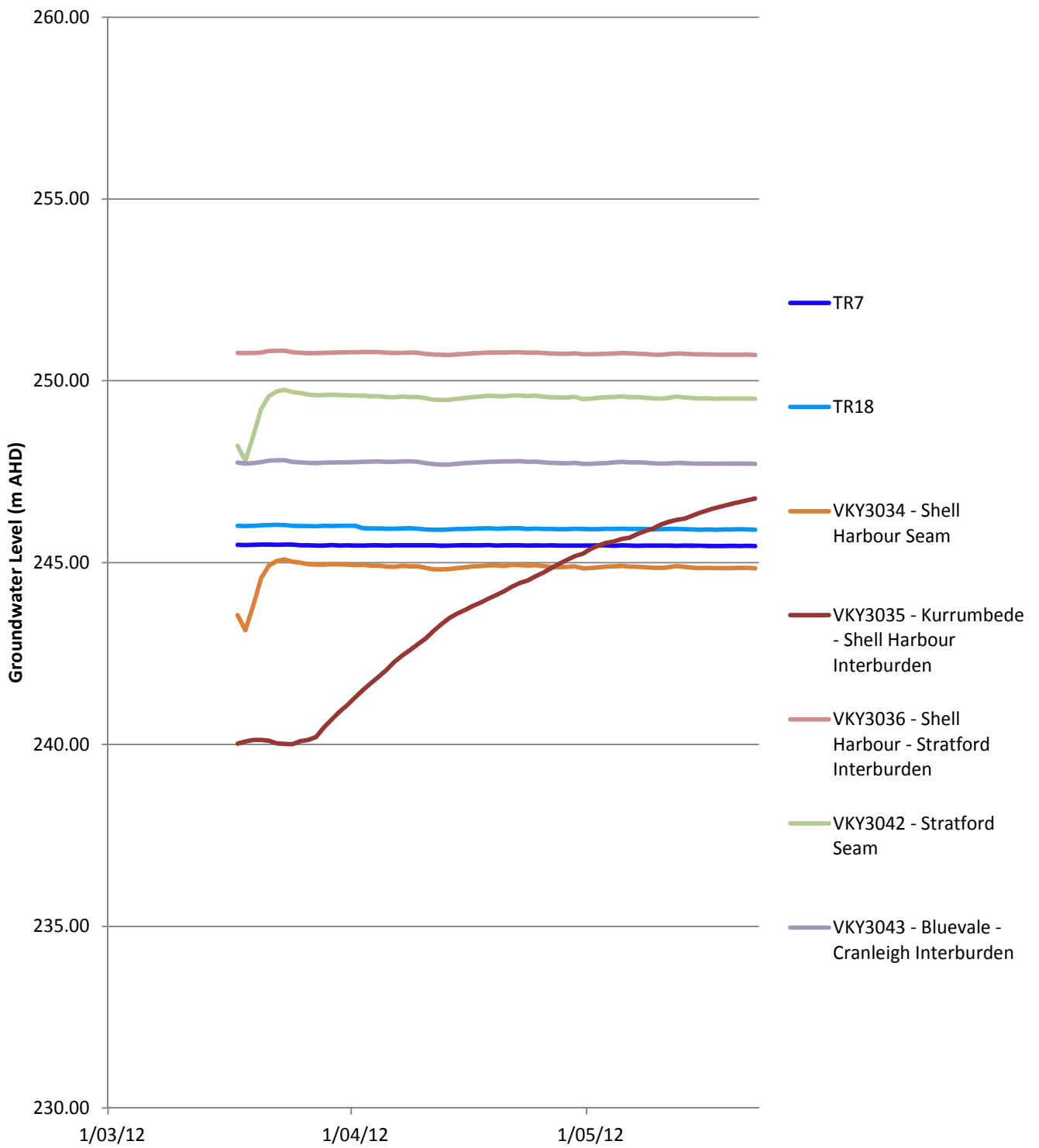
VKY3053 Hydrograph



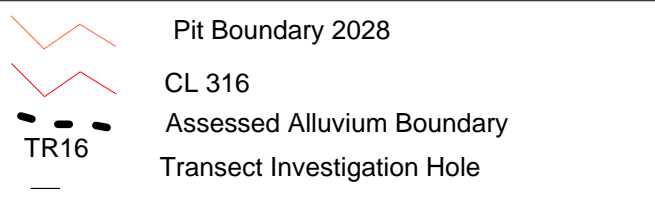
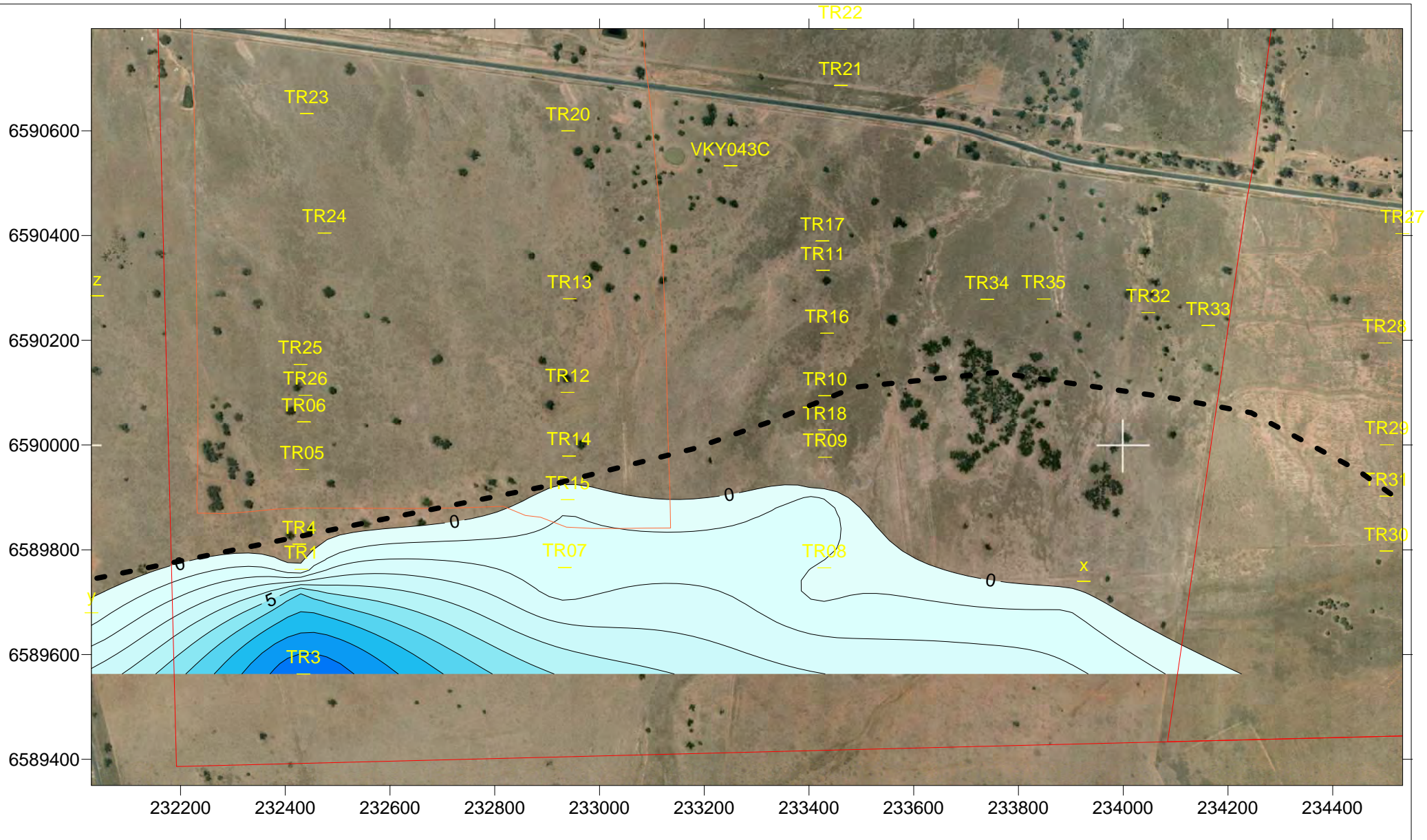
Hydrostatic Head Profile - VKY53



Date	22 May 2012	Scale	As Shown	Whitehaven Coal Pty Ltd Hydrograph and Hydrostatic Head Profile for VKY3053 Figure 2.15
Drawn by	AF	Checked by	Project	
Drawing Number		Revision	0	



Date	22 May 2012	Scale	As Shown	Whitehaven Coal Pty Ltd Hydrograph for Installed Standpipes
Drawn by	AF	Checked by	Project	
Drawing Number		Revision	0	
				Figure 2.16

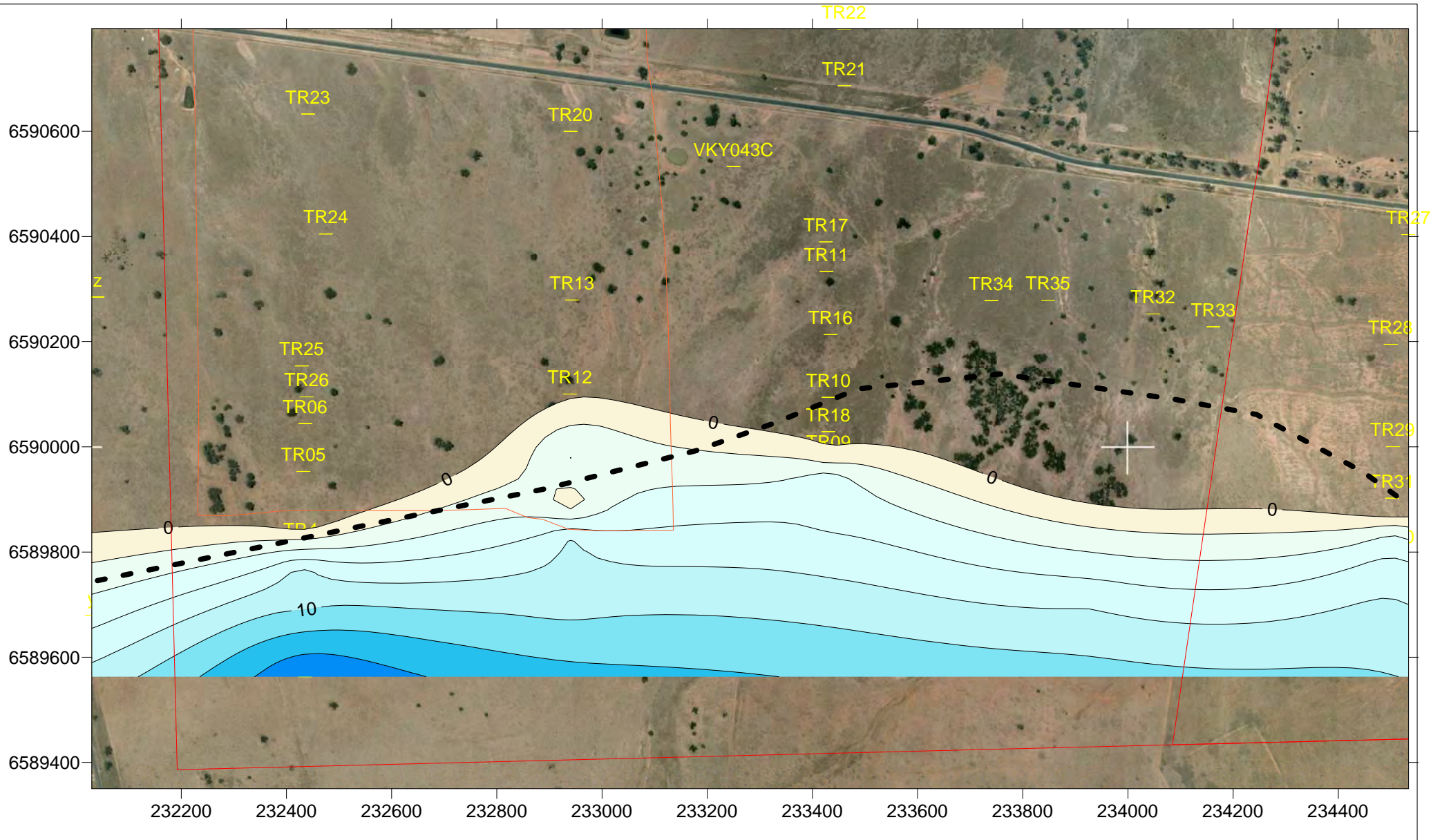





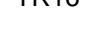

Date	14/04/2011	Scale	As Shown
Initials	AF	Project	
Drawing Number		Revision	A

Whitehaven Coal

Extent of Saturated Alluvium (m)

Figure 2.17



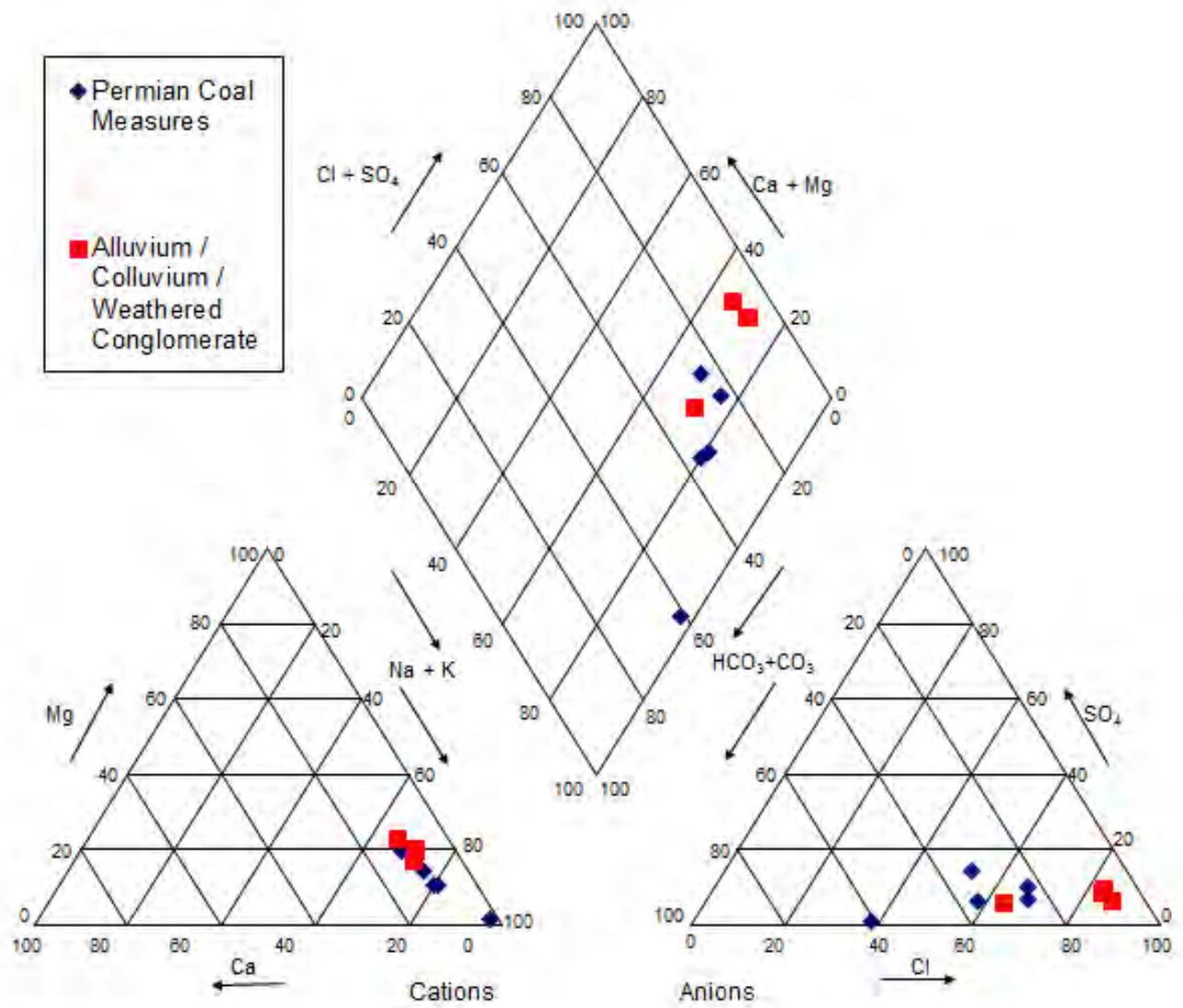
-  Pit Boundary 2028
-  CL 316
-  Assessed Alluvium Boundary
-  TR16
- 

Date	14/04/2011	Scale	As Shown
Initials	AF	Project	
Drawing Number		Revision	A

Whitehaven Coal

Extent of Saturated Highly Weathered Permian Conglomerate / Sandstones(m)

Figure 2.18

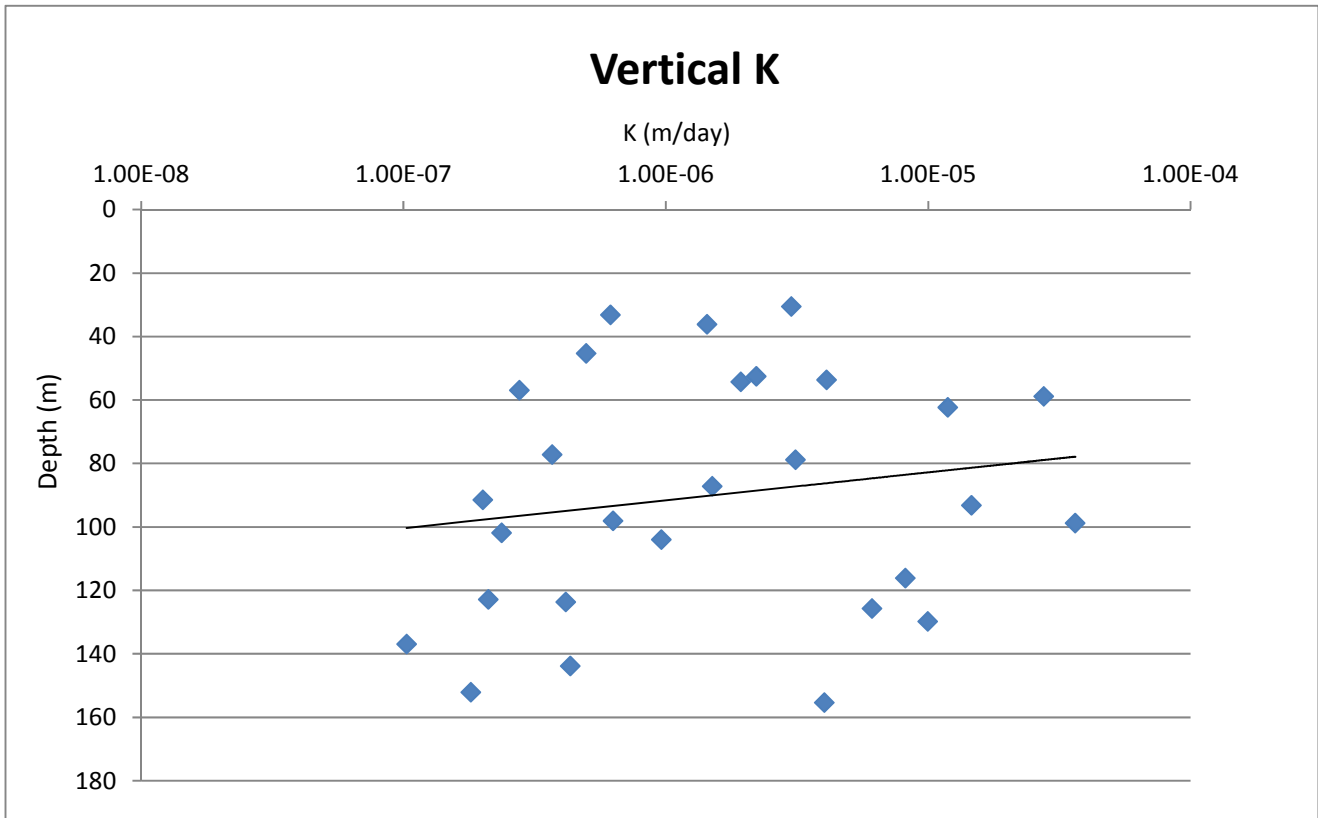
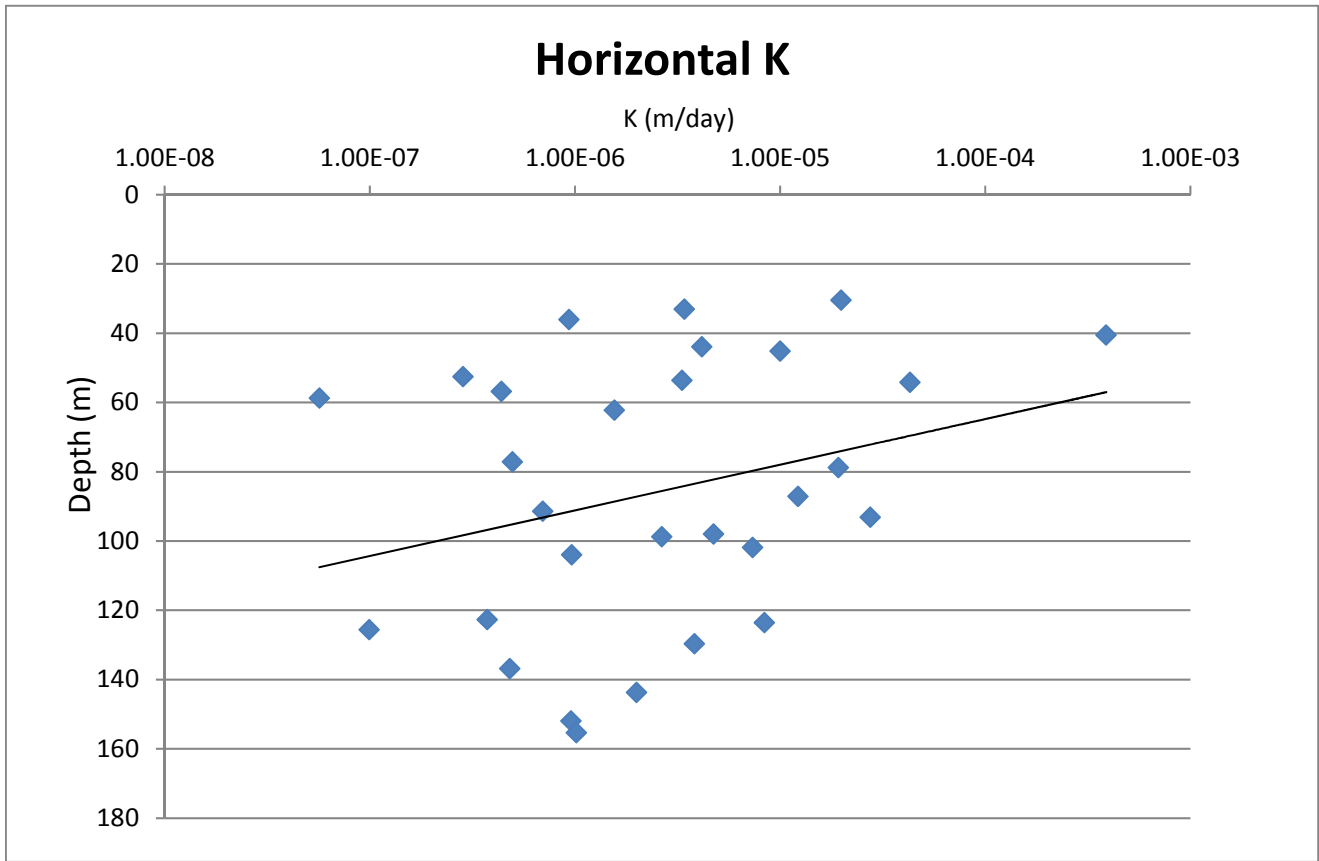


Date	22 May 2012	Scale	As Shown
Drawn by	AF	Checked by	Project
Drawing Number		Revision	0

Whitehaven Coal Pty Ltd

Piper Diagram

Figure 2.19



Date	12 March 2012	Scale	NA	Whitehaven Coal Pty Ltd Core Test Data - Hydraulic Conductivity (m/d) vs. Depth (m)
Drawn by	AF	Checked by	Project	
Drawing Number		Revision	0	

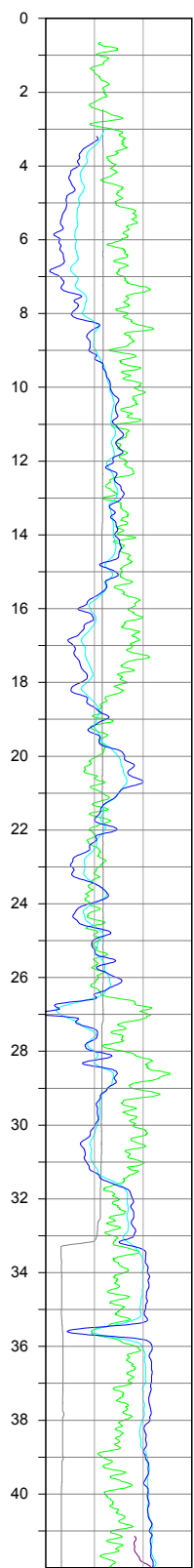
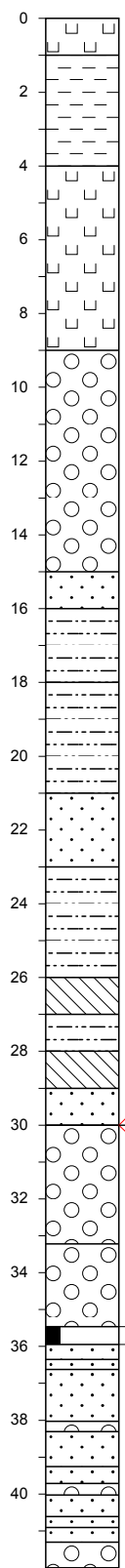
Figure 2.20

APPENDIX A

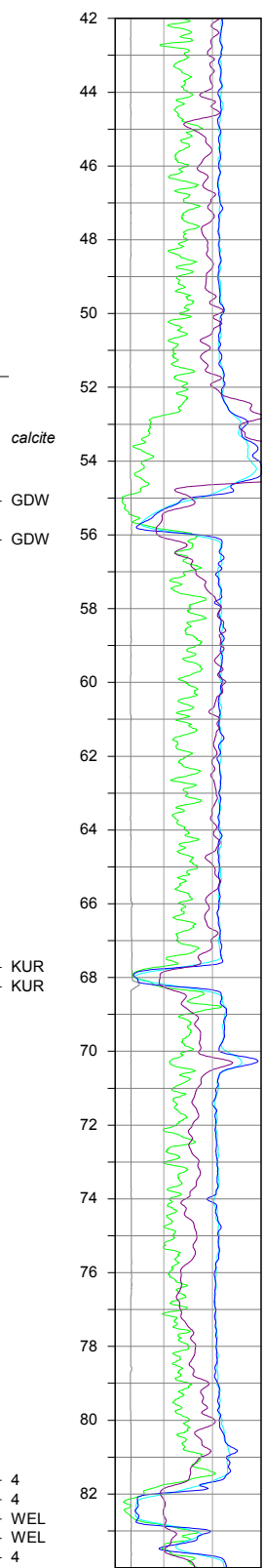
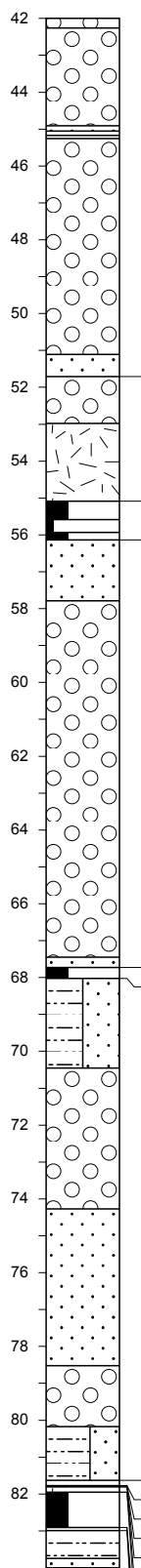
STANDPIPE PIEZOMETER BORE LOGS AND LICENCE DETAILS

Table A1: Standpipe Piezometer Drillhole Details

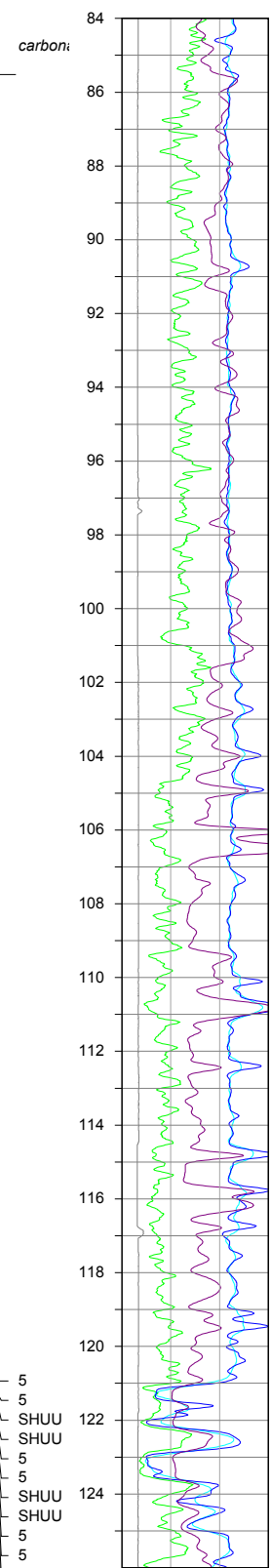
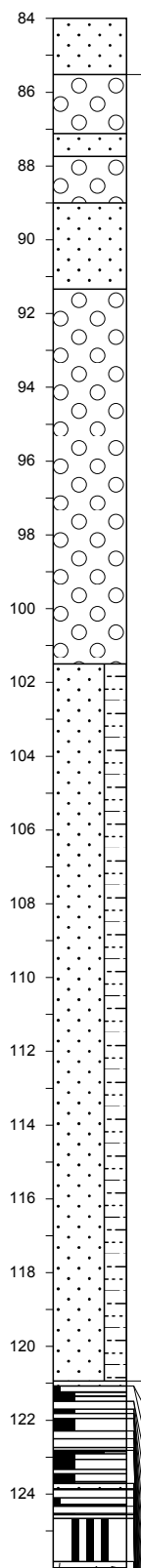
Hole ID	Easting (MGA)	Northing (MGA)	Bore Depth (m)
VKY3034	232519	6593822	200
VKY3035	232703	6593356	214
VKY3036	233120	6592921	225
VKY3042	232543	6592598	219.2
VKY3043	233250	6590533	245



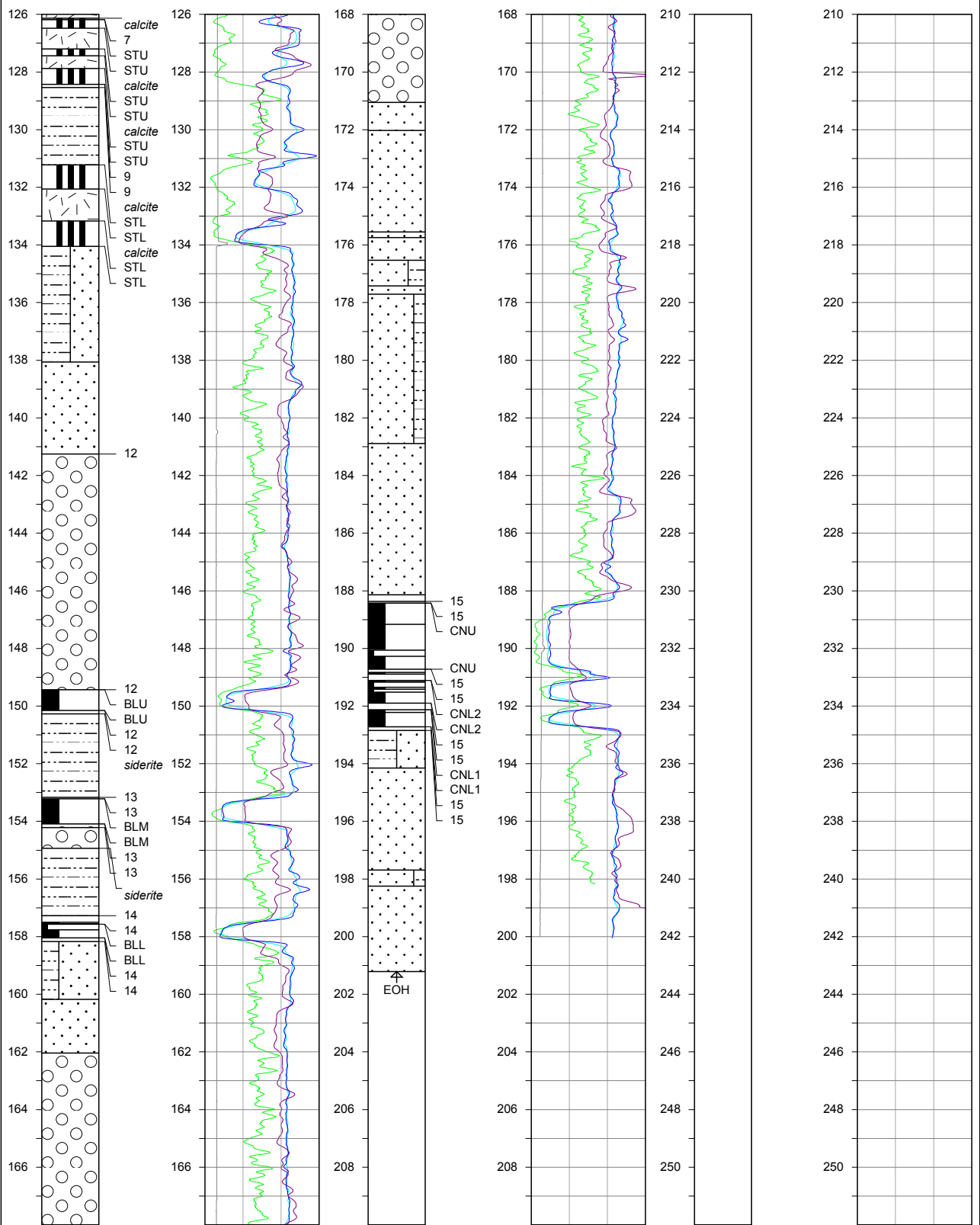
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 1.0 DENB 3.0
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 1.0 DENB 3.0
 1000.0 VL2F 5000



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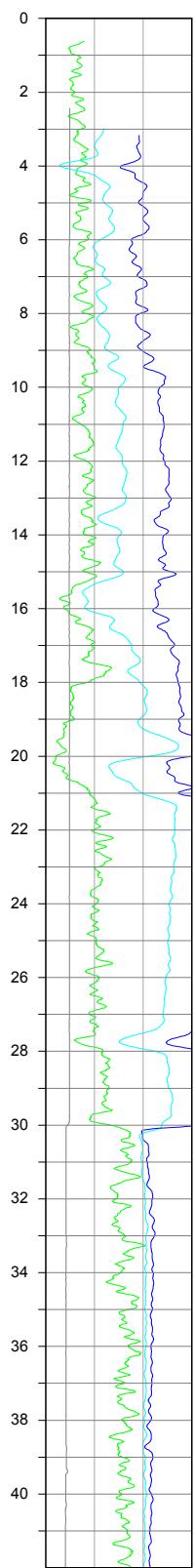
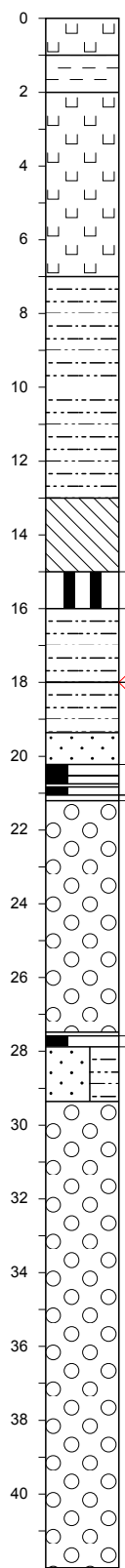
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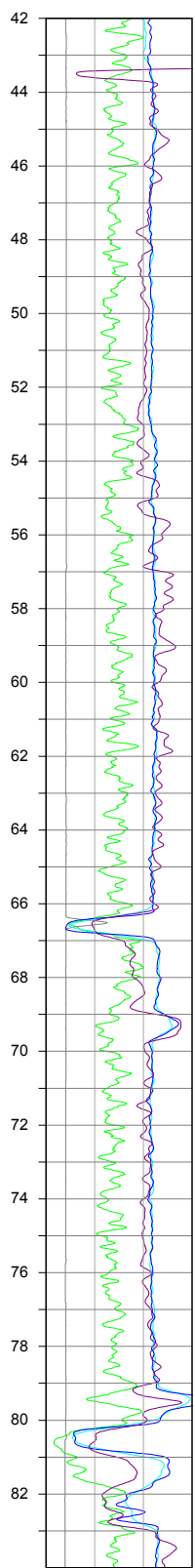
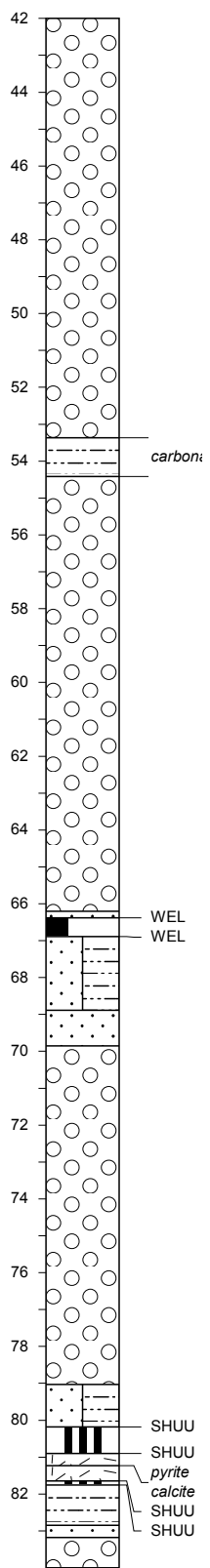


VKY034C

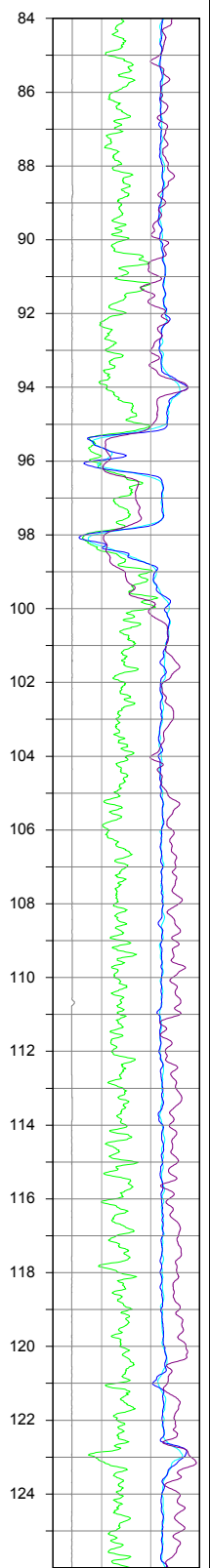
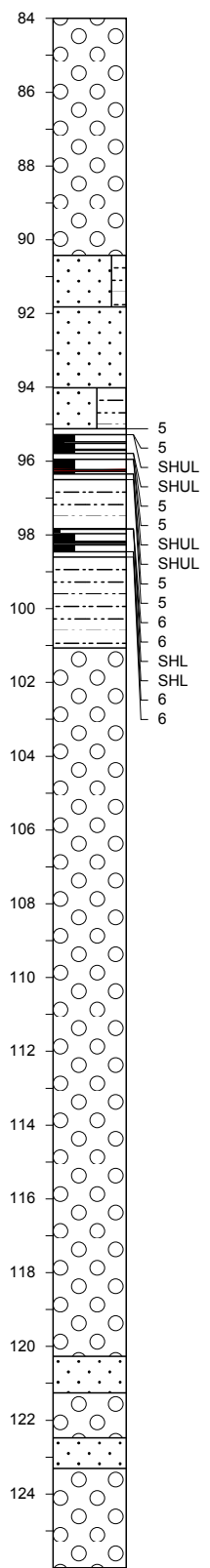
Vickery



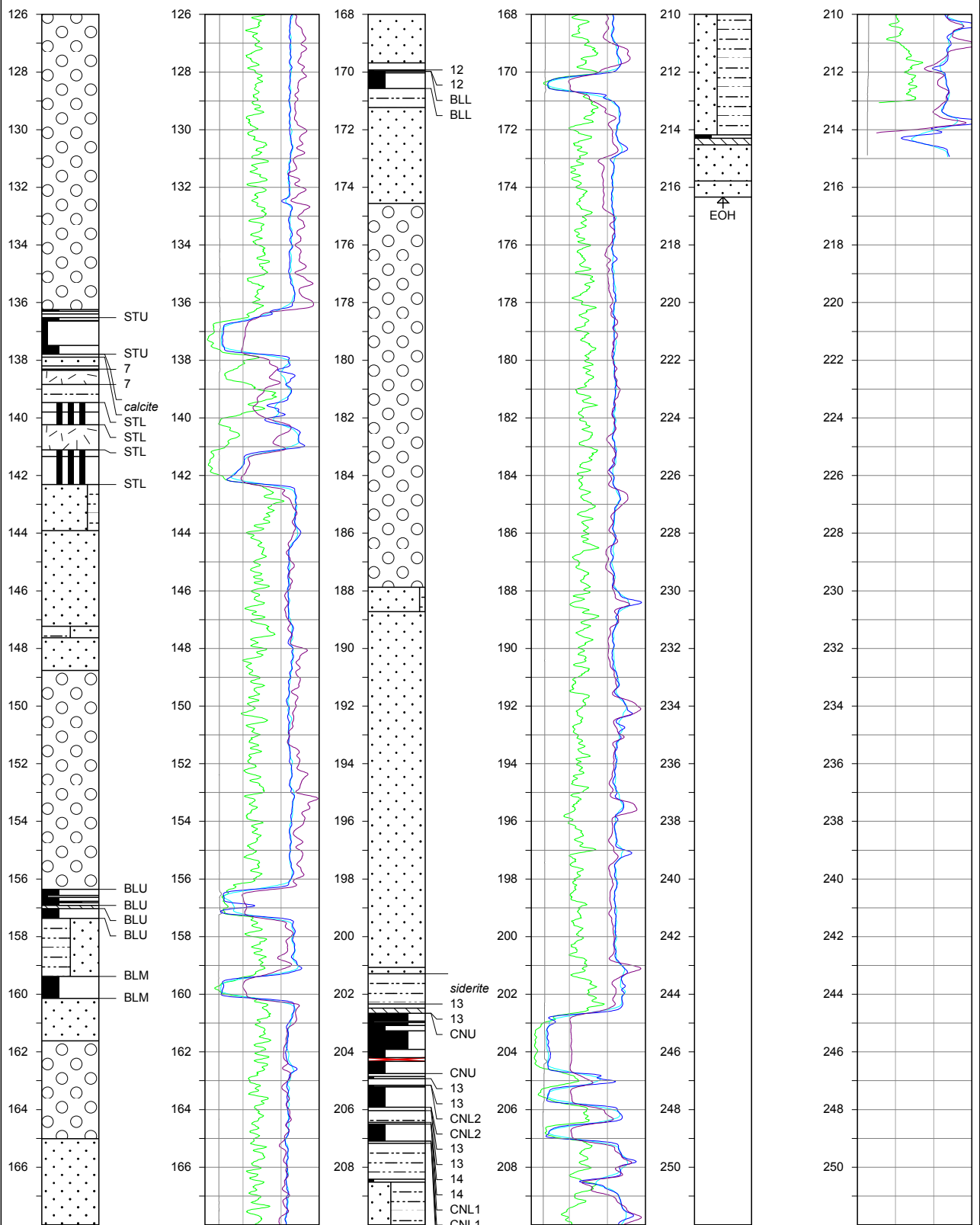
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1.0 DENB 3.0
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1.0 DENL 3.0
1.0 DENB 3.0
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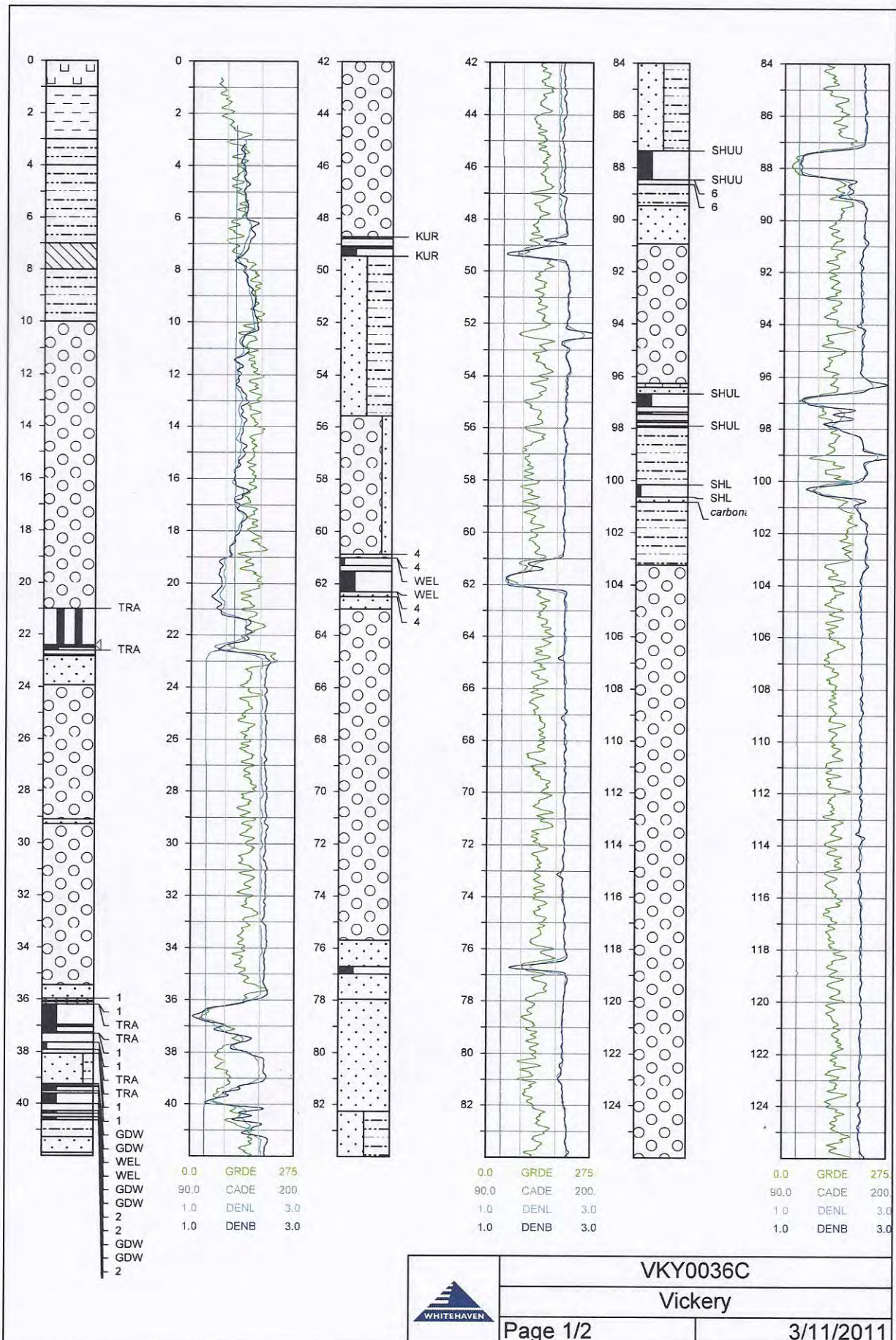


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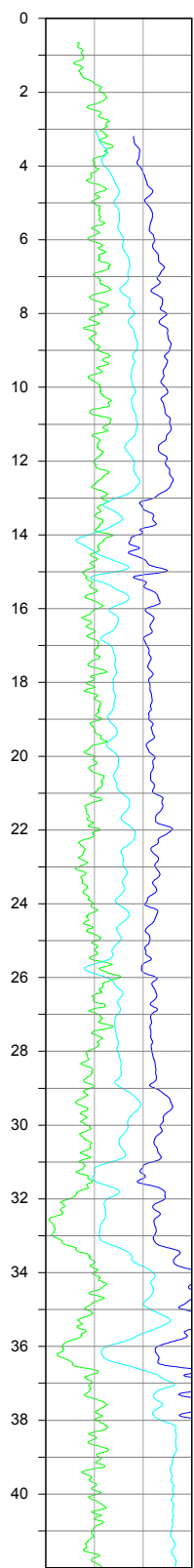
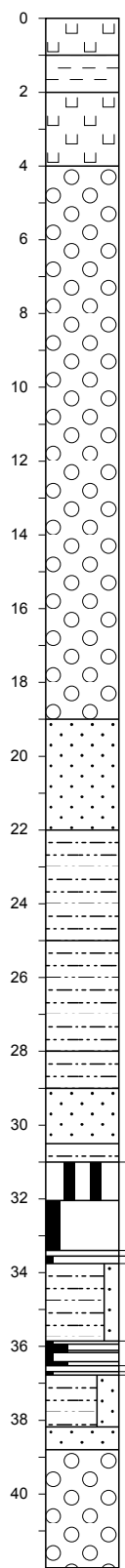
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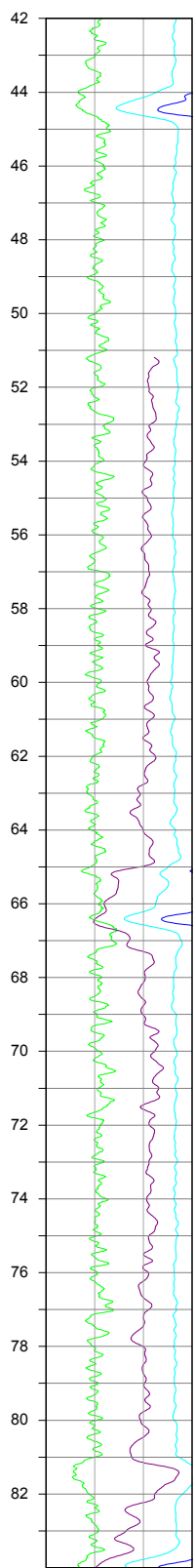
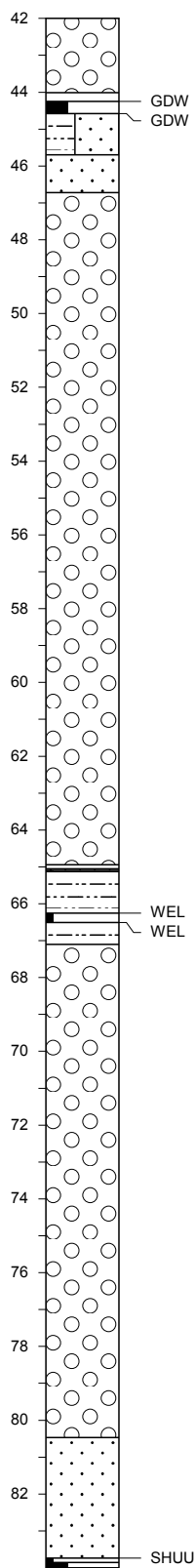


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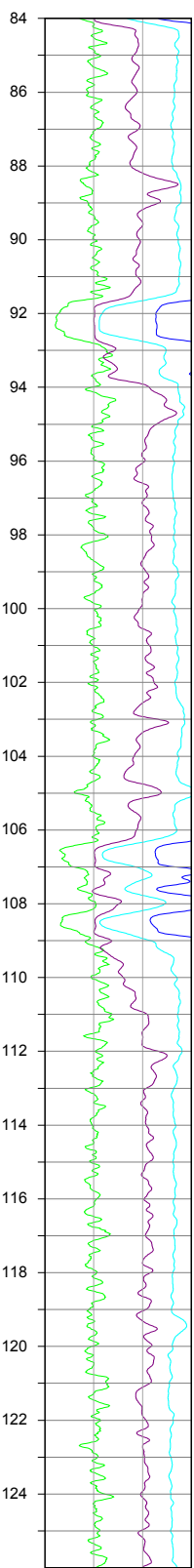
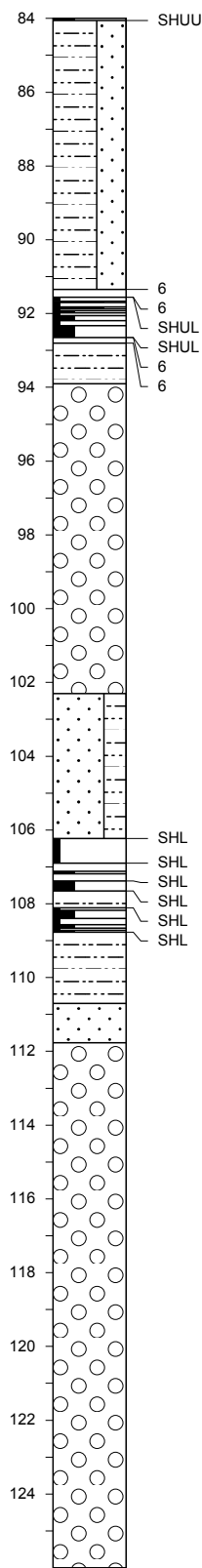
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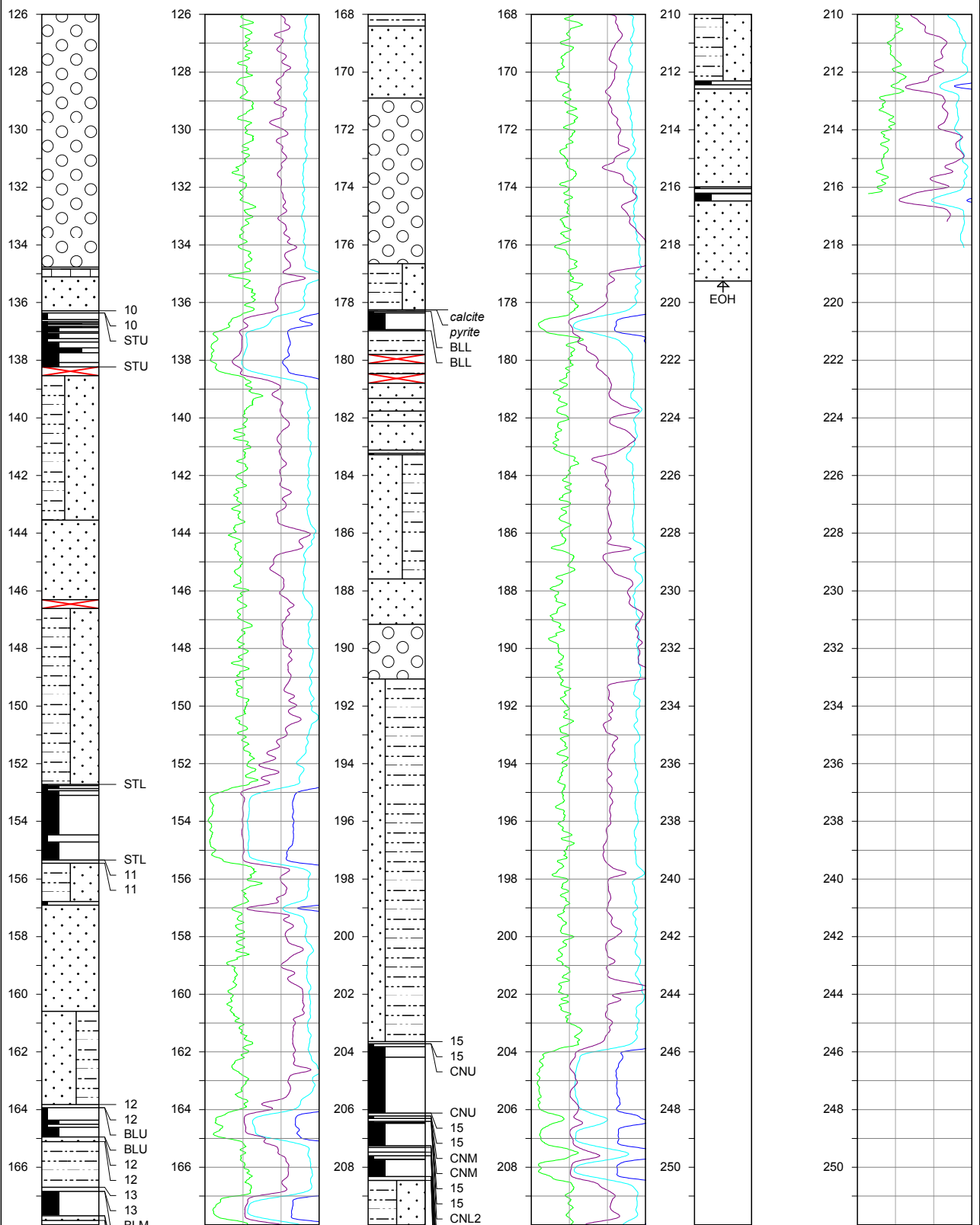
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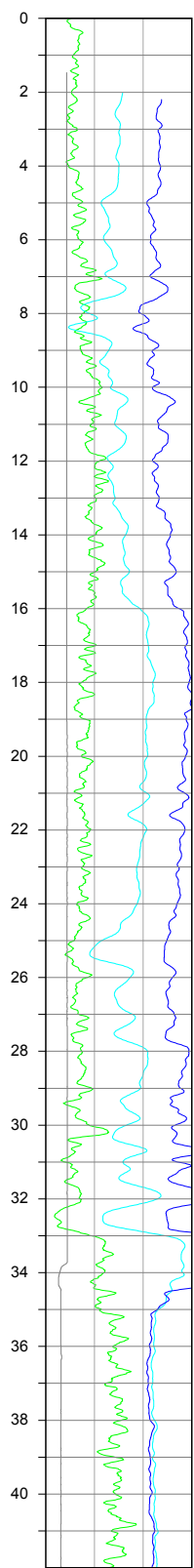
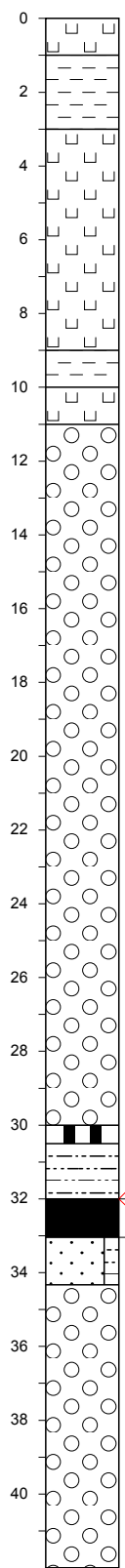
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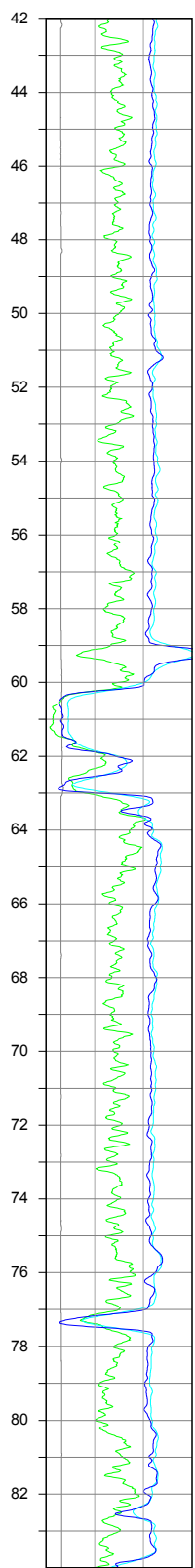
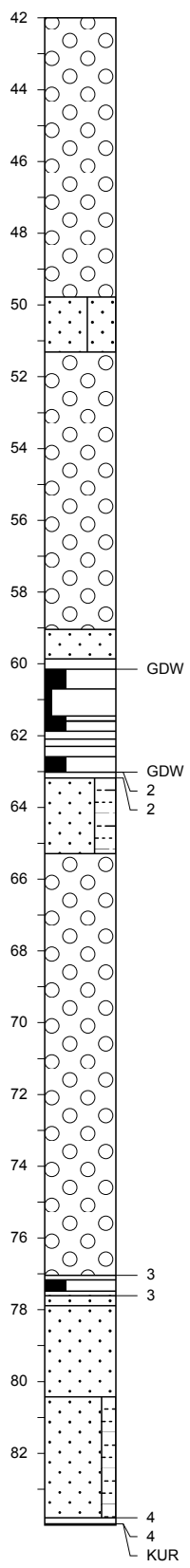


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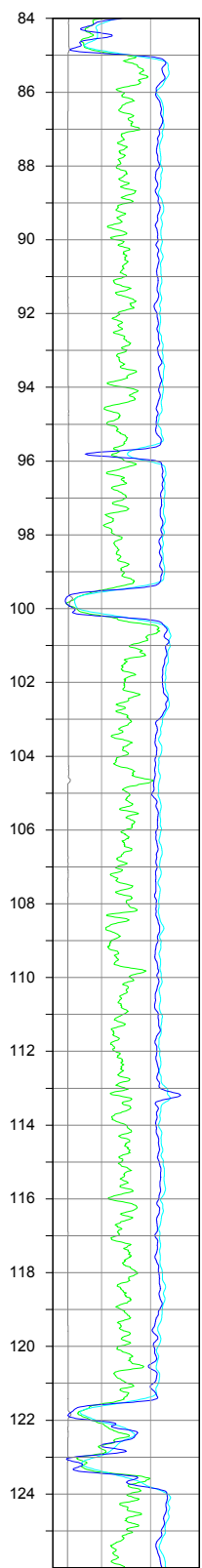
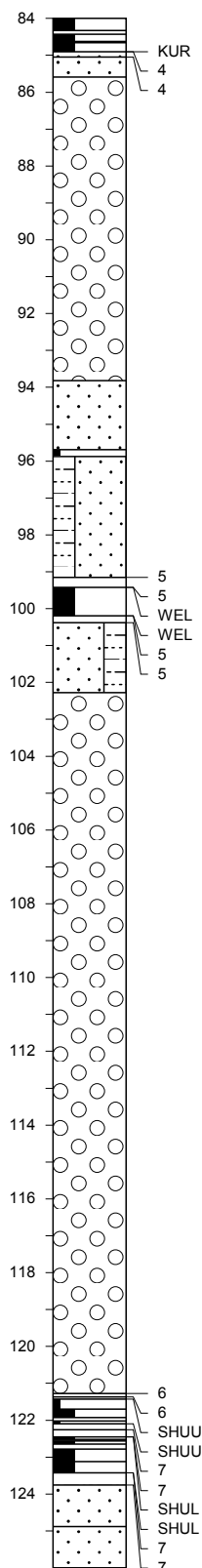
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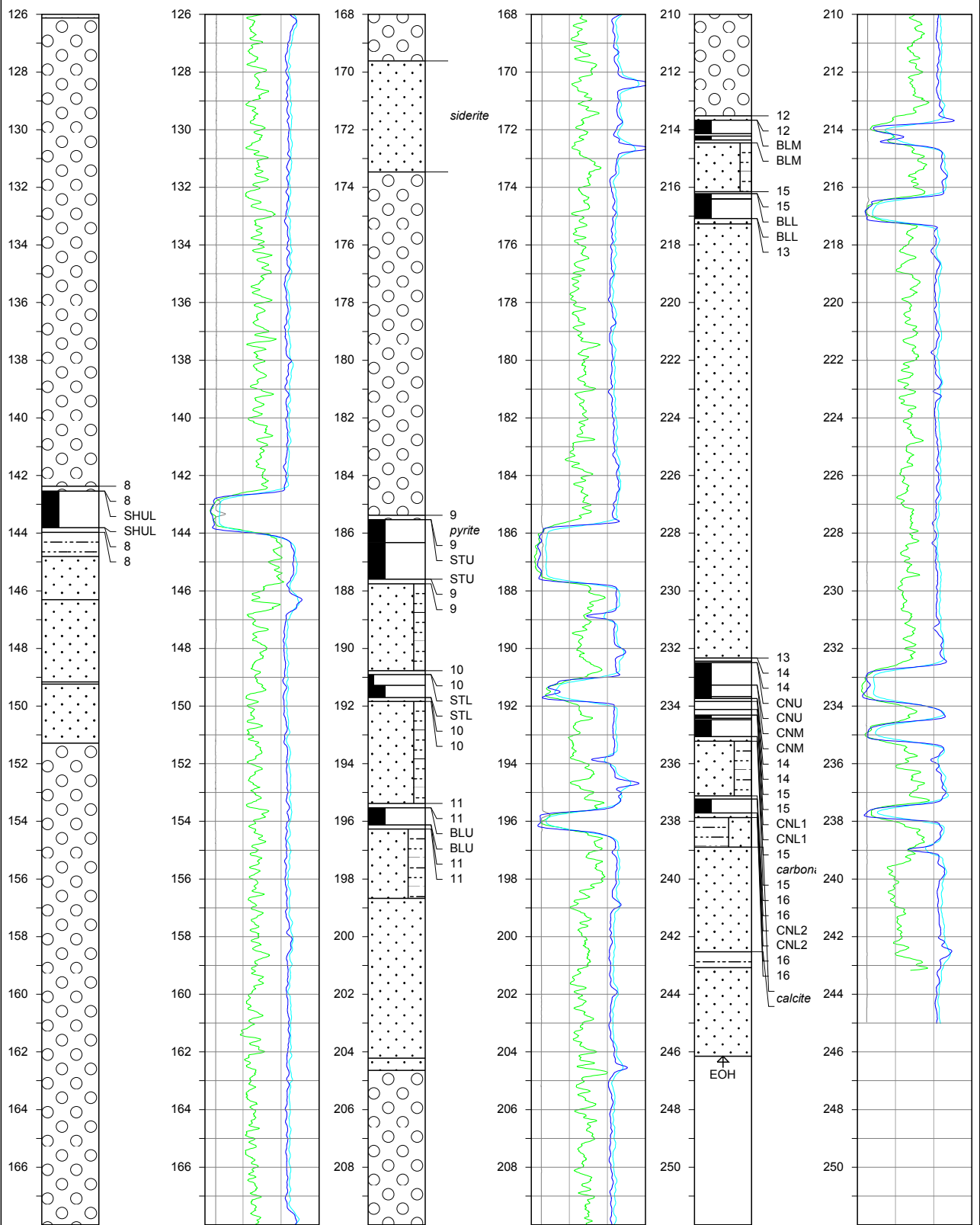
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 1.0 DENB 3.0



Vickery
 VKY043C



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 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275.
 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0



Driller's Licence No: **1**

Class of Licence:

Driller's Name:

Assistant Driller:

Contractor:

New bore Replacement bore

Deepened Enlarged

Reconditioned Other (specify)

Final Depth m

Work Licence No: **2**

Name of Licensee:

Intended Use:

Completion Date:

DRILLING DETAILS **3**

From (m)	To (m)	Hole Diameter (mm)	Drilling Method
0	201	203	See Code 3

WATER BEARING ZONES **4**

From (m)	To (m)	Thickness (m)	S W L (m)	Estimated Yield (L/s)		Test method	D D L at end of test (m)	Duration		Salinity (Conductivity or TDS)	
				Individual Aquifer	Cumulative			Hrs	min	Cond (µS/cm)	TDS (mg/L)
						See Code 4					

CASING / LINER DETAILS **5**

Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Method Fixing	Casing support method
Code 5	50	2	0	132	Code 5	See Code 5 2
						Type of casing bottom
						See Code 5 2
						Centralisers installed {Yes/No} No (indicate on sketch)
						Sump installed {Yes/No} Yes From 126 m To 129 m
						Pressure cemented {Yes/No} No From m To m
						Casing Protector cemented in place

WATER ENTRY DESIGN **6**

General							Screen	Slot Details		
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Opening type	Fixing	Aperture (mm)	Length (mm)	Width (mm)	Alignment
Code 5	50	2	126	132	See Code 6	See Code 5	1	150	1	See Code 6

GRAVEL PACK **7**

Type	Grade	Grain size (mm)		Depth (m)		Quantity	
		From	To	From	To	Litres	m ³
Rounded	Graded	3	5	118	136		
Crushed	Ungraded						
Bentonite/Grout seal (Yes/No)		Yes					
Method of placement of Gravel Pack		See Code 7		1			

For Departmental use only: **GW**

Work Licence No: 90BL256014

BORE DEVELOPMENT

8

Chemical used for breaking down drilling mud (Yes/No) No Name: _____

Method Bailing/Surging Jetting Airlifting Backwashing Pumping Other: _____

Duration _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs

DISINFECTION ON COMPLETION

9

Chemical(s) used	Quantity applied (Litres)	Method of application

PUMPING TESTS ON COMPLETION

10

Test type	Date	Pump intake depth (m)	Initial Water Level (SWL) (m)	Pumping rate (L/s)	Water Level at end of pumping (DDL) (m)	Duration of Test (hrs)	Recovery	
							Water level (m)	Time taken (hrs) (mins)
Multi stage (stepped drawdown)	Stage 1							
	Stage 2							
	Stage 3							
	Stage 4							
Single stage (constant rate)								
Height of measuring point above ground level _____ m		Test Method _____		See Code 4				

WORK PARTLY BACKFILLED OR ABANDONED

11

Original depth of work: 201 m Is work partly backfilled: (Yes/No) Yes

Is work abandoned: (Yes/No) No Method of abandonment: Backfilled Plugged Capped

Has any casing been left in the work (Yes/No) From 134 m To 201 m

Sealing / fill type	From depth (m)	To depth (m)	Sealing / fill type	From depth (m)	To depth (m)
See Code 11	132	134	See Code 11		
3			3		

Site chosen by: Hydrogeologist Geologist Driller Diviner Client Other _____

12

Lot No 2 DP No 1102940

Work Location Co ordinates Easting 232519 Northing 6593822 Zone 56

GPS: (Yes/No) Yes >> AMG/AGD or MGA/GDA (See explanation)

13

Please mark the work site with "X" on the CLID provided map.

Indicate also the distances in metres from two (2) adjacent boundaries, and attach the map to this Form A package.

Signatures:

Driller: _____

Licensee: _____

Date: _____

Date: _____



Driller's Licence No: **1**

Class of Licence:

Driller's Name:

Assistant Driller:

Contractor:

New bore Replacement bore

Deepened Enlarged

Reconditioned Other (specify)

Final Depth m

Work Licence No: **2**

Name of Licensee:

Intended Use:

Completion Date:

DRILLING DETAILS 3			
From (m)	To (m)	Hole Diameter (mm)	Drilling Method
0	214	96	See Code 3 13

WATER BEARING ZONES 4											
From (m)	To (m)	Thickness (m)	S W L (m)	Estimated Yield (L/s)		Test method See Code 4	D D L at end of test (m)	Duration		Salinity (Conductivity or TDS)	
				Individual Aquifer	Cumulative			Hrs	min	Cond (µS/cm)	TDS (mg/L)

CASING / LINER DETAILS 5									
Material Code 5	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Method Fixing Code 5	Casing support method See Code 5 2		Type of casing bottom See Code 5 2	
8	50	2	0	78	5	Centralisers installed {Yes/No}	No	(indicate on sketch)	
						Sump installed {Yes/No}	Yes	From 75 m To 78 m	
						Pressure cemented {Yes/No}	No	From m To m	
						Casing Protector cemented in place			

WATER ENTRY DESIGN 6										
General							Screen	Slot Details		
Material Code 5	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Opening type See Code 6	Fixing See Code 5	Aperture (mm)	Length (mm)	Width (mm)	Alignment See Code 6
8	50	2	72	75	5	5	1	150	1	V

GRAVEL PACK 7								
Type	Grade	Grain size (mm)		Depth (m)		Quantity		
		From	To	From	To	Litres	m ³	
Rounded	Graded	X	3	5	69	79		
Crushed	Ungraded							
Bentonite/Grout seal (Yes/No)		Yes			67	69		
Method of placement of Gravel Pack					See Code 7	1		

For Departmental use only:

Work Licence No: 90BL256013

BORE DEVELOPMENT

8

Chemical used for breaking down drilling mud (Yes/No) No Name: _____

Method Bailing/Surging Jetting Airlifting Backwashing Pumping Other: _____

Duration _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs

DISINFECTION ON COMPLETION

9

Chemical(s) used	Quantity applied (Litres)	Method of application

PUMPING TESTS ON COMPLETION

10

Test type	Date	Pump intake depth (m)	Initial Water Level (SWL) (m)	Pumping rate (L/s)	Water Level at end of pumping (DDL) (m)	Duration of Test (hrs)	Recovery	
							Water level (m)	Time taken (hrs) (mins)
Multi stage (stepped drawdown)	Stage 1							
	Stage 2							
	Stage 3							
	Stage 4							
Single stage (constant rate)								
Height of measuring point above ground level		0.47	m	Test Method			See Code 4	

WORK PARTLY BACKFILLED OR ABANDONED

11

Original depth of work: 216 m Is work partly backfilled: (Yes/No) Yes

Is work abandoned: (Yes/No) No Method of abandonment: Backfilled Plugged Capped

Has any casing been left in the work (Yes/No) Yes From 0 m To 75 m

Sealing / fill type	From depth (m)	To depth (m)	Sealing / fill type	From depth (m)	To depth (m)
See Code 11	79	81	See Code 11		
3					

Site chosen by: Hydrogeologist Geologist Driller Diviner Client Other _____

12

Lot No 2 DP No 1102940

Work Location Co ordinates Easting 232703.5 Northing 6593356.68 Zone 56

GPS: (Yes/No) Yes >> AMG/AGD or MGA/GDA (See explanation)

13

Please mark the work site with "X" on the CLID provided map.

Indicate also the distances in metres from two (2) adjacent boundaries, and attach the map to this Form A package.

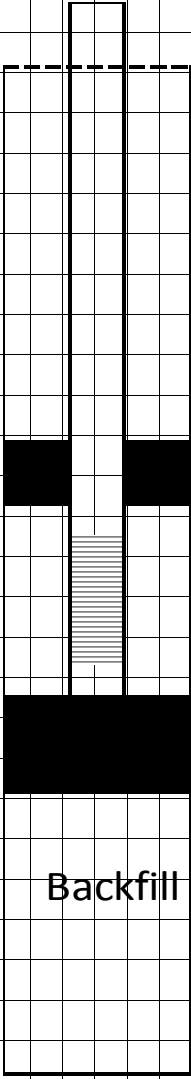
Signatures:

Driller: _____

Licensee: _____

Date: _____

Date: _____

DRILLER'S ROCK/STRATA DESCRIPTION (LITHOLOGY)			WORK CONSTRUCTION SKETCH
Depth		Description <div style="border: 1px solid black; padding: 2px;">See Code 15</div>	
From (m)	To (m)		
1	4	Soil / Clay minor sand	
4	7	Clay	
7	15	Weathered Siltstone	
15	16	Coal	
16	20	Weathered Siltstone	
20	21	Coal	
21	27.5	Conglomerate / Minor sandstone	
27.5	28	Coal	
28	66	Conglomerate	
66	67	Coal	
67	80	Conglomerate / Minor sandstone	
80	81	Coal	
81	95	Conglomerate / Minor sandstone	
95	99	Coal / Minor sandstone / siltstone	
99	136	Conglomerate / Minor sandstone	
136	142	Coal	
142	156	Sandstone / Conglomerate	
156	160	Coal	
160	170	Sandstone / Conglomerate	
170	171	Coal	
171	202	Sandstone / Conglomerate	
202	207	Coal	
207	216	Sandstone / Siltstone	
216		EOH	

WORK NOT CONSTRUCTED BY DRILLING RIG							
Method of excavation: Hand dug <input type="checkbox"/> Back hoe <input type="checkbox"/> Dragline <input type="checkbox"/> Dozer <input type="checkbox"/> Other <input type="text"/>							
Depth (m)	Length (m)	Width (m)	Diameter (m)	Lining material	Dimensions of liner (m)	From Depth (m)	To Depth (m)

Please attach copies of the following if available					
Geologist log	(Yes/No) <input type="checkbox"/>	Laboratory analysis of water Sample	(Yes/No) <input type="checkbox"/>	Pumping test(s)	(Yes/No) <input type="checkbox"/>
Geophysical log	(Yes/No) <input type="checkbox"/>	Sieve analysis of aquifer material	(Yes/No) <input type="checkbox"/>	Installed Pump details	(Yes/No) <input type="checkbox"/>



Driller's Licence No:

Class of Licence:

Driller's Name:

Assistant Driller:

Contractor:

New bore Replacement bore

Deepened Enlarged

Reconditioned Other (specify)

Final Depth m

Work Licence No:

Name of Licensee:

Intended Use:

Completion Date:

DRILLING DETAILS <input type="text" value="3"/>			
From (m)	To (m)	Hole Diameter (mm)	Drilling Method
0	214	96	See Code 3

WATER BEARING ZONES <input type="text" value="4"/>											
From (m)	To (m)	Thickness (m)	S W L (m)	Estimated Yield (L/s)		Test method	D D L at end of test (m)	Duration		Salinity (Conductivity or TDS)	
				Individual Aquifer	Cumulative			Hrs	min	Cond (µS/cm)	TDS (mg/L)
						See Code 4					

CASING / LINER DETAILS <input type="text" value="5"/>										
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Method Fixing	Casing support method				
Code 5					Code 5	See Code 5 <input type="text" value="2"/>				
						Type of casing bottom				
8	50	5	0	114	5	See Code 5 <input type="text" value="2"/>				
						Centralisers installed {Yes/No}	No	(indicate on sketch)		
						Sump installed {Yes/No}	Yes	From 75 m	To 78 m	
						Pressure cemented {Yes/No}	No	From	To	
						Casing Protector cemented in place				

WATER ENTRY DESIGN <input type="text" value="6"/>										
General						Screen	Slot Details			
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Opening type	Fixing	Aperture (mm)	Length (mm)	Width (mm)	Alignment
Code 5					See Code 6	See Code 5				See Code 6
8	50	2.5	114	111	5	1	1	150	1	V

GRAVEL PACK <input type="text" value="7"/>								
Type	Grade	Grain size (mm)		Depth (m)		Quantity		
		From	To	From	To	Litres	m ³	
Rounded	Graded	X	3	5	107	117		
Crushed	Ungraded							
Bentonite/Grout seal (Yes/No)		Yes			105	107		
Method of placement of Gravel Pack		See Code 7		1				

For Departmental use only:

Work Licence No: 90BL256015

BORE DEVELOPMENT

8

Chemical used for breaking down drilling mud (Yes/No) No Name: _____

Method Bailing/Surging Jetting Airlifting Backwashing Pumping Other: _____

Duration _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs

DISINFECTION ON COMPLETION

9

Chemical(s) used	Quantity applied (Litres)	Method of application

PUMPING TESTS ON COMPLETION

10

Test type	Date	Pump intake depth (m)	Initial Water Level (SWL) (m)	Pumping rate (L/s)	Water Level at end of pumping (DDL) (m)	Duration of Test (hrs)	Recovery	
							Water level (m)	Time taken (hrs) (mins)
Multi stage (stepped drawdown)	Stage 1							
	Stage 2							
	Stage 3							
	Stage 4							
Single stage (constant rate)								
Height of measuring point above ground level		0.65	m	Test Method			See Code 4	

WORK PARTLY BACKFILLED OR ABANDONED

11

Original depth of work: 226 m Is work partly backfilled: (Yes/No) Yes

Is work abandoned: (Yes/No) No Method of abandonment: Backfilled Plugged Capped

Has any casing been left in the work (Yes/No) Yes From 0 m To 114 m

Sealing / fill type	From depth (m)	To depth (m)	Sealing / fill type	From depth (m)	To depth (m)
See Code 11			See Code 11		
3	117	119	8		

Site chosen by: Hydrogeologist Geologist Driller Diviner Client Other _____

12

Lot No 2 DP No 1102940

Work Location Co ordinates Easting 233120 Northing 6592922 Zone 56

GPS: (Yes/No) Yes >> AMG/AGD or MGA/GDA (See explanation)

13

Please mark the work site with "X" on the CLID provided map.

Indicate also the distances in metres from two (2) adjacent boundaries, and attach the map to this Form A package.

Signatures:

Driller: _____

Licensee: _____

Date: _____

Date: _____



Driller's Licence No: DL1500 1
 Class of Licence:
 Driller's Name: Jason Manion
 Assistant Driller:
 Contractor: Manion Drilling
 New bore Replacement bore
 Deepened Enlarged
 Reconditioned Other (specify)
 Final Depth 242 m

Work Licence No: 90BL256010 2
 Name of Licensee: Whitehaven Coal Mine Ltd
 Intended Use: Vibrating Wire Piezometer
 Completion Date: 18/02/2012

DRILLING DETAILS 3			
From (m)	To (m)	Hole Diameter (mm)	Drilling Method
0	30	150	See Code 3
30	242	96	10
			13

WATER BEARING ZONES 4											
From (m)	To (m)	Thickness (m)	S W L (m)	Estimated Yield (L/s)		Test method	D D L at end of test (m)	Duration		Salinity (Conductivity or TDS)	
				Individual Aquifer	Cumulative			Hrs	min	Cond (µS/cm)	TDS (mg/L)
						See Code 4					

CASING / LINER DETAILS 5											
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Method Fixing	Casing support method					
Code 5					Code 5	See Code 5 1					
						Type of casing bottom					
						See Code 5 1					
						Centralisers installed {Yes/No}	No	(indicate on sketch)			
						Sump installed {Yes/No}	No	From		m To	
						Pressure cemented {Yes/No}	No	From		m To	
						Casing Protector cemented in place					

WATER ENTRY DESIGN 6										
General						Screen	Slot Details			
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Opening type	Fixing	Aperture (mm)	Length (mm)	Width (mm)	Alignment
Code 5					See Code 6	See Code 5				See Code 6

GRAVEL PACK 7								
Type	Grade	Grain size (mm)		Depth (m)		Quantity		
		From	To	From	To	Litres	m ³	
Rounded	Graded				0			
Crushed	Ungraded							
Bentonite/Grout seal (Yes/No)		Yes		0	242		2	
Method of placement of Gravel Pack		See Code 7						

For Departmental use only: G W

Work Licence No: 90BL256010

BORE DEVELOPMENT

8

Chemical used for breaking down drilling mud (Yes/No) No Name: _____

Method Bailing/Surging Jetting Airlifting Backwashing Pumping Other: _____

Duration _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs

DISINFECTION ON COMPLETION

9

Chemical(s) used	Quantity applied (Litres)	Method of application

PUMPING TESTS ON COMPLETION

10

Test type	Date	Pump intake depth (m)	Initial Water Level (SWL) (m)	Pumping rate (L/s)	Water Level at end of pumping (DDL) (m)	Duration of Test (hrs)	Recovery	
							Water level (m)	Time taken (hrs) (mins)
Multi stage (stepped drawdown)	Stage 1							
	Stage 2							
	Stage 3							
	Stage 4							
Single stage (constant rate)								
Height of measuring point above ground level _____ m		Test Method _____		See Code 4				

WORK PARTLY BACKFILLED OR ABANDONED

11

Original depth of work: 242 m Is work partly backfilled: (Yes/No) Yes

Is work abandoned: (Yes/No) No Method of abandonment: Backfilled Plugged Capped

Has any casing been left in the work (Yes/No) No From _____ m To _____ m

Sealing / fill type	From depth (m)	To depth (m)	Sealing / fill type	From depth (m)	To depth (m)
See Code 11	0	242	See Code 11		
1					

Site chosen by: Hydrogeologist Geologist Driller Diviner Client Other _____

12

Lot No 2 DP No 1102940

Work Location Co ordinates Easting 233397.62 Northing 6591348.76 Zone 56

GPS: (Yes/No) Yes >> AMG/AGD or MGA/GDA (See explanation)

13

Please mark the work site with "X" on the CLID provided map.

Indicate also the distances in metres from two (2) adjacent boundaries, and attach the map to this Form A package.

Signatures:

Driller: _____

Licensee: _____

Date: _____

Date: _____



Driller's Licence No: DL1500 **1**

Class of Licence:

Driller's Name: Jason Manion

Assistant Driller:

Contractor: Manion Drilling

New bore Replacement bore

Deepened Enlarged

Reconditioned Other (specify)

Final Depth 246 m

Work Licence No: 90BL256011 **2**

Name of Licensee: Whitehaven Coal Mine Ltd

Intended Use: Monitoring

Completion Date: 20/12/2012

DRILLING DETAILS 3			
From (m)	To (m)	Hole Diameter (mm)	Drilling Method
0	48	150	See Code 3
48	246	96	10
			13

WATER BEARING ZONES 4											
From (m)	To (m)	Thickness (m)	S W L (m)	Estimated Yield (L/s)		Test method	D D L at end of test (m)	Duration		Salinity (Conductivity or TDS)	
				Individual Aquifer	Cumulative			Hrs	min	Cond (µS/cm)	TDS (mg/L)
						See Code 4					

CASING / LINER DETAILS 5										
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Method Fixing	Casing support method				
Code 5	50	2	0	230	Code 5	See Code 5 2				
Type of casing bottom						See Code 5 2				
Centralisers installed {Yes/No}						No	(indicate on sketch)			
Sump installed {Yes/No}						Yes	From	246 m	To	246 m
Pressure cemented {Yes/No}						No	From		To	
Casing Protector cemented in place										

WATER ENTRY DESIGN 6										
General							Screen	Slot Details		
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Opening type	Fixing	Aperture (mm)	Length (mm)	Width (mm)	Alignment
Code 5	50	2	234	243	See Code 6	See Code 5	1	150	1	See Code 6
6	50	2	234	243	3	1	1	150	1	V

GRAVEL PACK 7								
Type	Grade	Grain size (mm)		Depth (m)		Quantity		
		From	To	From	To	Litres	m ³	
Rounded	Graded	X	3	5	230	246		
Crushed	Ungraded							
Bentonite/Grout seal (Yes/No)		Yes			228	230		
Method of placement of Gravel Pack		See Code 7		1				

For Departmental use only: **GW**

Work Licence No: 90BL256011

BORE DEVELOPMENT

8

Chemical used for breaking down drilling mud (Yes/No) No Name: _____

Method Bailing/Surging Jetting Airlifting Backwashing Pumping Other: _____

Duration _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs

DISINFECTION ON COMPLETION

9

Chemical(s) used	Quantity applied (Litres)	Method of application

PUMPING TESTS ON COMPLETION

10

Test type	Date	Pump intake depth (m)	Initial Water Level (SWL) (m)	Pumping rate (L/s)	Water Level at end of pumping (DDL) (m)	Duration of Test (hrs)	Recovery	
							Water level (m)	Time taken (hrs) (mins)
Multi stage (stepped drawdown)	Stage 1							
	Stage 2							
	Stage 3							
	Stage 4							
Single stage (constant rate)								
Height of measuring point above ground level _____ m		Test Method _____		See Code 4				

WORK PARTLY BACKFILLED OR ABANDONED

11

Original depth of work: 246 m Is work partly backfilled: (Yes/No) No

Is work abandoned: (Yes/No) No Method of abandonment: Backfilled Plugged Capped

Has any casing been left in the work (Yes/No) No From _____ m To _____ m

Sealing / fill type	From depth (m)	To depth (m)	Sealing / fill type	From depth (m)	To depth (m)
See Code 11			See Code 11		
3		134			

Site chosen by: Hydrogeologist Geologist Driller Diviner Client Other _____

12

Lot No 2 DP No 1102940

Work Location Co ordinates Easting 233250.25 Northing 6590533.37 Zone 56

GPS: (Yes/No) Yes >> AMG/AGD or MGA/GDA (See explanation)

13

Please mark the work site with "X" on the CLID provided map.

Indicate also the distances in metres from two (2) adjacent boundaries, and attach the map to this Form A package.

Signatures:

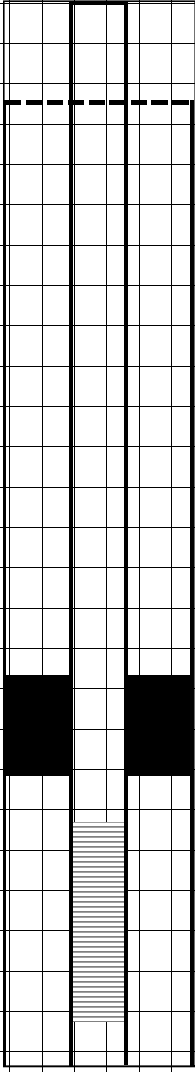
Driller: _____

Licensee: _____

Date: _____

Date: _____

DRILLER'S ROCK/STRATA DESCRIPTION (LITHOLOGY)			WORK CONSTRUCTION SKETCH
Depth		Description <div style="border: 1px solid black; padding: 2px;">See Code 15</div>	
From (m)	To (m)		
0	1	Soil	
1	3	Clay minor sand	
4	30	Weathered Conglomerate / Sandstone	
11	30	Conglomerate / Minor sandstone	
30	34	Coal / Minor Sandstone	
34	59	Conglomerate / Minor sandstone	
59	60	Sandstone	
60	63	Coal	
69	65	Sandstone	
65	77	Conglomerate / Minor sandstone	
77	77.5	Coal	
77.5	84	Sandstone / Conglomerate	
84	85	Coal	
85	99	Sandstone / Conglomerate	
99	100	Coal	
100	121	Conglomerate / Minor sandstone	
121	123	Coal	
123	142.5	Sandstone / Conglomerate	
142.5	144	Coal	
144	151	Sandstone	
151	185.5	Conglomerate / Minor sandstone	
185.5	187	Coal	
187	204	Sandstone / Coal	
204	213.5	Conglomerate / Minor sandstone	
213.5	217	Sandstone / Coal	
217	232	Sandstone	
232	238	Sandstone / Coal	
238	246	Sandstone	



15

WORK NOT CONSTRUCTED BY DRILLING RIG							
Method of excavation: Hand dug <input type="checkbox"/> Back hoe <input type="checkbox"/> Dragline <input type="checkbox"/> Dozer <input type="checkbox"/> Other <input type="text"/>							
Depth (m)	Length (m)	Width (m)	Diameter (m)	Lining material	Dimensions of liner (m)	From Depth (m)	To Depth (m)

16

Please attach copies of the following if available					
Geologist log	(Yes/No) <input type="checkbox"/>	Laboratory analysis of water Sample	(Yes/No) <input type="checkbox"/>	Pumping test(s)	(Yes/No) <input type="checkbox"/>
Geophysical log	(Yes/No) <input type="checkbox"/>	Sieve analysis of aquifer material	(Yes/No) <input type="checkbox"/>	Installed Pump details	(Yes/No) <input type="checkbox"/>

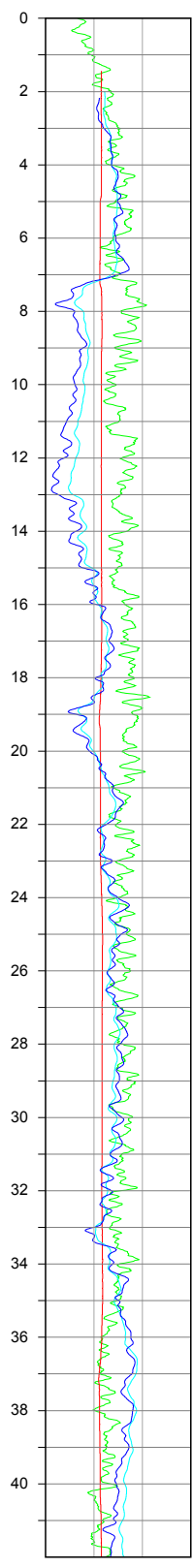
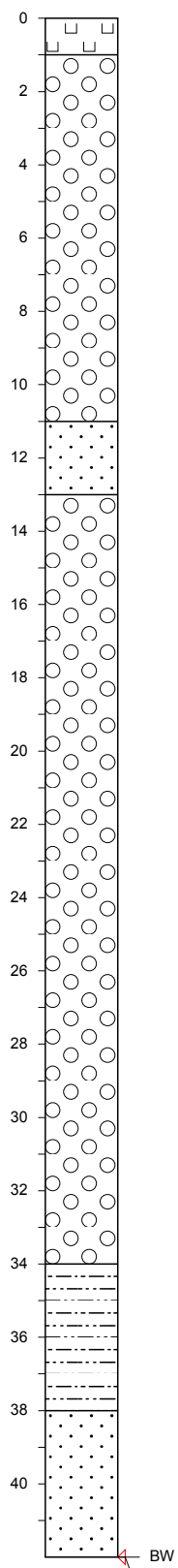
17

APPENDIX B

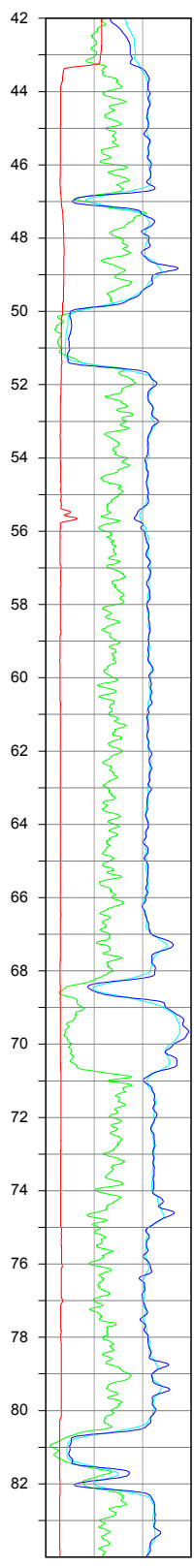
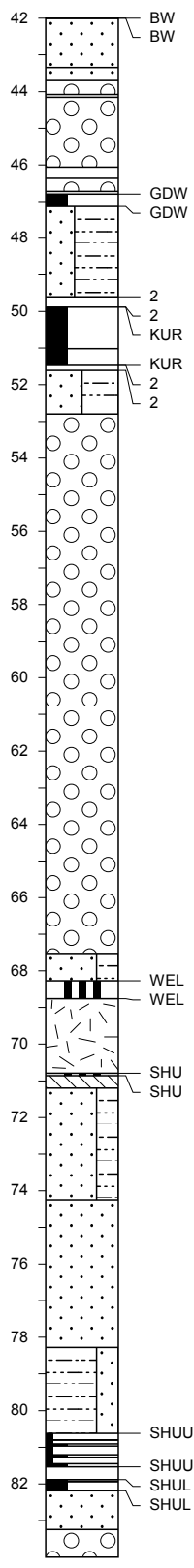
VIBRATING WIRE PIEZOMETER BORE LOGS AND LICENCE DETAILS

Table B1: Vibrating Wire Piezometer Drillhole Details

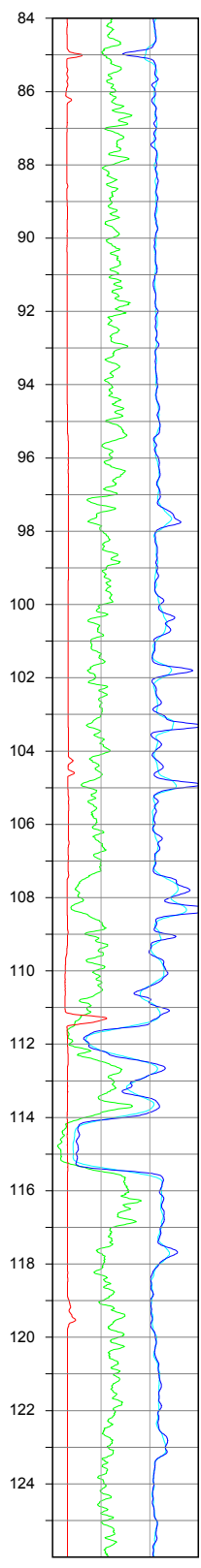
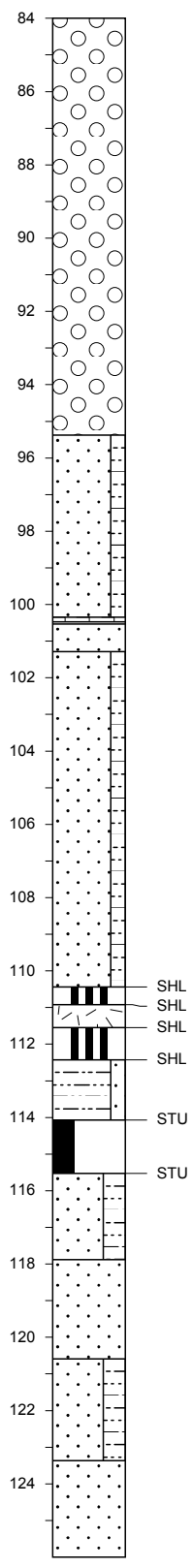
Hole ID	Easting (MGA)	Northing (MGA)	Bore Depth (m)
VKY3033	232366	6594263	258
VKY3041	233397	6591348	338
VKY3053	230098	6593816	90



0.0 GRDE 275.
 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0



0.0 GRDE 275.
 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0

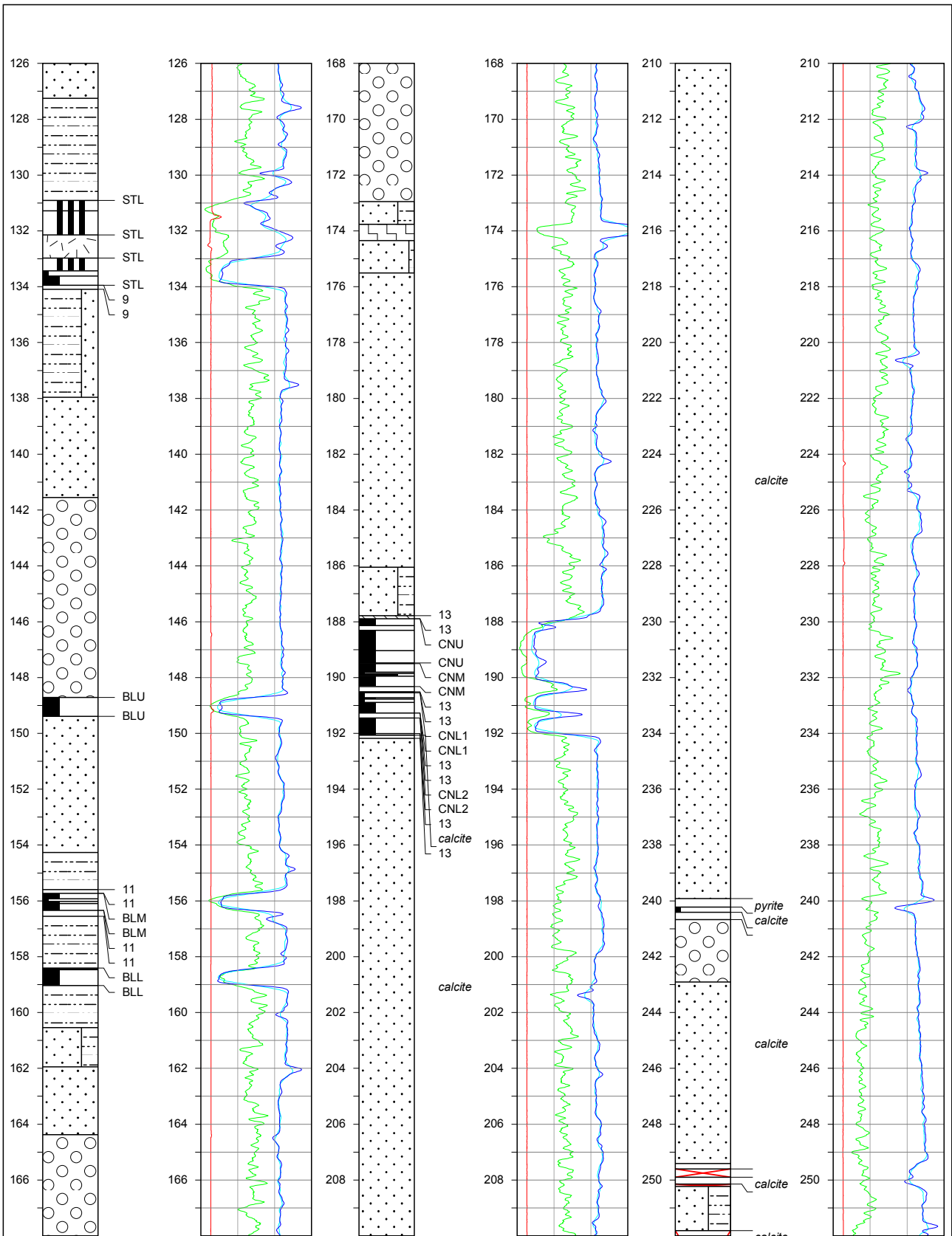


0.0 GRDE 275.
 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0



VKY033C

Vickery



0.0 GRDE 275.
 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0

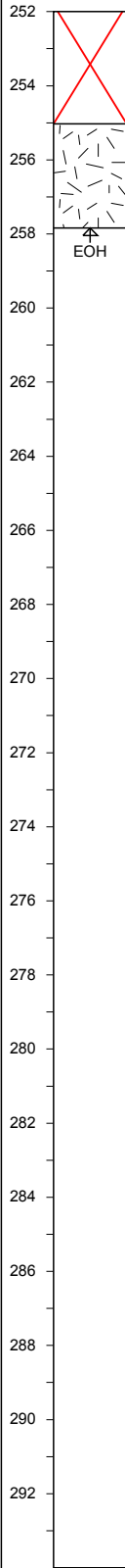
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 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275.
 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0

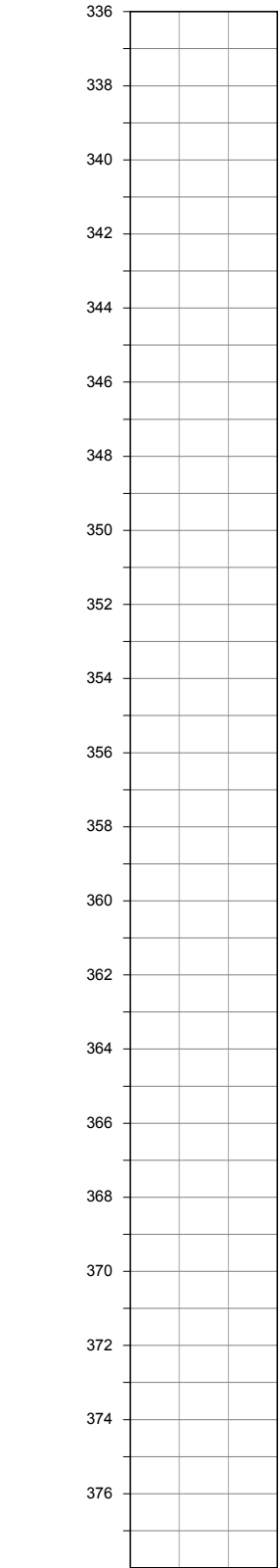
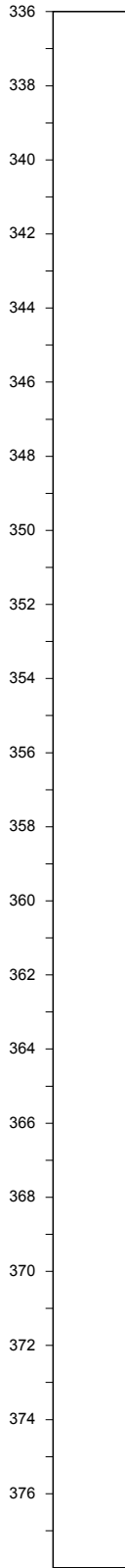
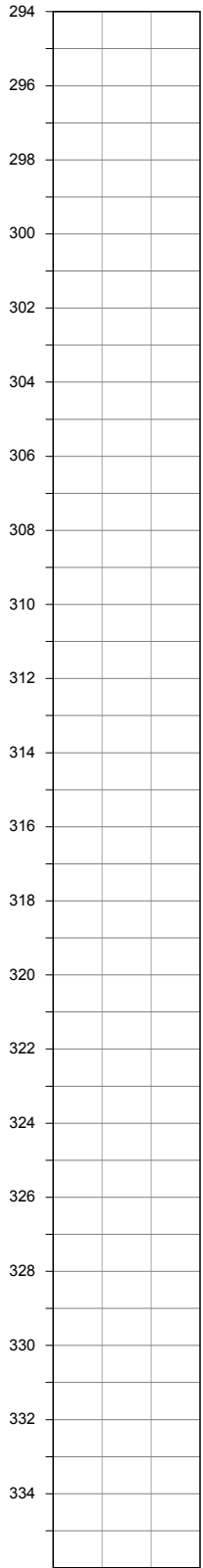
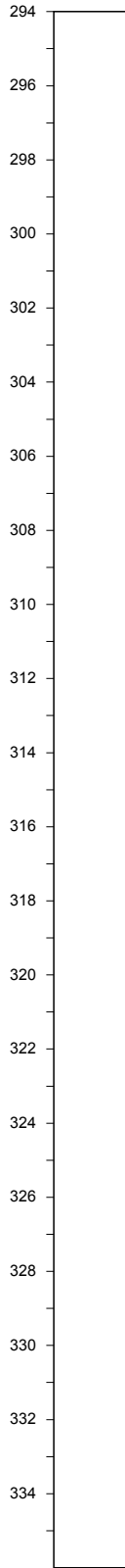
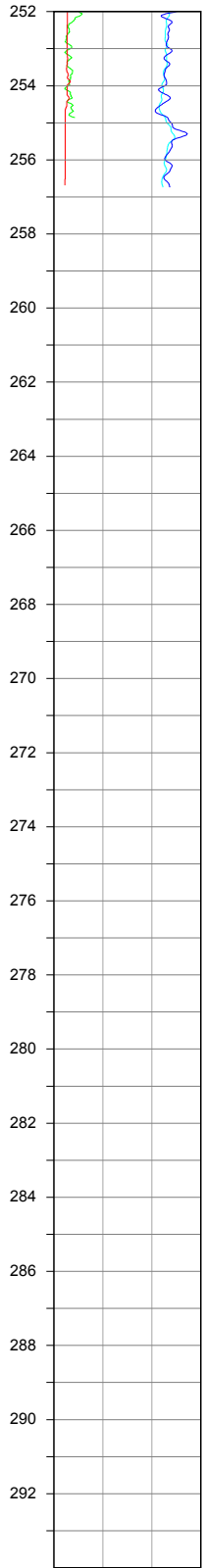


VKY033C

Vickery



calcite



0.0 GRDE 275.
 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0

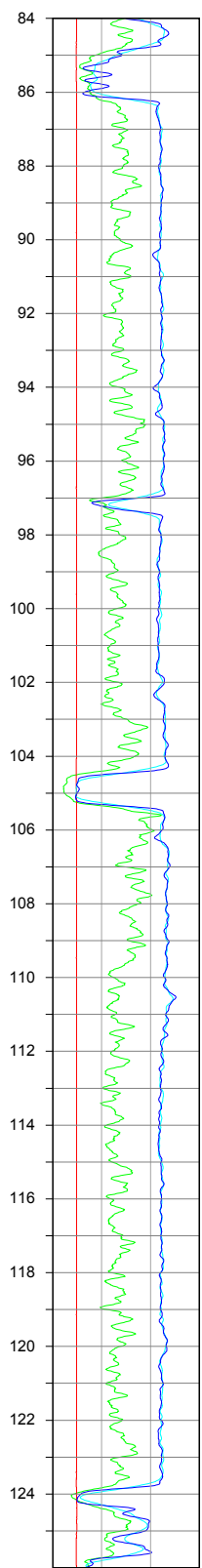
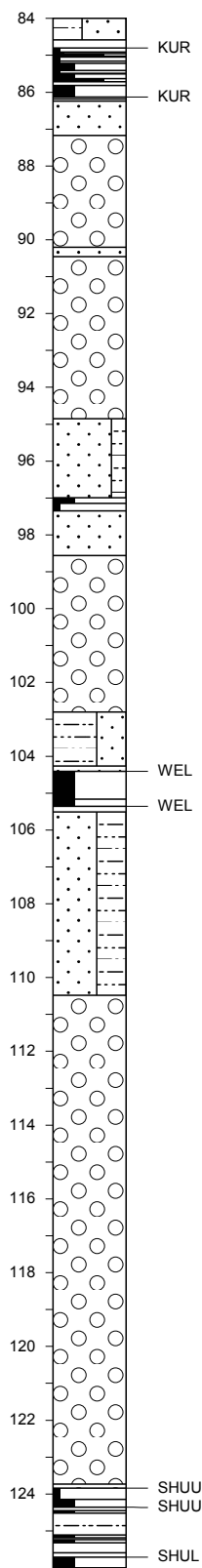
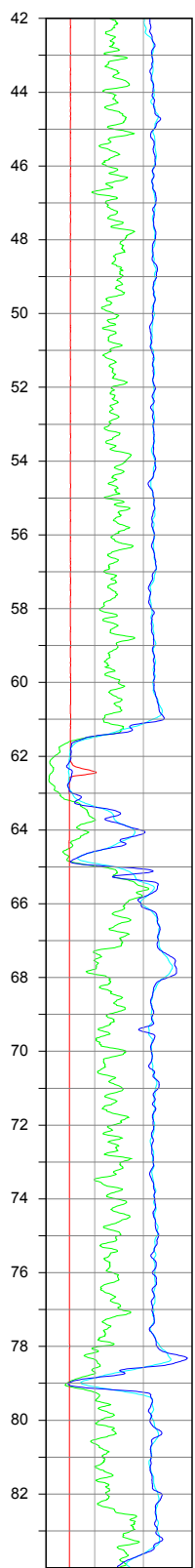
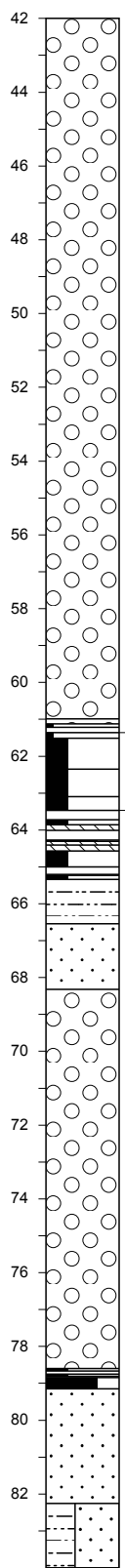
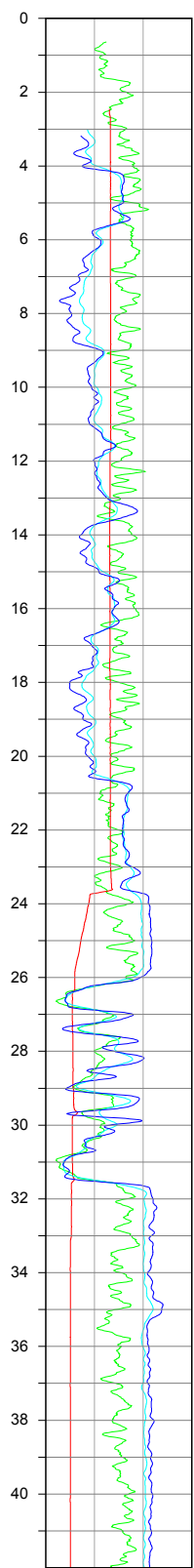
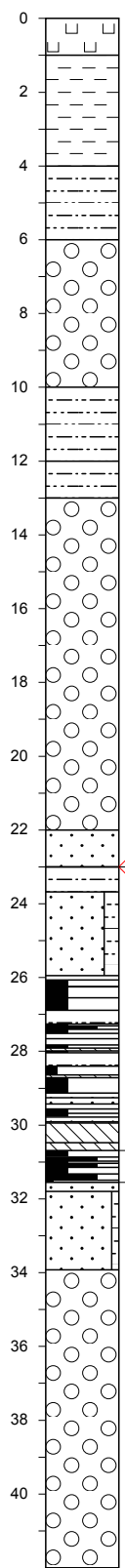
0.0 GRDE 275.
 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275.
 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0



VKY033C

Vickery



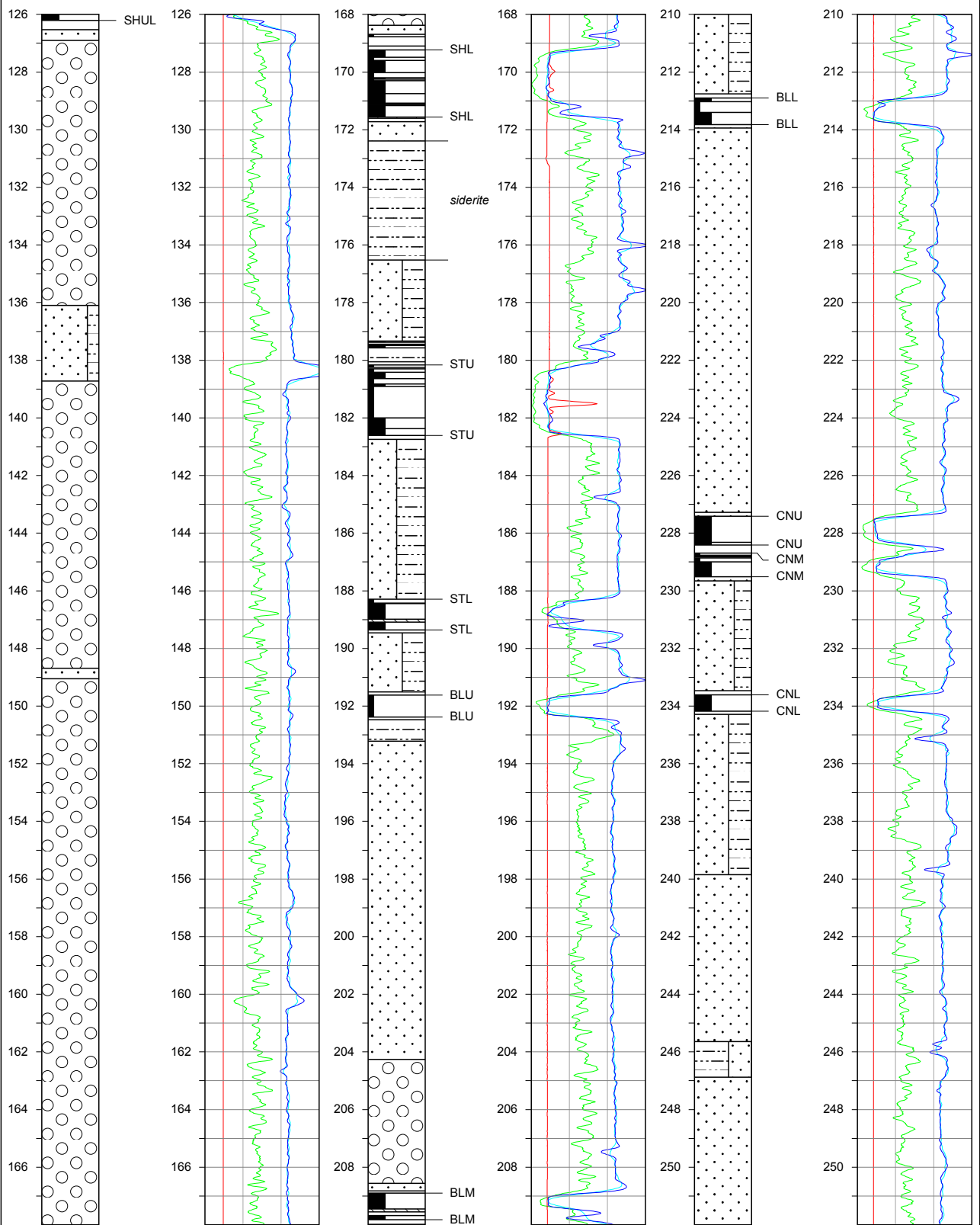
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 1.0 DENB 3.0

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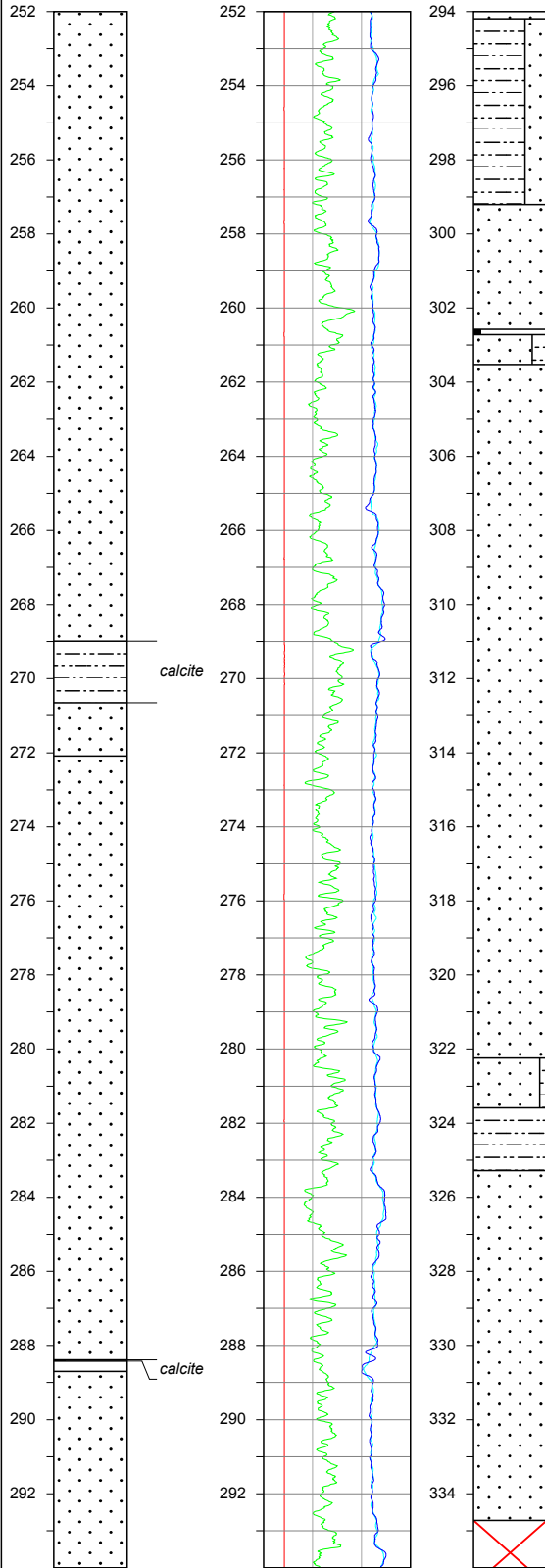
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 1.0 DENB 3.0



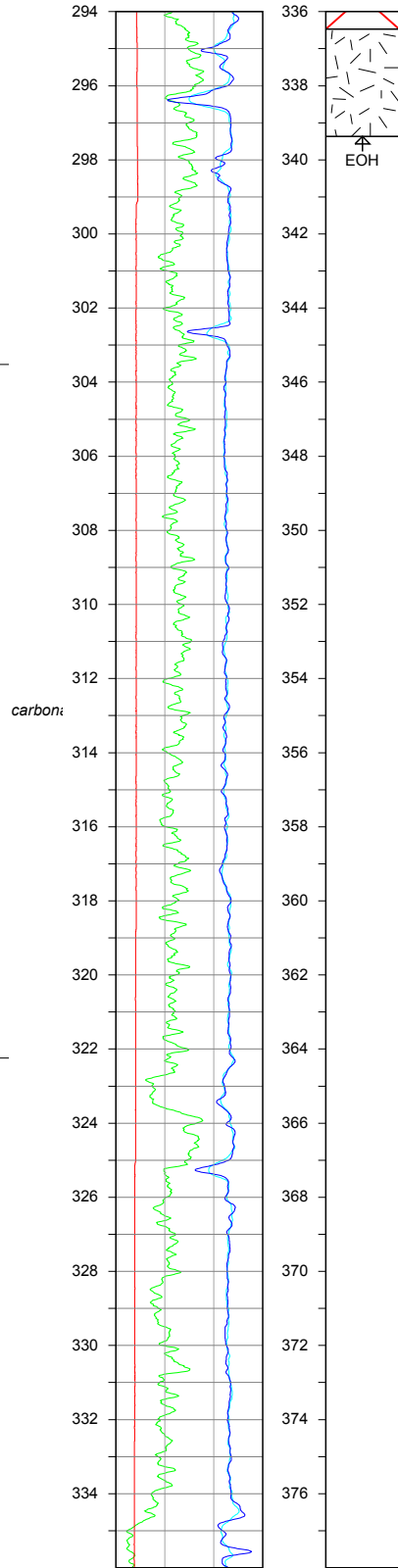
VICKERY
 VKY0041C



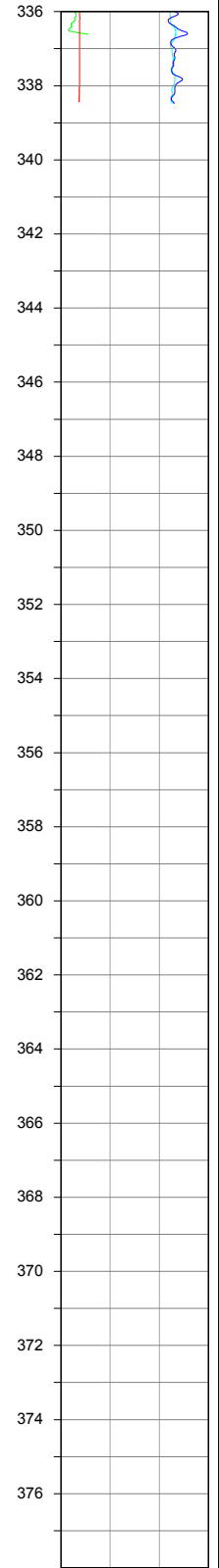
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1.0	DENB	3.0



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 1.0 DENL 3.0
 1.0 DENB 3.0



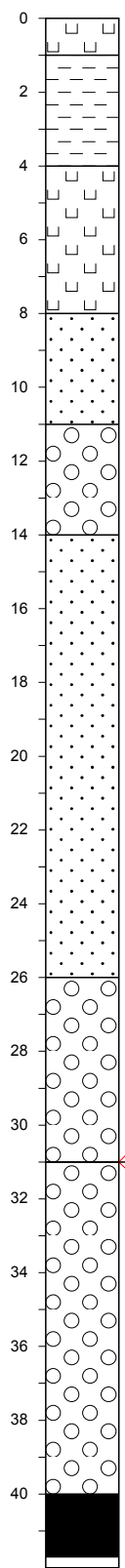
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 1.0 DENB 3.0



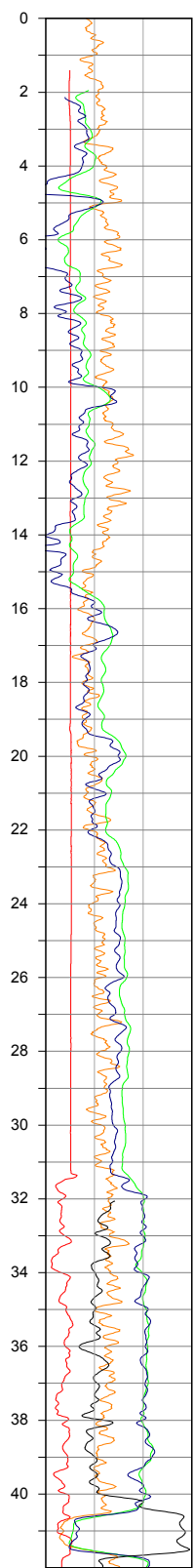
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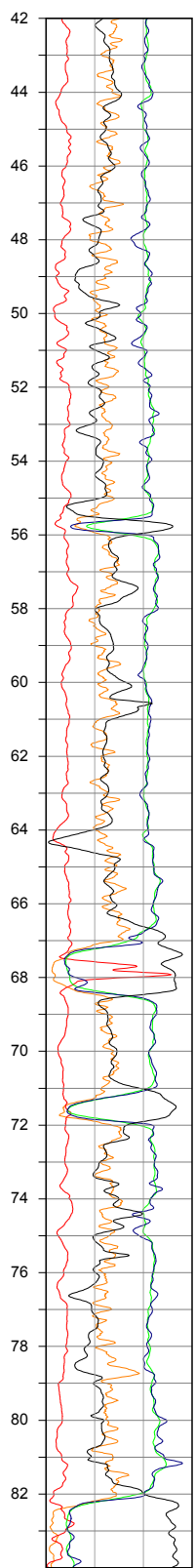
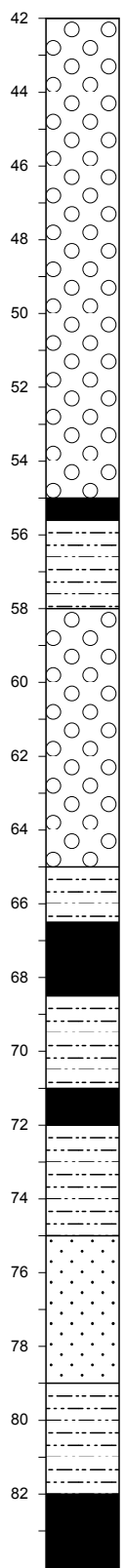
VICKERY
 VKY0041C



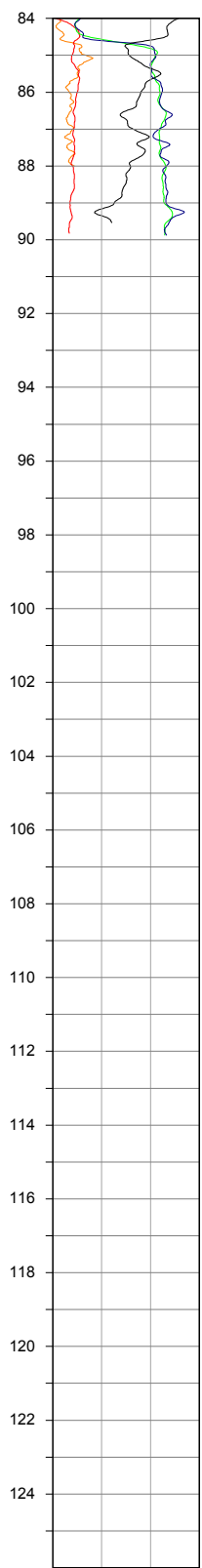
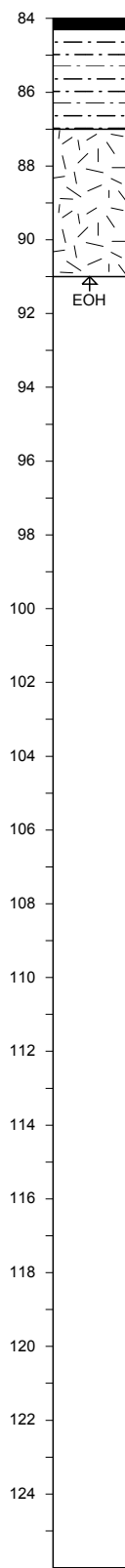
BW
BW



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1.0 DENL 3.0
1.0 DENB 3.0
164.5 MC2A 467.



0.0 GRDE 275.
115.6 CADE 213.
1.0 DENL 3.0
1.0 DENB 3.0
164.5 MC2A 467.



0.0 GRDE 275.
115.6 CADE 213.
1.0 DENL 3.0
1.0 DENB 3.0
164.5 MC2A 467.



Driller's Licence No: DL1500 1
 Class of Licence:
 Driller's Name: Jason Manion
 Assistant Driller:
 Contractor: Manion Drilling
 New bore Replacement bore
 Deepened Enlarged
 Reconditioned Other (specify)
 Final Depth 258 m

Work Licence No: 90BL256012 2
 Name of Licensee: Whitehaven Coal Mine Ltd
 Intended Use: Vibrating Wire Piezometer
 Completion Date: 19/02/2012

DRILLING DETAILS 3			
From (m)	To (m)	Hole Diameter (mm)	Drilling Method
0	31	150	See Code 3
31	258	96	10
			13

WATER BEARING ZONES 4											
From (m)	To (m)	Thickness (m)	S W L (m)	Estimated Yield (L/s)		Test method	D D L at end of test (m)	Duration		Salinity (Conductivity or TDS)	
				Individual Aquifer	Cumulative			Hrs	min	Cond (µS/cm)	TDS (mg/L)
						See Code 4					

CASING / LINER DETAILS 5										
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Method Fixing	Casing support method		See Code 5 1		
Code 5					Code 5	Type of casing bottom		See Code 5 1		
						Centralisers installed {Yes/No}	No	(indicate on sketch)		
						Sump installed {Yes/No}	No	From	m To	m
						Pressure cemented {Yes/No}	No	From	m To	m
						Casing Protector cemented in place				

WATER ENTRY DESIGN 6										
General						Screen	Slot Details			
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Opening type	Fixing	Aperture (mm)	Length (mm)	Width (mm)	Alignment
Code 5					See Code 6	See Code 5				See Code 6

GRAVEL PACK 7								
Type	Grade	Grain size (mm)		Depth (m)		Quantity		m ³
		From	To	From	To	Litres		
Rounded	Graded				0			
Crushed	Ungraded							
Bentonite/Grout seal (Yes/No)		Yes		0	91			0.8
Method of placement of Gravel Pack		See Code 7						

For Departmental use only: GW

Work Licence No: 90BL256012

BORE DEVELOPMENT

8

Chemical used for breaking down drilling mud (Yes/No) No Name: _____

Method Bailing/Surging Jetting Airlifting Backwashing Pumping Other: _____

Duration _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs

DISINFECTION ON COMPLETION

9

Chemical(s) used	Quantity applied (Litres)	Method of application

PUMPING TESTS ON COMPLETION

10

Test type	Date	Pump intake depth (m)	Initial Water Level (SWL) (m)	Pumping rate (L/s)	Water Level at end of pumping (DDL) (m)	Duration of Test (hrs)	Recovery	
							Water level (m)	Time taken (hrs) (mins)
Multi stage (stepped drawdown)	Stage 1							
	Stage 2							
	Stage 3							
	Stage 4							
Single stage (constant rate)								
Height of measuring point above ground level			m	Test Method			See Code 4	

WORK PARTLY BACKFILLED OR ABANDONED

11

Original depth of work: 258 m Is work partly backfilled: (Yes/No) Yes

Is work abandoned: (Yes/No) No Method of abandonment: Backfilled Plugged Capped

Has any casing been left in the work (Yes/No) No From _____ m To _____ m

Sealing / fill type	From depth (m)	To depth (m)	Sealing / fill type	From depth (m)	To depth (m)
See Code 11	0	258	See Code 11		

Site chosen by: Hydrogeologist Geologist Driller Diviner Client Other _____

12

Lot No 2 DP No 1102940

Work Location Co ordinates Easting 232366.51 Northing 6594263.41 Zone 56

GPS: (Yes/No) Yes >> AMG/AGD or MGA/GDA (See explanation)

13

Please mark the work site with "X" on the CLID provided map.

Indicate also the distances in metres from two (2) adjacent boundaries, and attach the map to this Form A package.

Signatures:

Driller: _____

Licensee: _____

Date: _____

Date: _____

DRILLER'S ROCK/STRATA DESCRIPTION (LITHOLOGY)			WORK CONSTRUCTION SKETCH
Depth		Description <div style="border: 1px solid black; padding: 2px;">See Code 15</div>	
From (m)	To (m)		
0	1	Soil	
1	42	Weathered Conglomerate / Sandstone	
42	44	Sandstone	
44	44.5	Coal	
44.5	50	Conglomerate / Minor sandstone	
50	52	Coal	
52	81	Conglomerate / Minor sandstone	
81	82	Coal	
82	110	Conglomerate / Minor sandstone	
110	115	Coal	
115	131	Siltstone / Sandstone	
131	134	Coal	
134	149	Siltstone / Sandstone	
149	150	Coal	
150	155	Siltstone	
155	157	Coal	
157	188	Siltstone / Sandstone / Conglomerate	
188	192	Coal	
192	255	Sandstone / minor conglomerate	
255	258	Volcanics	

15

WORK NOT CONSTRUCTED BY DRILLING RIG							
Method of excavation: Hand dug <input type="checkbox"/> Back hoe <input type="checkbox"/> Dragline <input type="checkbox"/> Dozer <input type="checkbox"/> Other <input style="width: 100px;" type="text"/>							
Depth (m)	Length (m)	Width (m)	Diameter (m)	Lining material	Dimensions of liner (m)	From Depth (m)	To Depth (m)

16

Please attach copies of the following if available			
Geologist log	(Yes/No) <input type="checkbox"/>	Laboratory analysis of water Sample	(Yes/No) <input type="checkbox"/>
Geophysical log	(Yes/No) <input type="checkbox"/>	Sieve analysis of aquifer material	(Yes/No) <input type="checkbox"/>
		Pumping test(s)	(Yes/No) <input type="checkbox"/>
		Installed Pump details	(Yes/No) <input type="checkbox"/>

17



Driller's Licence No: DL1500 1
 Class of Licence:
 Driller's Name: Jason Manion
 Assistant Driller:
 Contractor: Manion Drilling
 New bore Replacement bore
 Deepened Enlarged
 Reconditioned Other (specify)
 Final Depth 242 m

Work Licence No: 90BL256010 2
 Name of Licensee: Whitehaven Coal Mine Ltd
 Intended Use: Vibrating Wire Piezometer
 Completion Date: 18/02/2012

DRILLING DETAILS 3			
From (m)	To (m)	Hole Diameter (mm)	Drilling Method
0	30	150	See Code 3
30	242	96	10
			13

WATER BEARING ZONES 4											
From (m)	To (m)	Thickness (m)	S W L (m)	Estimated Yield (L/s)		Test method	D D L at end of test (m)	Duration		Salinity (Conductivity or TDS)	
				Individual Aquifer	Cumulative			Hrs	min	Cond (µS/cm)	TDS (mg/L)
						See Code 4					

CASING / LINER DETAILS 5												
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Method Fixing	Casing support method		See Code 5 1				
Code 5					Code 5	Type of casing bottom		See Code 5 1				
						Centralisers installed {Yes/No}	No	(indicate on sketch)				
						Sump installed {Yes/No}	No	From		m To		m
						Pressure cemented {Yes/No}	No	From		m To		m
						Casing Protector cemented in place						

WATER ENTRY DESIGN 6										
General							Screen	Slot Details		
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Opening type	Fixing	Aperture (mm)	Length (mm)	Width (mm)	Alignment
Code 5					See Code 6	See Code 5				See Code 6

GRAVEL PACK 7								
Type	Grade	Grain size (mm)		Depth (m)		Quantity		m ³
		From	To	From	To	Litres		
Rounded	Graded				0			
Crushed	Ungraded							
Bentonite/Grout seal (Yes/No)		Yes			0	242		2
Method of placement of Gravel Pack		See Code 7						

For Departmental use only: GW

Work Licence No: 90BL256010

BORE DEVELOPMENT

8

Chemical used for breaking down drilling mud (Yes/No) No Name: _____

Method Bailing/Surging Jetting Airlifting Backwashing Pumping Other: _____

Duration _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs

DISINFECTION ON COMPLETION

9

Chemical(s) used	Quantity applied (Litres)	Method of application

PUMPING TESTS ON COMPLETION

10

Test type	Date	Pump intake depth (m)	Initial Water Level (SWL) (m)	Pumping rate (L/s)	Water Level at end of pumping (DDL) (m)	Duration of Test (hrs)	Recovery	
							Water level (m)	Time taken (hrs) (mins)
Multi stage (stepped drawdown)	Stage 1							
	Stage 2							
	Stage 3							
	Stage 4							
Single stage (constant rate)								
Height of measuring point above ground level _____ m		Test Method _____		See Code 4				

WORK PARTLY BACKFILLED OR ABANDONED

11

Original depth of work: 242 m Is work partly backfilled: (Yes/No) Yes

Is work abandoned: (Yes/No) No Method of abandonment: Backfilled Plugged Capped

Has any casing been left in the work (Yes/No) No From _____ m To _____ m

Sealing / fill type	From depth (m)	To depth (m)	Sealing / fill type	From depth (m)	To depth (m)
See Code 11	0	242	See Code 11		

Site chosen by: Hydrogeologist Geologist Driller Diviner Client Other _____

12

Lot No 2 DP No 1102940

Work Location Co ordinates Easting 233397.62 Northing 6591348.76 Zone 56

GPS: (Yes/No) Yes >> AMG/AGD or MGA/GDA (See explanation)

13

Please mark the work site with "X" on the CLID provided map.

Indicate also the distances in metres from two (2) adjacent boundaries, and attach the map to this Form A package.

Signatures:

Driller: _____

Licensee: _____

Date: _____

Date: _____



Work Licence No: 90BL256010

DRILLER'S ROCK/STRATA DESCRIPTION (LITHOLOGY)			WORK CONSTRUCTION SKETCH
Depth		Description See Code 15	
From (m)	To (m)		

15

WORK NOT CONSTRUCTED BY DRILLING RIG							
Method of excavation: Hand dug <input type="checkbox"/> Back hoe <input type="checkbox"/> Dragline <input type="checkbox"/> Dozer <input type="checkbox"/> Other <input type="text"/>							
Depth (m)	Length (m)	Width (m)	Diameter (m)	Lining material	Dimensions of liner (m)	From Depth (m)	To Depth (m)

16

Please attach copies of the following if available			
Geologist log	(Yes/No) <input type="checkbox"/>	Laboratory analysis of water Sample	(Yes/No) <input type="checkbox"/>
Geophysical log	(Yes/No) <input type="checkbox"/>	Sieve analysis of aquifer material	(Yes/No) <input type="checkbox"/>
		Pumping test(s)	(Yes/No) <input type="checkbox"/>
		Installed Pump details	(Yes/No) <input type="checkbox"/>

17



Driller's Licence No: DL1500 **1**

Class of Licence:

Driller's Name: Jason Manion

Assistant Driller:

Contractor: Manion Drilling

New bore Replacement bore

Deepened Enlarged

Reconditioned Other (specify)

Final Depth 91 m

Work Licence No: 90BL256009 **2**

Name of Licensee: Whitehaven Coal Mine Ltd

Intended Use: Vibrating Wire Piezometer

Completion Date: 12/02/2012

DRILLING DETAILS 3			
From (m)	To (m)	Hole Diameter (mm)	Drilling Method
0	31	150	See Code 3
31	91	96	10
			13

WATER BEARING ZONES 4											
From (m)	To (m)	Thickness (m)	S W L (m)	Estimated Yield (L/s)		Test method	D D L at end of test (m)	Duration		Salinity (Conductivity or TDS)	
				Individual Aquifer	Cumulative			Hrs	min	Cond (µS/cm)	TDS (mg/L)
						See Code 4					

CASING / LINER DETAILS 5											
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Method Fixing	Casing support method					
Code 5					Code 5	See Code 5 2					
						Type of casing bottom					
						See Code 5 2					
						Centralisers installed {Yes/No}	No	(indicate on sketch)			
						Sump installed {Yes/No}	No	From		m To	
						Pressure cemented {Yes/No}	No	From		m To	
						Casing Protector cemented in place					

WATER ENTRY DESIGN 6										
General						Screen	Slot Details			
Material	OD (mm)	Wall Thickness (mm)	From (m)	To (m)	Opening type	Fixing	Aperture (mm)	Length (mm)	Width (mm)	Alignment
Code 5					See Code 6	See Code 5				See Code 6

GRAVEL PACK 7								
Type	Grade	Grain size (mm)		Depth (m)		Quantity		
		From	To	From	To	Litres	m ³	
Rounded	Graded					0		
Crushed	Ungraded							
Bentonite/Grout seal (Yes/No)		Yes		0	91	0.8		
Method of placement of Gravel Pack		See Code 7						

For Departmental use only: **GW**

Work Licence No: 90BL256009

BORE DEVELOPMENT

8

Chemical used for breaking down drilling mud (Yes/No) No Name: _____

Method Bailing/Surging Jetting Airlifting Backwashing Pumping Other: _____

Duration _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs _____ hrs

DISINFECTION ON COMPLETION

9

Chemical(s) used	Quantity applied (Litres)	Method of application

PUMPING TESTS ON COMPLETION

10

Test type	Date	Pump intake depth (m)	Initial Water Level (SWL) (m)	Pumping rate (L/s)	Water Level at end of pumping (DDL) (m)	Duration of Test (hrs)	Recovery	
							Water level (m)	Time taken (hrs) (mins)
Multi stage (stepped drawdown)	Stage 1							
	Stage 2							
	Stage 3							
	Stage 4							
Single stage (constant rate)								
Height of measuring point above ground level _____ m		Test Method _____		See Code 4				

WORK PARTLY BACKFILLED OR ABANDONED

11

Original depth of work: 91 m Is work partly backfilled: (Yes/No) Yes

Is work abandoned: (Yes/No) No Method of abandonment: Backfilled Plugged Capped

Has any casing been left in the work (Yes/No) No From _____ m To _____ m

Sealing / fill type	From depth (m)	To depth (m)	Sealing / fill type	From depth (m)	To depth (m)
See Code 11	0	91	See Code 11		
1	0	91			

Site chosen by: Hydrogeologist Geologist Driller Diviner Client Other _____

12

Lot No 37 DP No 754929

Work Location Co ordinates Easting 230098.36 Northing 6593816.53 Zone 56

GPS: (Yes/No) Yes >> AMG/AGD or MGA/GDA (See explanation)

13

Please mark the work site with "X" on the CLID provided map.

Indicate also the distances in metres from two (2) adjacent boundaries, and attach the map to this Form A package.

Signatures:

Driller: _____

Licensee: _____

Date: _____

Date: _____

DRILLER'S ROCK/STRATA DESCRIPTION (LITHOLOGY)			WORK CONSTRUCTION SKETCH
Depth		Description <div style="border: 1px solid black; padding: 2px; text-align: center;">See Code 15</div>	
From (m)	To (m)		
0	1	Soil	
1	3	Clay minor sand	
4	8	Weathered Conglomerate / Sandstone	
8	11	Sandstone	
11	14	Conglomerate / Minor sandstone	
14	26	Sandstone	
26	40	Conglomerate / Minor sandstone	
40	42	Coal	
42	55	Conglomerate / Minor sandstone	
55	55.5	Coal	
55.5	58	Siltstone	
58	65	Conglomerate / Minor sandstone	
65	66.5	Siltstone	
66.5	68.5	Coal	
68.5	71	Siltstone	
71	72	Coal	
72	75	Siltstone	
75	79	Sandstone	
79	82	Siltstone	
82	84	Coal	
84	87	Siltstone	
87	91	Volcanics	

15

WORK NOT CONSTRUCTED BY DRILLING RIG							
Method of excavation: Hand dug <input type="checkbox"/> Back hoe <input type="checkbox"/> Dragline <input type="checkbox"/> Dozer <input type="checkbox"/> Other <input type="text"/>							
Depth (m)	Length (m)	Width (m)	Diameter (m)	Lining material	Dimensions of liner (m)	From Depth (m)	To Depth (m)

16

Please attach copies of the following if available			
Geologist log	(Yes/No) <input type="checkbox"/>	Laboratory analysis of water Sample	(Yes/No) <input type="checkbox"/>
Geophysical log	(Yes/No) <input type="checkbox"/>	Sieve analysis of aquifer material	(Yes/No) <input type="checkbox"/>
		Pumping test(s)	(Yes/No) <input type="checkbox"/>
		Installed Pump details	(Yes/No) <input type="checkbox"/>

17

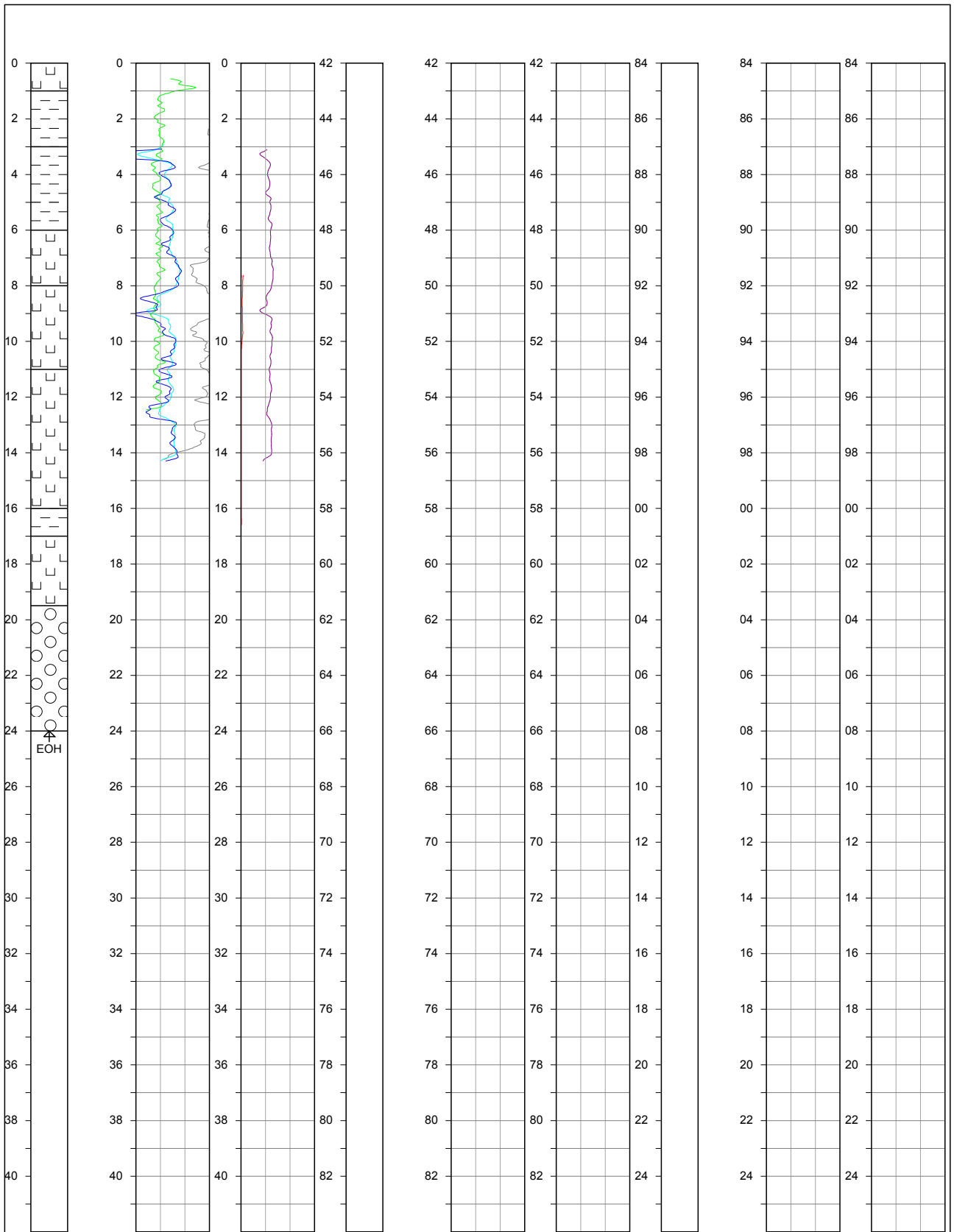
APPENDIX C
UPPER NAMOI ALLUVIUM / WEATHERED PERMIAN STRATA
BORE LOGS AND LICENCE DETAILS

Project Vickery
 BoreName TR 0001
 Total Depth 24m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting 232431
 Northing 6589763

County
 Parish
 Portion
 Map
 File

Date Commenced 7/12/2011
 Date Completed 7/12/2011
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Shane Cox
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0001			PAGE 1			
DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Soil	brown, unconsolidated, extremely weathered.		
3.000	2.000	2.000	Clay	yellow brown, soft, highly weathered.		
6.000	3.000	3.000	Clay	grey brown, soft, moderately weathered.		
8.000	2.000	2.000	Gravel	orange brown, pebbly, unconsolidated, highly weathered.		
11.000	3.000	3.000	Gravel	grey brown, pebbly, unconsolidated, moderately weathered.		
16.000	5.000	5.000	Gravel	yellow brown, pebbly, unconsolidated, highly weathered.		
17.000	1.000	1.000	Clay	grey brown, soft, highly weathered.		
19.500	2.500	2.500	Gravel	mottled brown, pebbly, unconsolidated, ferruginous, highly weathered.		
24.000	4.500	4.500	Conglomerate	mottled brown, cobbly, hard, moderately weathered, subrounded.		Water @ 17m



0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0



Vickery
 TR001

Project Vickery
 BoreName TR 0003
 Total Depth 36m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting 232431
 Northing 6589563

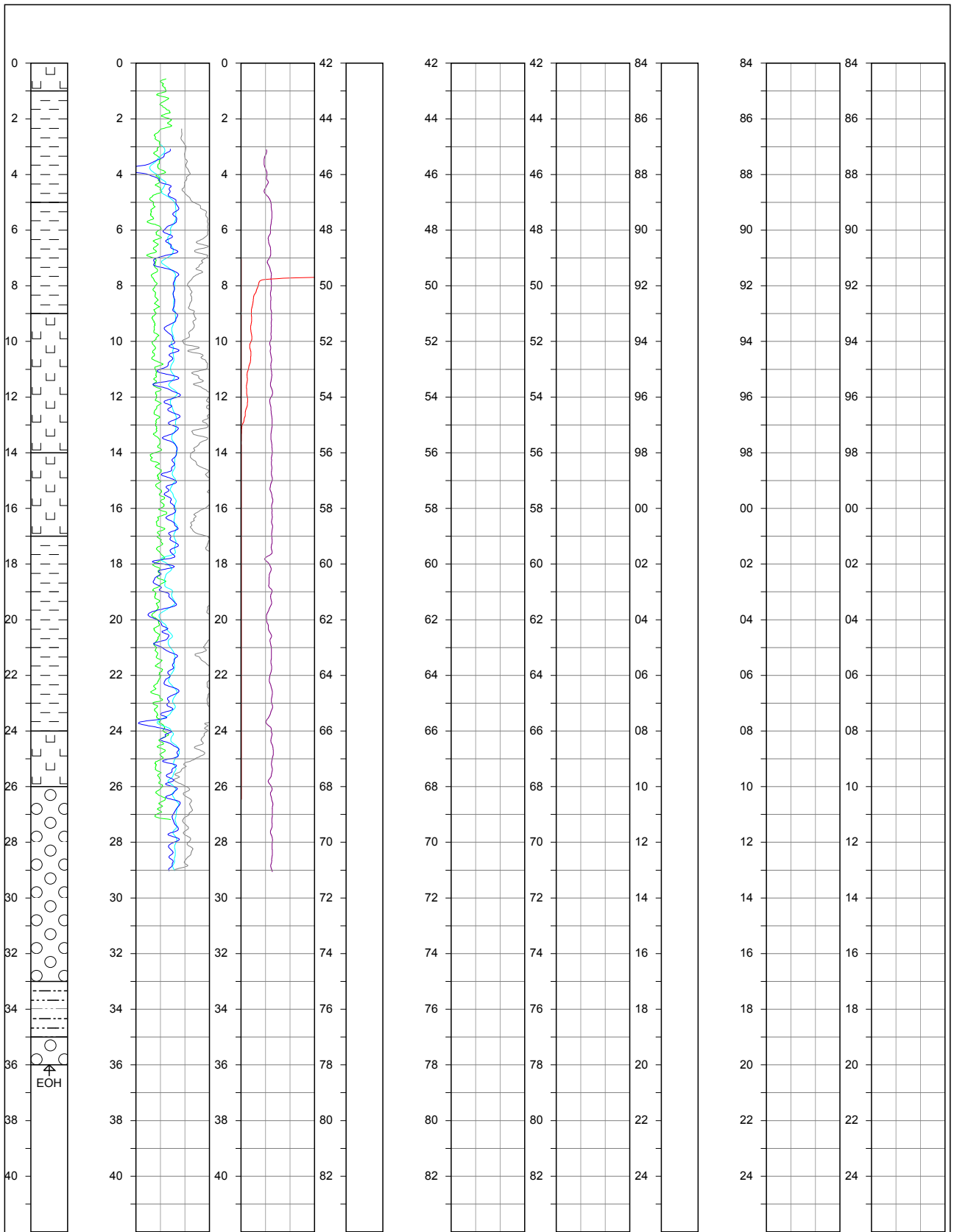
County
 Parish
 Portion
 Map
 File

Date Commenced 7/12/2011
 Date Completed 7/12/2011
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Shane Cox
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0003

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Soil	dark brown, unconsolidated, extremely weathered.		
5.000	4.000	4.000	Clay	orange brown, unconsolidated, ferruginous, moderately weathered.		
9.000	4.000	4.000	Clay	grey brown, soft, moderately weathered.		
14.000	5.000	5.000	Gravel	grey brown, pebbly, unconsolidated, moderately weathered.		
17.000	3.000	3.000	Gravel	orange brown, cobbly, unconsolidated, sandy ferruginous, moderately weathered.		
24.000	7.000	7.000	Clay	grey brown, pebbly, soft, moderately weathered.		
26.000	2.000	2.000	Gravel	mottled brown, cobbly, unconsolidated, highly weathered.		
33.000	7.000	7.000	Conglomerate	red brown, pebbly, moderately hard, ferruginous, moderately weathered.		
35.000	2.000	2.000	Siltstone	light grey, moderately hard, slightly weathered.		
36.000	1.000	1.000	Conglomerate	mottled brown, pebbly, hard, slightly weathered.		Water @ 17m



0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0



Vickery
TR003

Project Vickery
 Bore Name TR 0004
 Total Depth 21m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting 232431
 Northing 6589963

County
 Parish
 Portion
 Map
 File

Date Commenced 7/12/2011
 Date Completed 7/12/2011
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Shane Cox
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0004

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Soil	brown, unconsolidated, extremely weathered.		
4.000	3.000	3.000	Clay	yellow brown, unconsolidated, highly weathered.		
6.000	2.000	2.000	Sand	brown, medium to coarse grained, unconsolidated, highly weathered.		
8.000	2.000	2.000	Clay	grey brown, soft, highly weathered.		
10.000	2.000	2.000	Gravel	red brown, pebbly, clayey, highly weathered.		
16.000	6.000	6.000	Gravel	mottled brown, pebbly, unconsolidated, moderately weathered.		
19.000	3.000	3.000	Gravel	brown, cobbly, unconsolidated, highly weathered.		
21.000	2.000	2.000	Conglomerate	mottled brown, pebbly, hard, moderately weathered.		Water @ 17m

Project Vickery
 BoreName TR 0005
 Total Depth 31m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

County
 Parish
 Portion
 Map
 File

Date Commenced 15/12/2011
 Date Completed 15/12/2011
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Shane Cox
 Logged By Craig Zirkler
 District
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BORE NAME TR 0005

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
2.000	2.000	2.000	Soil	red brown, unconsolidated, extremely weathered.		
3.000	1.000	1.000	Clay	grey brown, unconsolidated, highly weathered.		
6.000	3.000	3.000	Gravel	grey brown, pebbly, unconsolidated, highly weathered.		
9.000	3.000	3.000	Clay	brown grey, unconsolidated, highly weathered.		
11.000	2.000	2.000	Gravel	red brown, unconsolidated, highly weathered.		
13.000	2.000	2.000	Clay	brown grey, pebbly, unconsolidated, highly weathered.		
14.000	1.000	1.000	Gravel	mottled brown, pebbly, unconsolidated, highly weathered.		Small amount of water @ 14m > 16.0m > 23m
22.000	8.000	8.000	Conglomerate	mottled brown, pebbly, hard, moderately weathered.		
23.000	1.000	1.000	Conglomerate	orange brown, pebbly, hard, moderately weathered.		
31.000	8.000	8.000	Conglomerate	mottled brown, pebbly, hard, slightly weathered.		

_ Project Vickery
 BoreName TR 0006
 Total Depth 25m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

County
 Parish
 Portion
 Map
 File

Date Commenced 15/12/2011
 Date Completed 15/12/2011
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Shane Cox
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0006

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Soil	red brown, unconsolidated, extremely weathered.		
4.000	3.000	3.000	Gravel	red brown, pebbly, unconsolidated, highly weathered.		
7.000	3.000	3.000	Clay	light grey, unconsolidated, moderately weathered.		
17.000	10.000	10.000	Conglomerate	mottled brown, pebbly, moderately hard, moderately weathered.		
19.000	2.000	2.000	Claystone	brown, moderately hard, moderately weathered.		
25.000	6.000	6.000	Conglomerate	mottled brown, hard, slightly weathered.		

Project Vickery
 BoreName TR 0007
 Total Depth 36m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

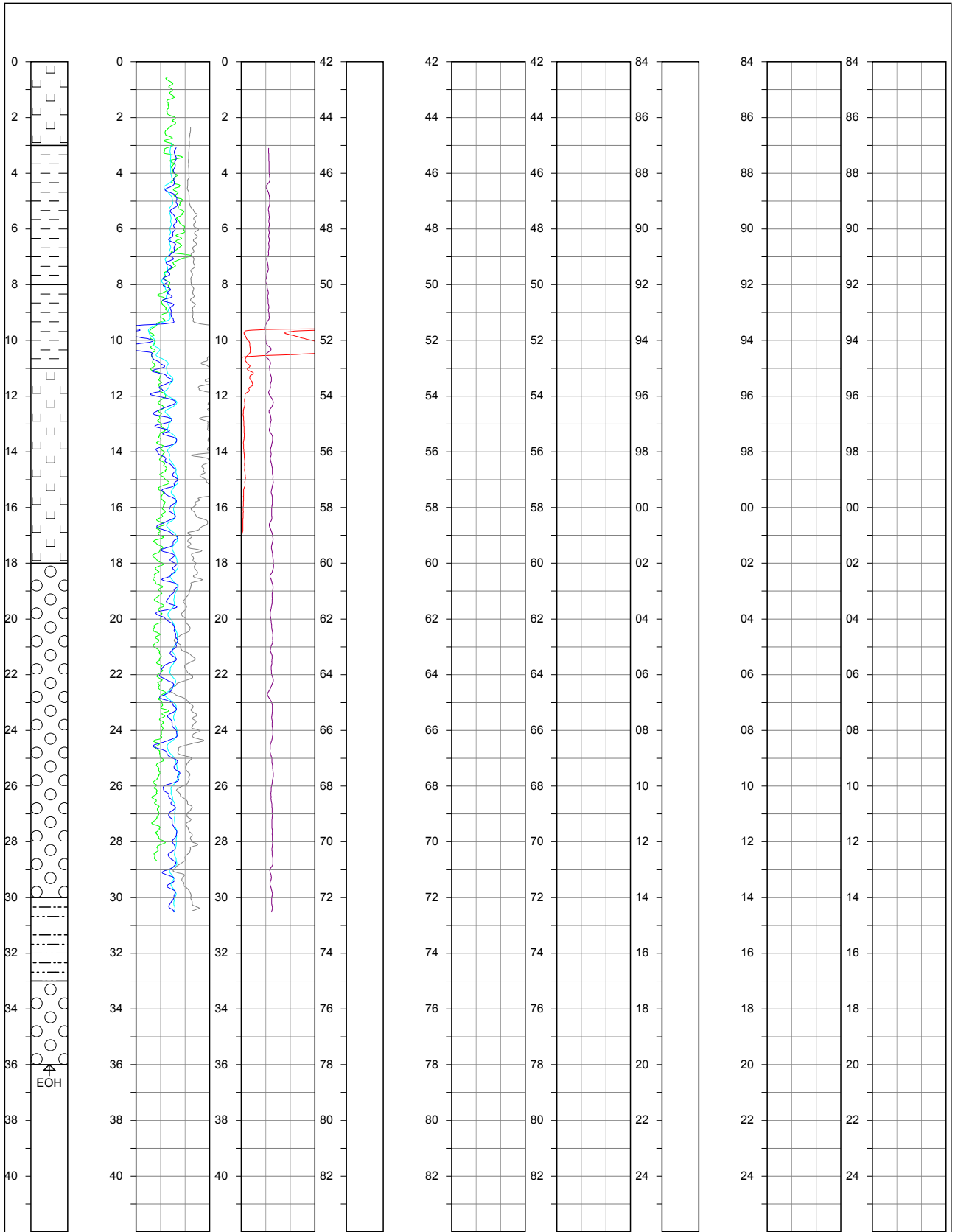
County
 Parish
 Portion
 Map
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Date Commenced 15/12/2011
 Date Completed 16/12/2011
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Shane Cox
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0007

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
3.000	3.000	3.000	Soil	grey brown, unconsolidated, extremely weathered.		
8.000	5.000	5.000	Clay	grey brown, unconsolidated, highly weathered.		
11.000	3.000	3.000	Clay	brown grey, unconsolidated, highly weathered.		
18.000	7.000	7.000	Gravel	mottled brown, cobbly, unconsolidated, highly weathered.		Water @ 17m-600g/hr-conglomerate fractionated - gravel like
30.000	12.000	12.000	Conglomerate	mottled brown, pebbly, moderately hard, ferruginous, moderately weathered.		
33.000	3.000	3.000	Siltstone	light grey, soft, clayey, moderately weathered.		
36.000	3.000	3.000	Conglomerate	mottled brown, pebbly, moderately hard, slightly weathered.		



0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0



Vickery
TR007

Project Vickery
 BoreName TR 0008
 Total Depth 42m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

County
 Parish
 Portion
 Map
 File

Date Commenced 16/12/2011
 Date Completed 16/12/2011
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Shane Cox
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0008

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Soil	dark brown, unconsolidated, extremely weathered.		
4.000	3.000	3.000	Clay	yellow brown, unconsolidated, highly weathered.		
7.000	3.000	3.000	Clay	grey brown, unconsolidated, highly weathered.		
11.000	4.000	4.000	Gravel	mottled brown, pebbly, unconsolidated, highly weathered.		
14.000	3.000	3.000	Sand	red brown, medium to coarse grained, unconsolidated, highly weathered.		Moisture @ 13m > 19m
19.000	5.000	5.000	Gravel	mottled brown, pebbly, unconsolidated, highly weathered.		
21.000	2.000	2.000	Siltstone	light brown, soft, moderately weathered.		
24.000	3.000	3.000	Sandstone	light brown, medium to coarse grained, soft, clayey, moderately weathered.		
28.000	4.000	4.000	Conglomerate	mottled brown, pebbly, hard, moderately weathered.		Water 1100g/hr @ 25 > 26m
30.000	2.000	2.000	Sandstone	red brown, medium to coarse grained, moderately hard, moderately weathered.		
40.000	10.000	10.000	Conglomerate	mottled brown, pebbly, hard, slightly weathered.		
42.000	2.000	2.000	COAL	Weathered, brown black, slightly weathered.		

Project Vickery
 BoreName TR 0009
 Total Depth 37m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

County
 Parish
 Portion
 Map
 File

Date Commenced 16/12/2011
 Date Completed 16/12/2011
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Shane Cox
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0009

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Soil	brown, unconsolidated, extremely weathered.		
3.000	2.000	2.000	Clay	dark brown, unconsolidated, moderately weathered.		
7.000	4.000	4.000	Gravel	mottled brown, pebbly, unconsolidated, clayey, highly weathered.		
9.000	2.000	2.000	Gravel	red brown, cobbly, unconsolidated, highly weathered.		
14.000	5.000	5.000	Clay	light grey, soft, highly weathered.		
15.000	1.000	1.000	Sand	orange brown, medium to coarse grained, unconsolidated, highly weathered.		Moisture @ 15m
17.000	2.000	2.000	Gravel	mottled brown, pebbly, unconsolidated, moderately weathered.		
26.000	9.000	9.000	Conglomerate	mottled brown, pebbly, hard, moderately weathered.		Water 1000g/hr @ 23m
27.000	1.000	1.000	Sandstone	light brown, medium to coarse grained, moderately hard, moderately weathered.		
37.000	10.000	10.000	Conglomerate	mottled brown, pebbly, hard, slightly weathered.		

Project Vickery
 BoreName TR 0010
 Total Depth 25m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

County
 Parish
 Portion
 Map
 File

Date Commenced 16/12/2011
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 Contractor Mannion Drilling
 Driller Shane Cox
 Logged By Craig Zirkler
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BORE NAME TR 0010

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Soil	dark brown, unconsolidated, extremely weathered.		
9.000	8.000	8.000	Gravel	mottled brown, unconsolidated, clayey, highly weathered.		
12.000	3.000	3.000	Clay	light grey, soft, highly weathered.		
14.000	2.000	2.000	Gravel	mottled brown, unconsolidated, sandy, highly weathered.		Slightly moist 13-14m
18.000	4.000	4.000	Siltstone	light brown, moderately hard, moderately weathered.		
21.000	3.000	3.000	Conglomerate	mottled brown, pebbly, hard, moderately weathered.		
22.000	1.000	1.000	Sandstone	orange brown, medium to coarse grained, moderately hard, ferruginous, slightly weathered.		
25.000	3.000	3.000	Conglomerate	mottled brown, pebbly, hard, slightly weathered.		

Project Vickery
 BoreName TR 0011
 Total Depth 19m
 Site Vickery

Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

County
 Parish
 Portion
 Map
 File

Date Commenced 16/12/2011
 Date Completed 16/12/2011
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Shane Cox
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0011

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA	SEAM	COMMENTS
1.000	1.000	1.000	Soil brown, unconsolidated.		
4.000	3.000	3.000	Clay grey brown, pebbly, unconsolidated.		
10.000	6.000	6.000	Gravel mottled brown, pebbly, unconsolidated, clayey.		
19.000	9.000	9.000	Conglomerate mottled brown, cobbly, hard.		

Project Vickery
 BoreName TR 0012
 Total Depth 30m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

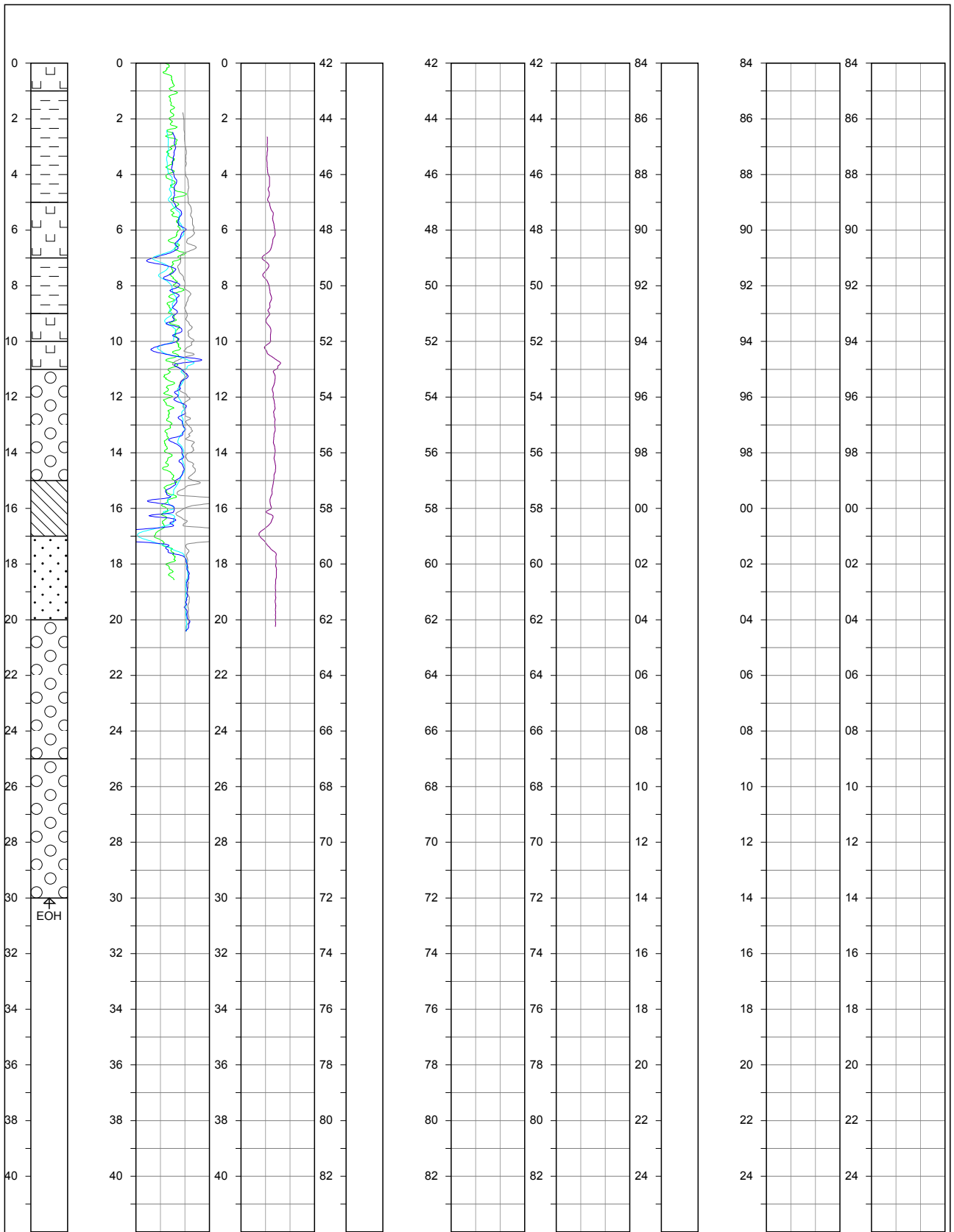
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Date Commenced 11/1/2012
 Date Completed 11/1/2012
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Jason Mannion
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0012

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Soil	light brown, unconsolidated, extremely weathered.		
5.000	4.000	4.000	Clay	brown grey, unconsolidated, highly weathered.		
7.000	2.000	2.000	Gravel	red brown, pebbly, unconsolidated, highly weathered.		
9.000	2.000	2.000	Clay	brown grey, unconsolidated, highly weathered.		
10.000	1.000	1.000	Gravel	mottled brown, pebbly, unconsolidated, highly weathered.		
11.000	1.000	1.000	Gravel	dark brown, pebbly, unconsolidated, ferruginous, highly weathered.		
15.000	4.000	4.000	Conglomerate	mottled brown, pebbly, hard, moderately weathered.		
17.000	2.000	2.000	Carbonaceous Mudstone	brown black, soft, puggy, moderately weathered.		
20.000	3.000	3.000	Sandstone	grey brown, moderately hard, moderately weathered.		
25.000	5.000	5.000	Conglomerate	mottled brown, pebbly, hard, moderately weathered.		Slight amount of water @21m
30.000	5.000	5.000	Conglomerate	mottled, cobbly, hard, slightly weathered.		



0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0



Vickery
 TR012

Project Vickery
 BoreName TR 0013
 Total Depth 19m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

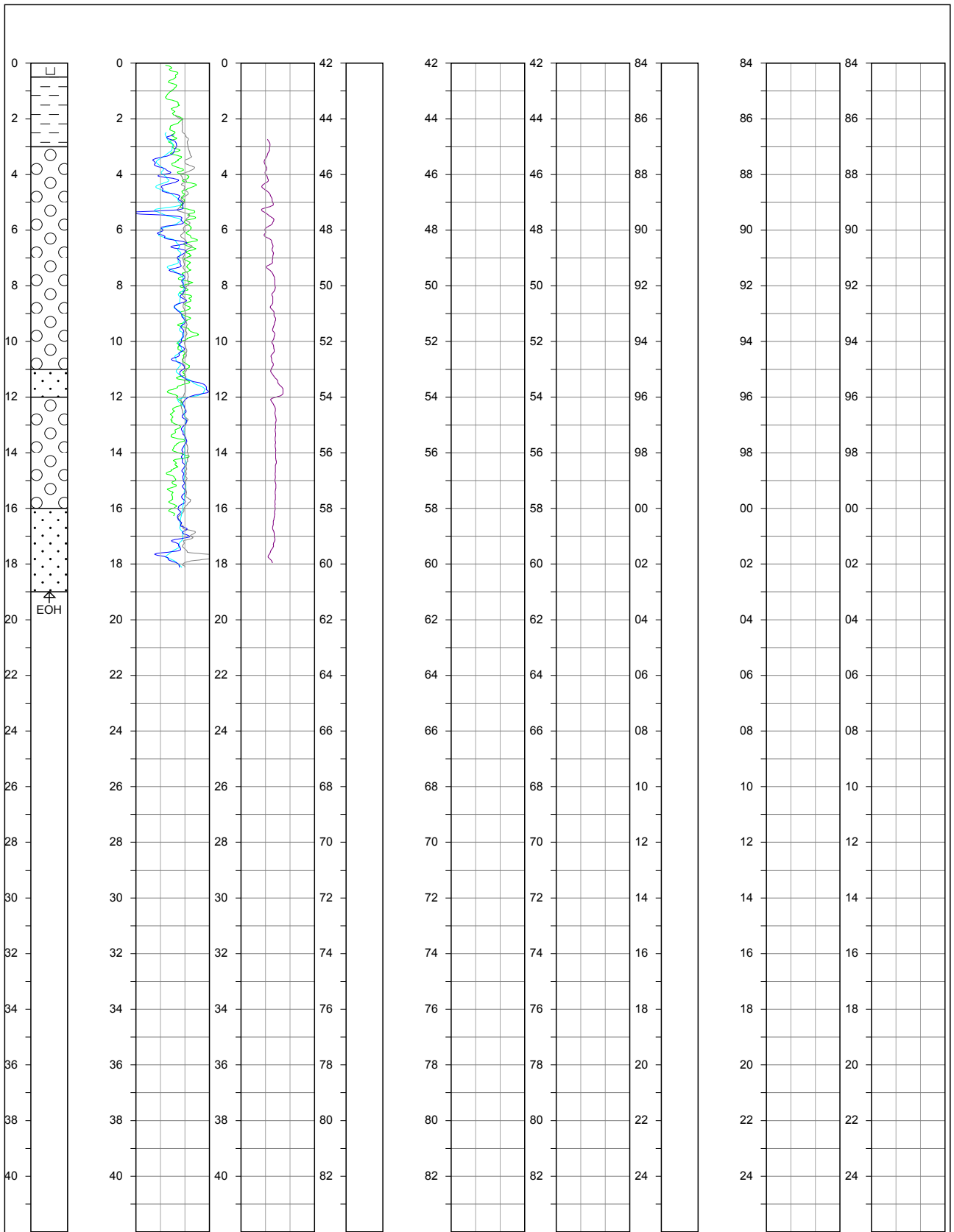
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Date Commenced 11/1/2012
 Date Completed 11/1/2012
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Jason Mannion
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0013

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA	SEAM	COMMENTS
0.500	0.500	0.500	Soil red brown, unconsolidated, extremely weathered.		
3.000	2.500	2.500	Clay red brown, unconsolidated, highly weathered.		
11.000	8.000	8.000	Conglomerate mottled grey, pebbly, hard, moderately weathered.		
12.000	1.000	1.000	Sandstone brown, medium to coarse grained, hard, ferruginous, moderately weathered.		
16.000	4.000	4.000	Conglomerate mottled brown, hard, moderately weathered.		
19.000	3.000	3.000	Sandstone light brown, medium to coarse grained, moderately hard, moderately weathered.		Water @ 17m



0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0



Vickery
 TR013

Project Vickery
 BoreName TR 0014
 Total Depth 37m
 Site Vickery
 Locality

Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

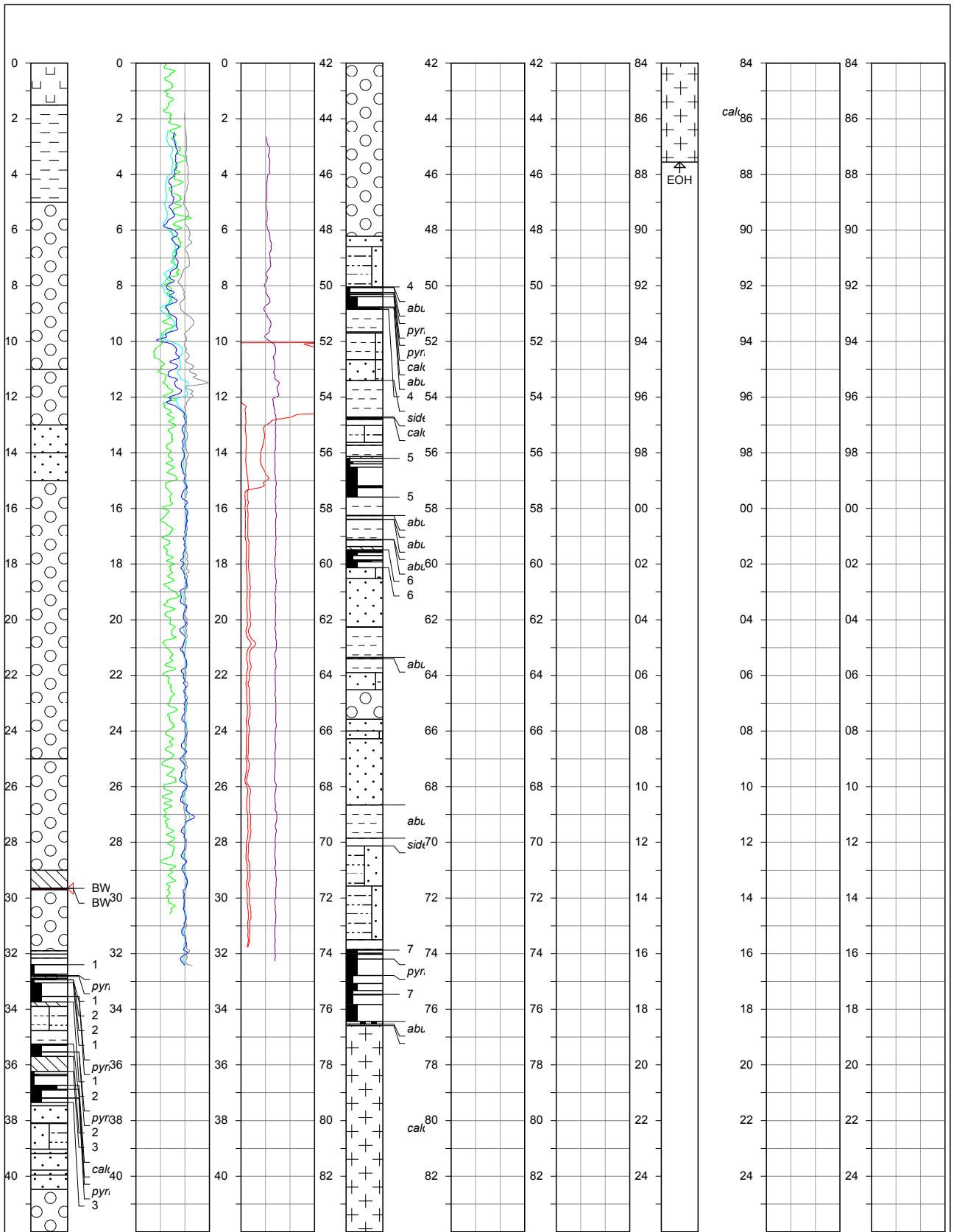
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Date Commenced 11/1/2012
 Date Completed 11/1/2012
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Jason Mannion
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0014

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Soil	red brown, unconsolidated.		
4.000	3.000	3.000	Clay	brown, unconsolidated.		
9.000	5.000	5.000	Clay	brown purple, pebbly, unconsolidated.		
11.000	2.000	2.000	Clay	brown grey, unconsolidated.		
12.000	1.000	1.000	Sandstone	light brown, fine to medium grained, moderately hard.		
23.000	11.000	11.000	Conglomerate	mottled brown, pebbly, hard.		
26.000	3.000	3.000	Sandstone	light brown, medium to coarse grained, moderately hard.		
30.000	4.000	4.000	Conglomerate	mottled brown, pebbly, hard.		
31.000	1.000	1.000	Sandstone	grey brown, medium to coarse grained, moderately hard.		
33.000	2.000	2.000	Conglomerate	mottled brown, pebbly, hard, ferruginous.		
34.000	1.000	1.000	No Recovery			Water
37.000	3.000	3.000	Conglomerate	mottled brown, pebbly, hard.		



0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0



Vickery
 TR014

Project Vickery
 BoreName TR 0015
 Total Depth 31m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

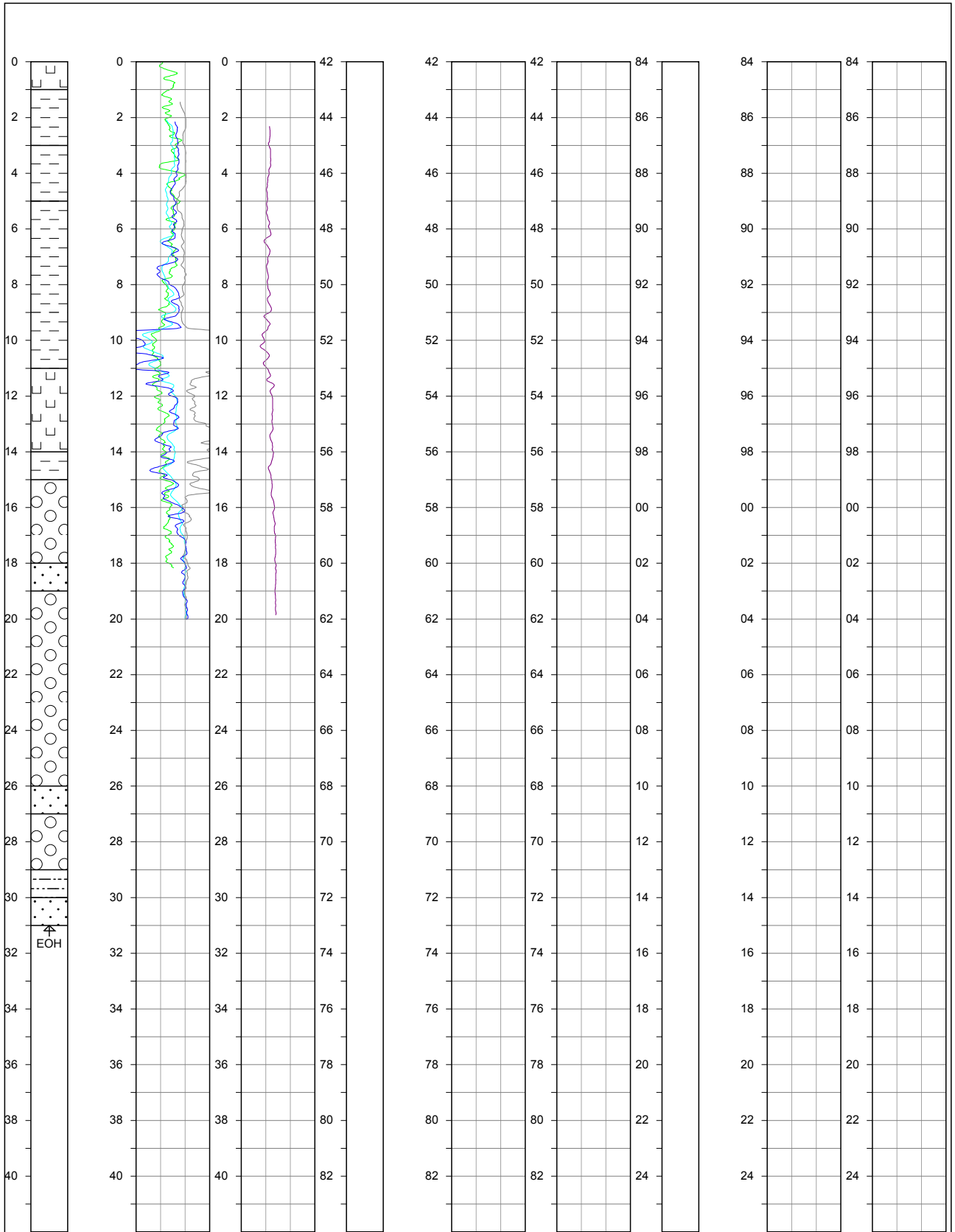
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Date Commenced 12/1/2012
 Date Completed 12/1/2012
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Jason Mannion
 Logged By Craig Zirkler
 District
 Distribution
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BORE NAME TR 0015

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Soil	dark brown, unconsolidated.		
3.000	2.000	2.000	Clay	grey brown, unconsolidated.		
5.000	2.000	2.000	Clay	yellow brown, unconsolidated.		
11.000	6.000	6.000	Clay	brown grey, unconsolidated.		
14.000	3.000	3.000	Gravel	grey brown, cobbly, unconsolidated.		
15.000	1.000	1.000	Clay	brown grey, unconsolidated.		Moisture
18.000	3.000	3.000	Conglomerate	mottled brown, pebbly, hard.		
19.000	1.000	1.000	Sandstone	brown, medium to coarse grained, moderately hard.		
26.000	7.000	7.000	Conglomerate	mottled brown, pebbly, hard.		
27.000	1.000	1.000	Sandstone	grey brown, medium to coarse grained, moderately hard, ferruginous.		
29.000	2.000	2.000	Conglomerate	mottled brown, pebbly, hard.		
30.000	1.000	1.000	Siltstone	brown grey, moderately hard.		
31.000	1.000	1.000	Sandstone	orange brown, moderately hard.		



0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0



Vickery
 TR015

Project Vickery
 BoreName TR 0016
 Total Depth 31m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

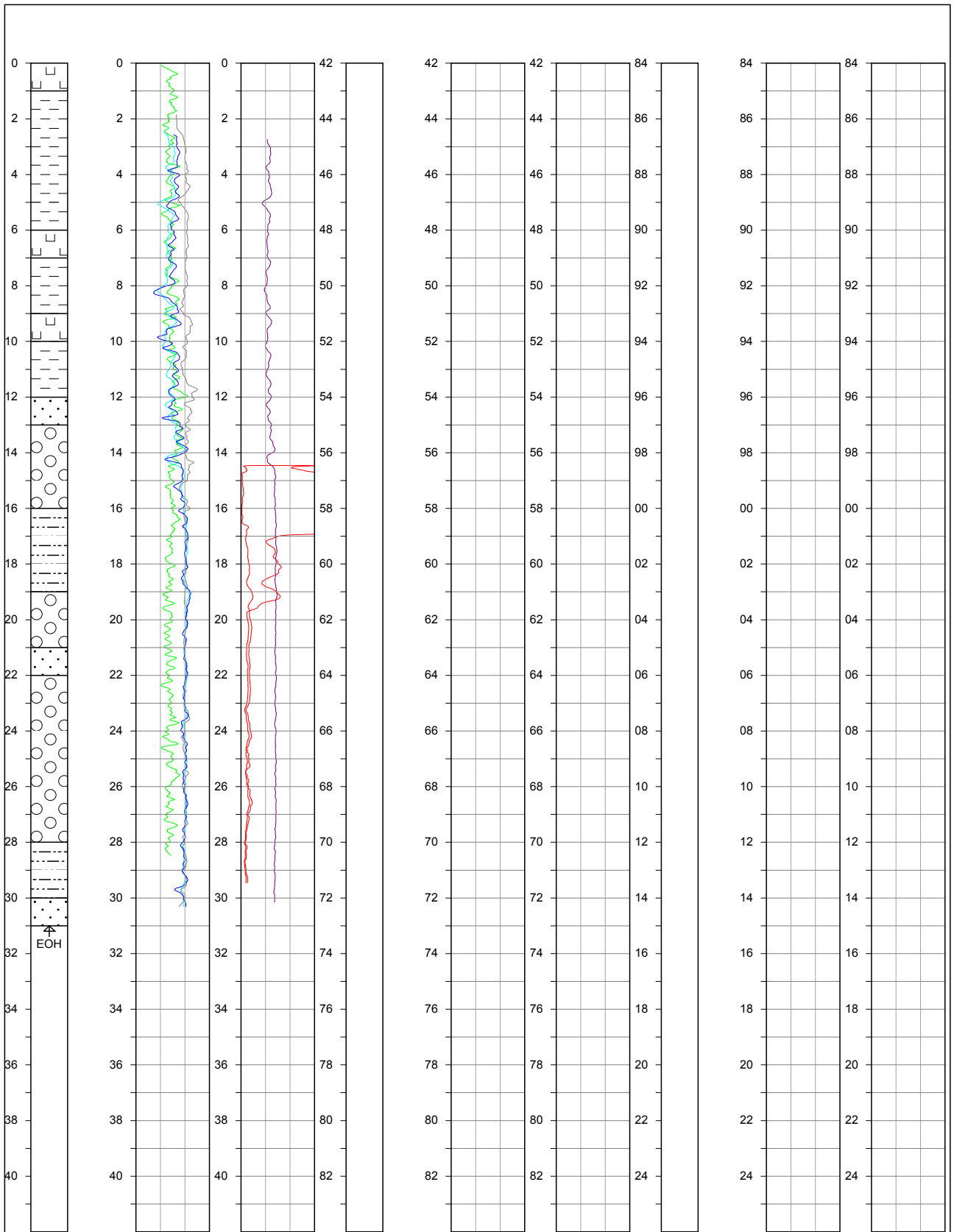
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Date Commenced 12/1/2012
 Date Completed 12/1/2012
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Jason Mannion
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0016

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Gravel	mottled brown, unconsolidated.		
6.000	5.000	5.000	Clay	grey brown, unconsolidated.		
7.000	1.000	1.000	Gravel	grey brown, pebbly, unconsolidated.		
9.000	2.000	2.000	Clay	grey brown, unconsolidated.		
10.000	1.000	1.000	Gravel	mottled brown, pebbly, unconsolidated.		
12.000	2.000	2.000	Clay	grey brown, unconsolidated.		Moisture
13.000	1.000	1.000	Sandstone	light brown, medium to coarse grained, moderately hard.		
16.000	3.000	3.000	Conglomerate	mottled brown, pebbly, hard.		
19.000	3.000	3.000	Siltstone	grey brown, moderately hard.		
21.000	2.000	2.000	Conglomerate	mottled brown, pebbly, hard.		
22.000	1.000	1.000	Sandstone	light brown, medium to coarse grained, moderately hard.		
28.000	6.000	6.000	Conglomerate	mottled brown, pebbly, hard.		
30.000	2.000	2.000	Siltstone	grey brown, moderately hard.		
31.000	1.000	1.000	Sandstone	dark brown, moderately hard.		



0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200 0.0 FE1 200
 1.0 DENL 3.0 0.0 FE2 300
 1.0 DENB 3.0



Vickery
TR016

Project Vickery
 BoreName TR 0017
 Total Depth 19m
 Site Vickery
 Locality

Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

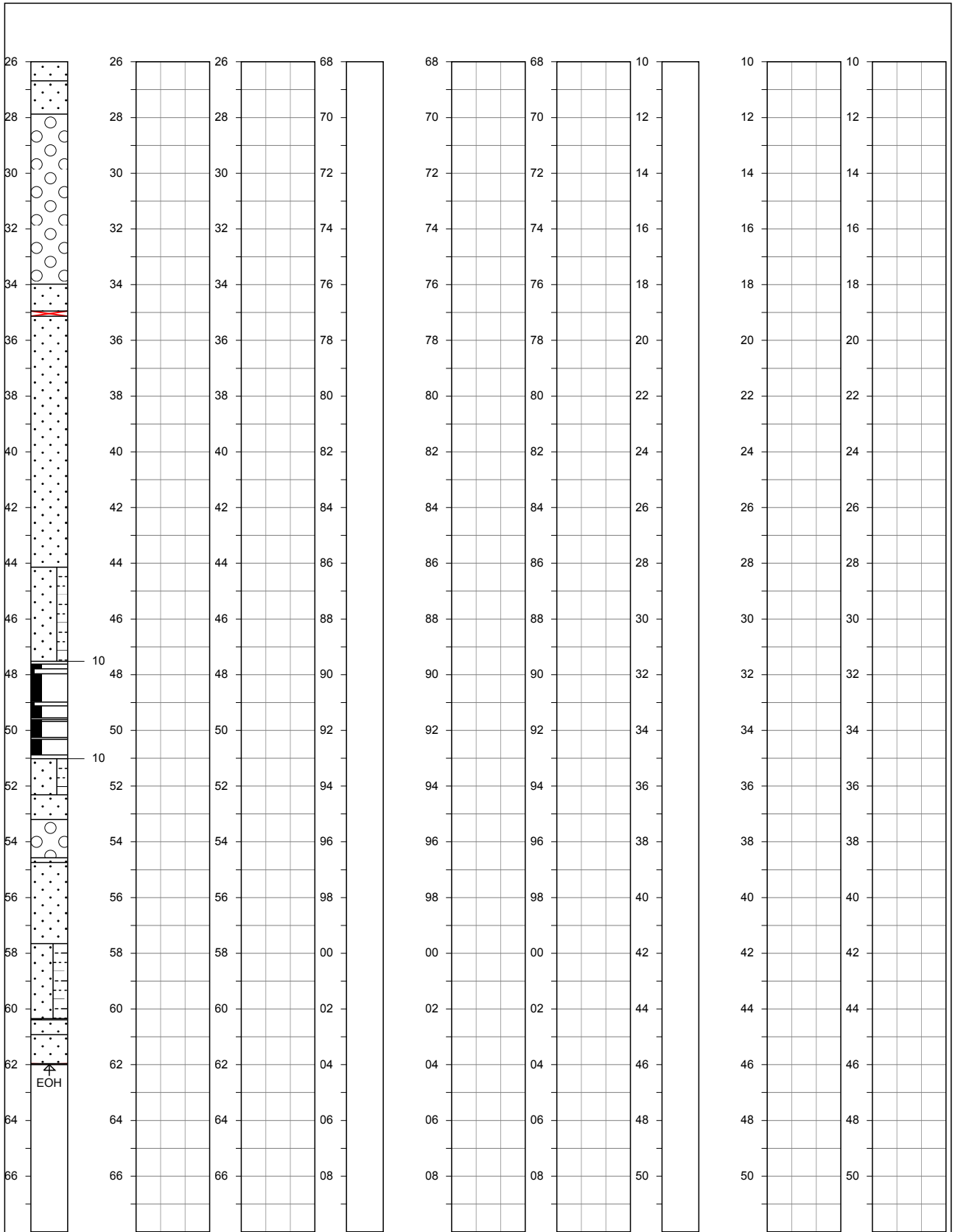
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Date Commenced 12/1/2012
 Date Completed 12/1/2012
 Commissioned by Whitehaven Coal
 Contractor Mannion Drilling
 Driller Jason Mannion
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0017

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
0.500	0.500	0.500	Soil	brown, unconsolidated.		
5.000	4.500	4.500	Clay	mottled brown, pebbly, unconsolidated.		
8.000	3.000	3.000	Gravel	mottled brown, pebbly, unconsolidated.		
9.000	1.000	1.000	Clay	red brown, unconsolidated, ferruginous.		
12.000	3.000	3.000	Conglomerate	mottled brown, pebbly, hard.		
16.000	4.000	4.000	Sandstone	light brown, fine to medium grained, moderately hard.		
17.000	1.000	1.000	Conglomerate	mottled brown, pebbly.		
18.000	1.000	1.000	Sandstone	red brown, medium to coarse grained, moderately hard, ferruginous.		
19.000	1.000	1.000	Conglomerate	mottled brown, pebbly, hard.		



0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275 0.0 CODE 5.0
 90.0 CADE 200
 1.0 DENL 3.0
 1.0 DENB 3.0



Vickery
 TR017

Project Vickery
 BoreName TR0018
 Total Depth 23m
 Site Vickery
 Locality
 Collar Level
 Datum AHD
 Easting
 Northing
 Collar Level
 Easting
 Northing

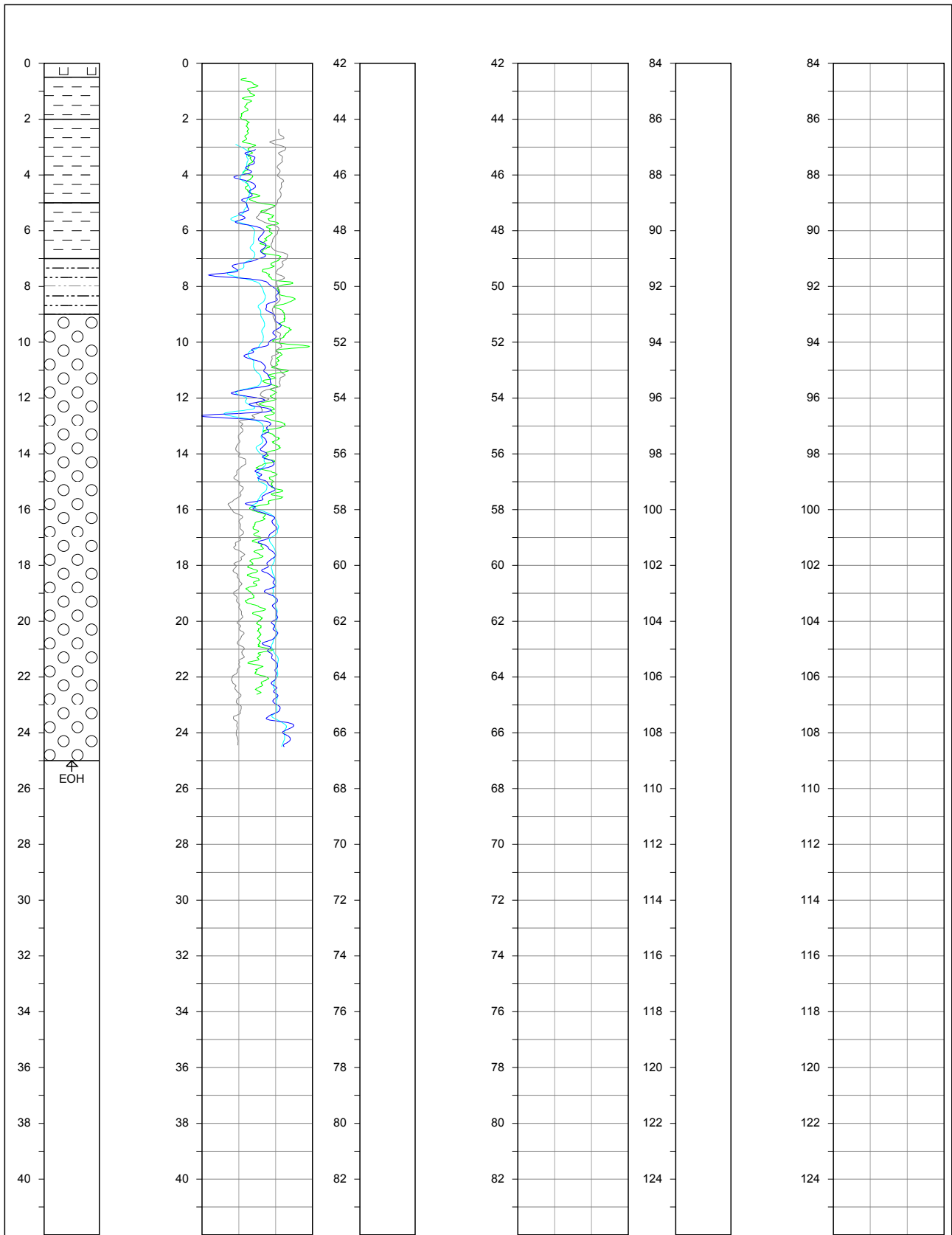
County
 Parish
 Portion
 Map
 File

Date Commenced 12/1/2012
 Date Completed 12/1/2012
 Commissioned by Whiteven Coal
 Contractor Mannion Drilling
 Driller Jason Mannion
 Logged By Craig Zirkler
 District
 Distribution
 Comments

BORE NAME TR 0018

PAGE 1

DEPTH	THICKNESS	RECOVERED	GEOLOGICAL DESCRIPTION OF STRATA		SEAM	COMMENTS
1.000	1.000	1.000	Soil	dark brown, unconsolidated, extremely weathered.		
12.000	11.000	11.000	Clay	grey brown, unconsolidated, moderately weathered.		
15.000	3.000	3.000	Gravel	mottled brown, unconsolidated, moderately weathered.		
18.000	3.000	3.000	Gravel	buff, unconsolidated, highly weathered.		
19.000	1.000	1.000	Conglomerate	brown, pebbly, hard, moderately weathered.		
20.000	1.000	1.000	Siltstone	grey brown, moderately hard, moderately weathered.		
21.000	1.000	1.000	Sandstone	light brown, fine to medium grained, moderately hard, moderately weathered.		Water @ 18m
23.000	2.000	2.000	Conglomerate	mottled brown, pebbly, hard, moderately weathered.		



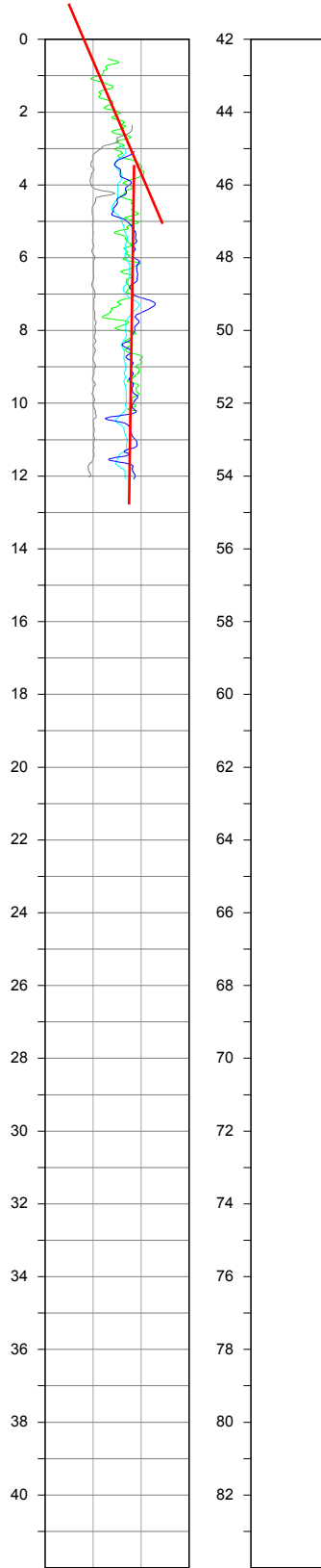
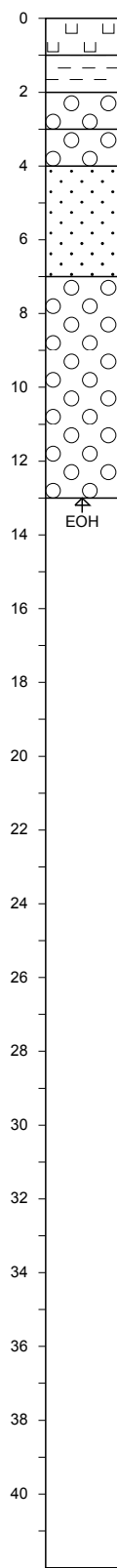
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 1.0 DENB 3.0

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 1.0 DENB 3.0

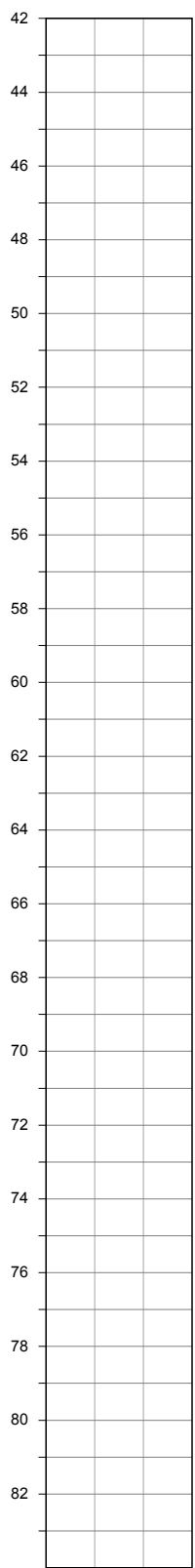
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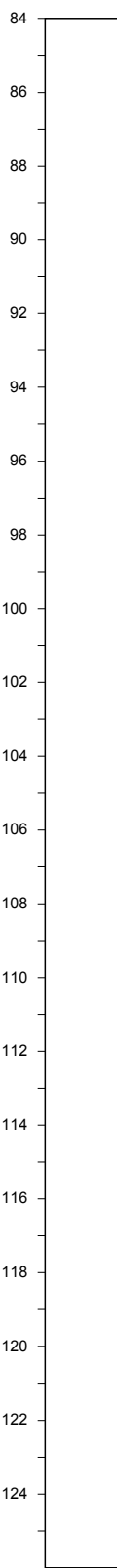
Vickery
 TR0020



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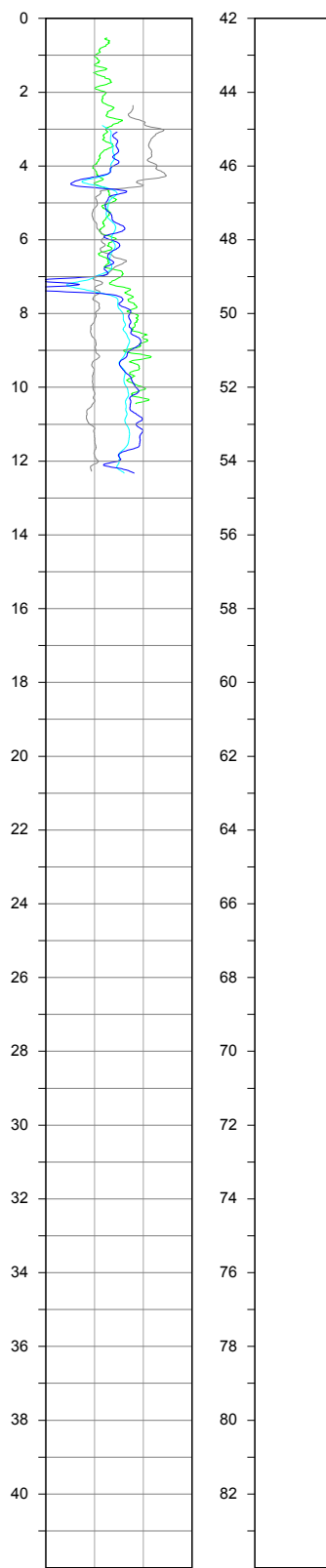
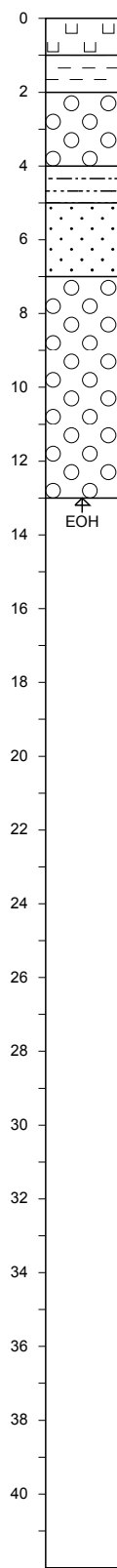
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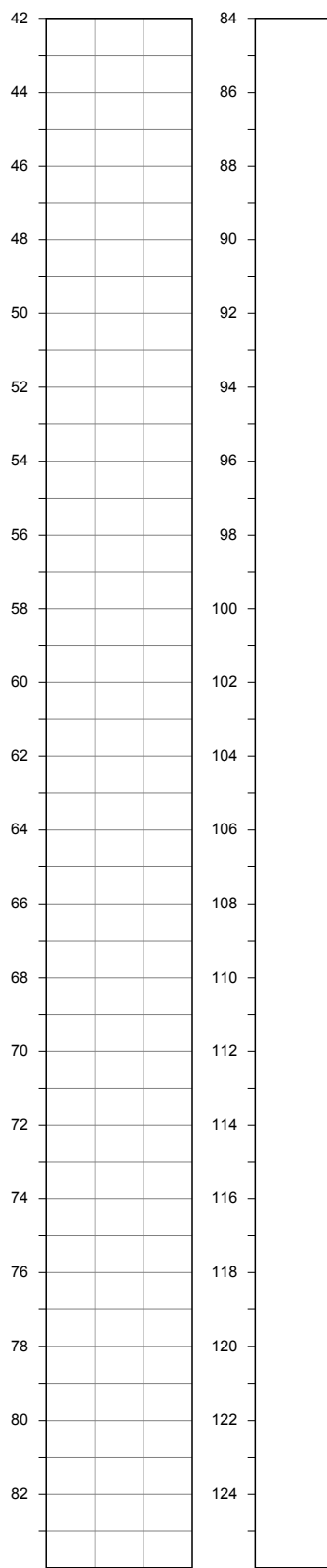
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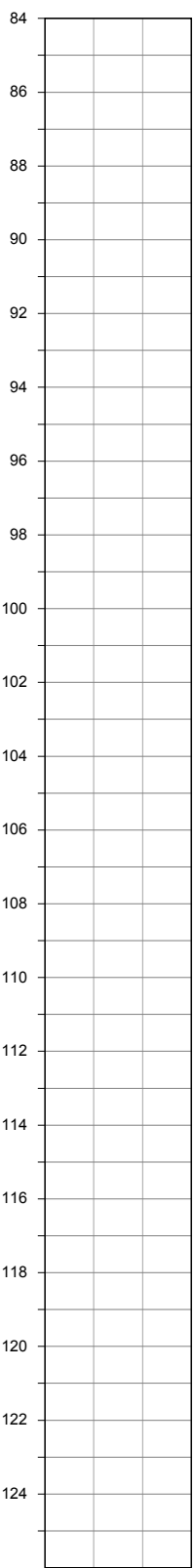
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 TR0023



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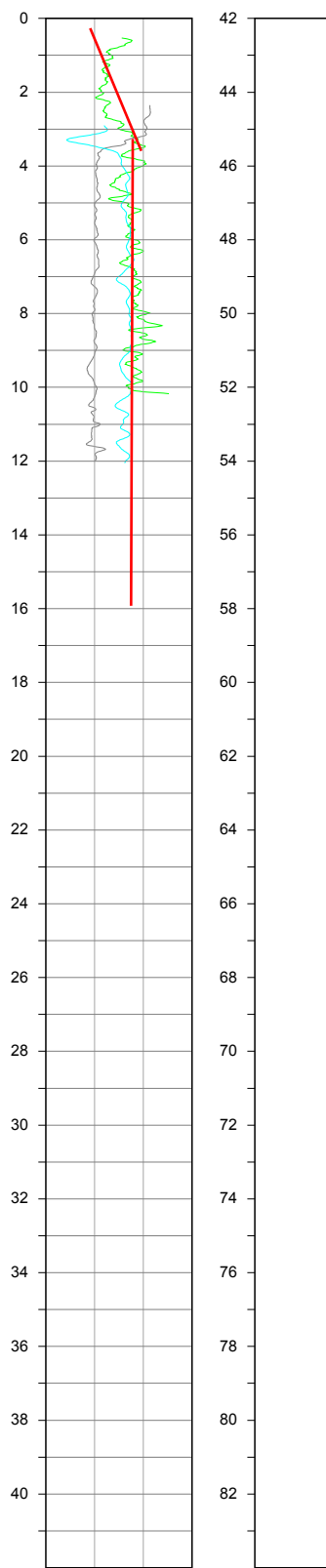
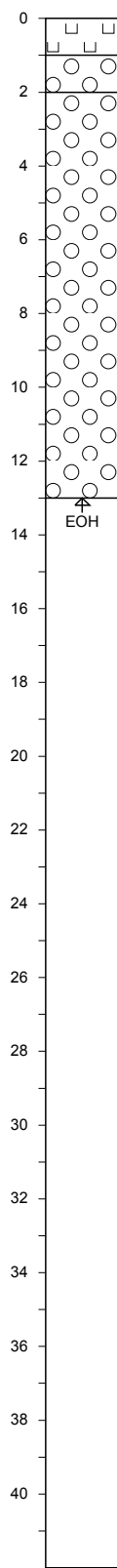
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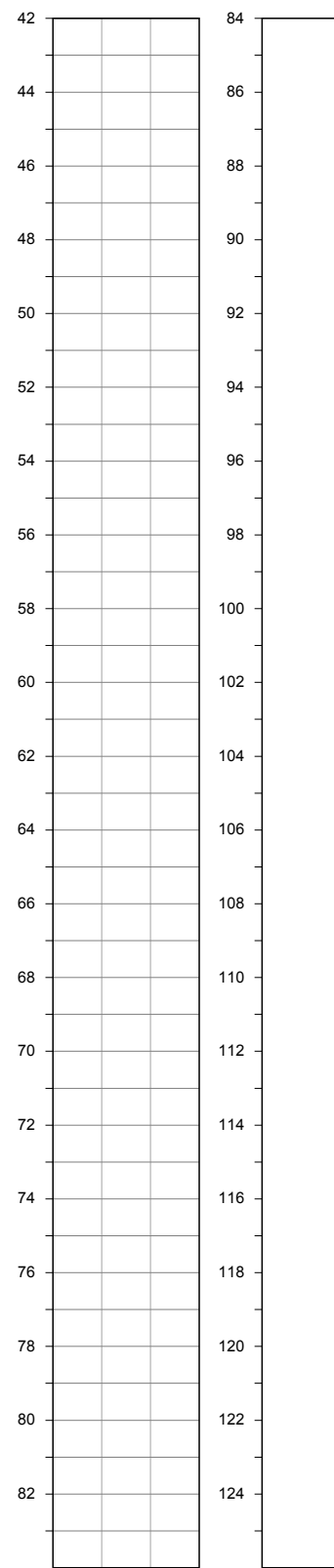
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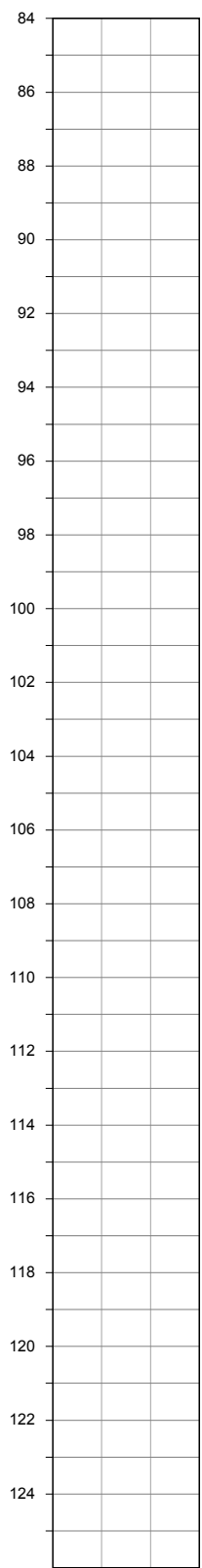
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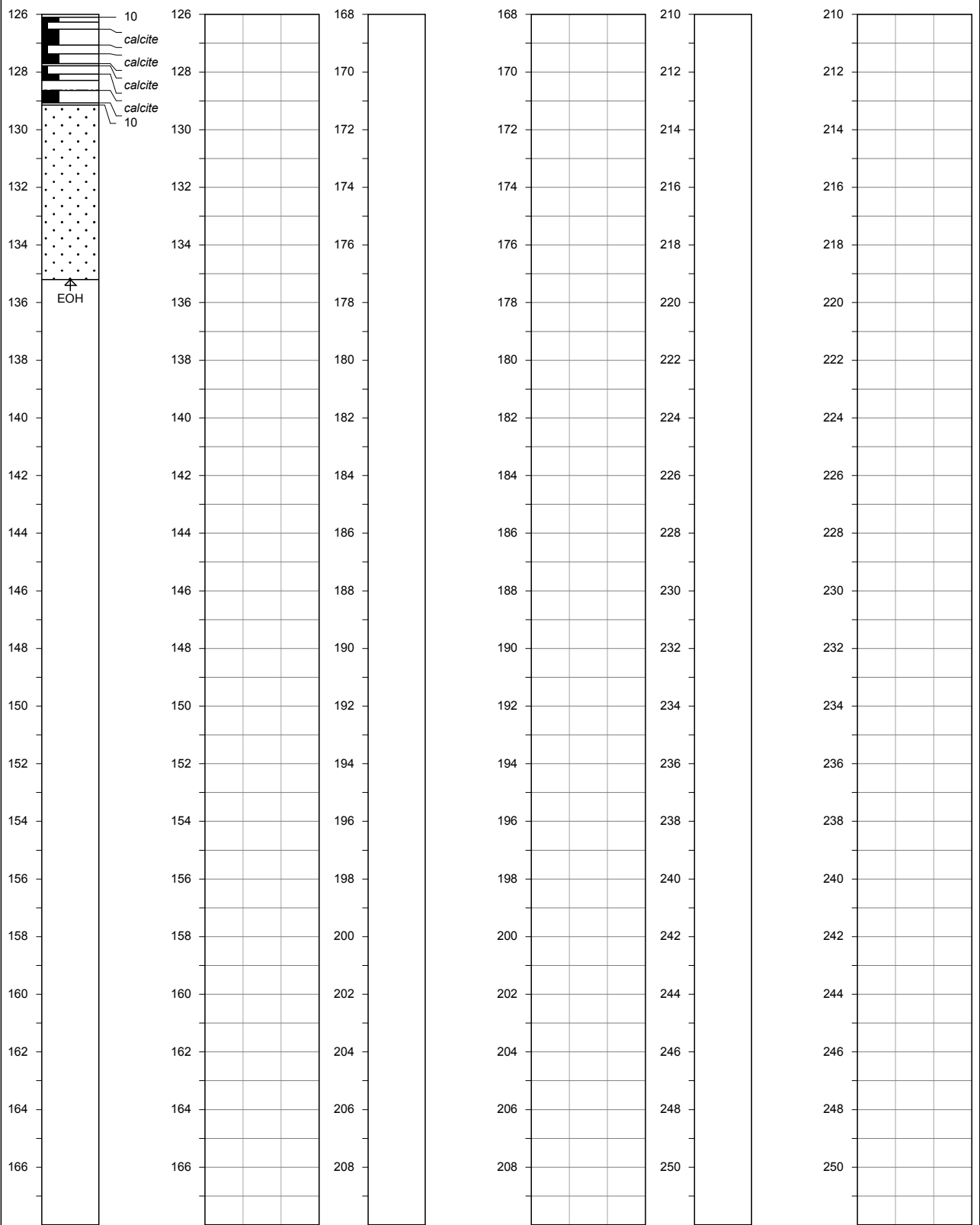
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 1.0 DENB 3.0



Vickery
 TR0025



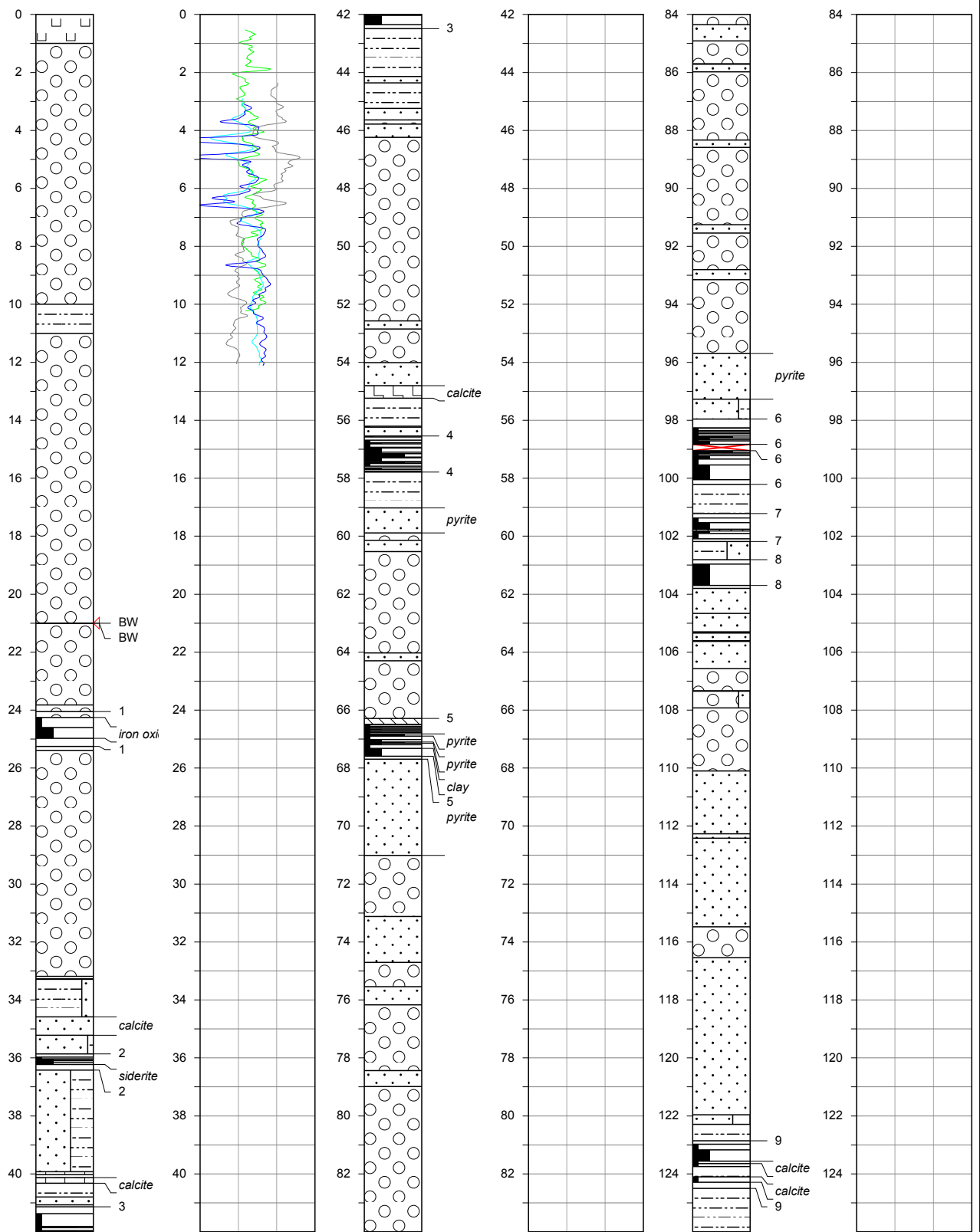
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0.0 GRDE 275.
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 1.0 DENL 3.0
 1.0 DENB 3.0



Vickery
 TR0027



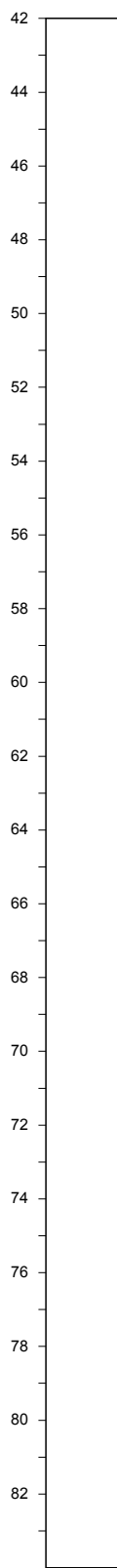
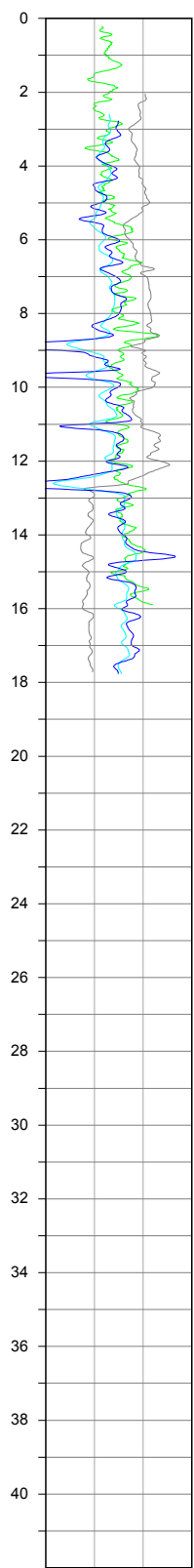
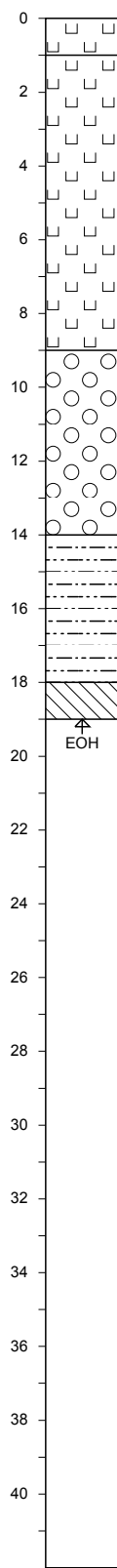
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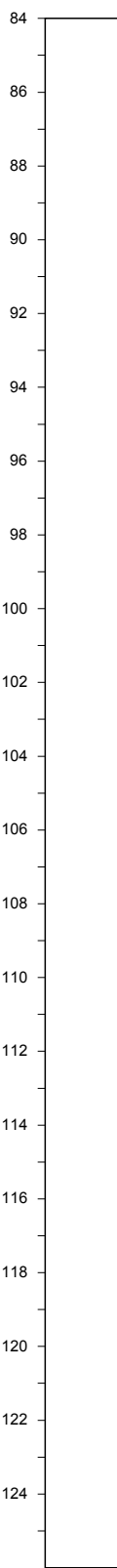
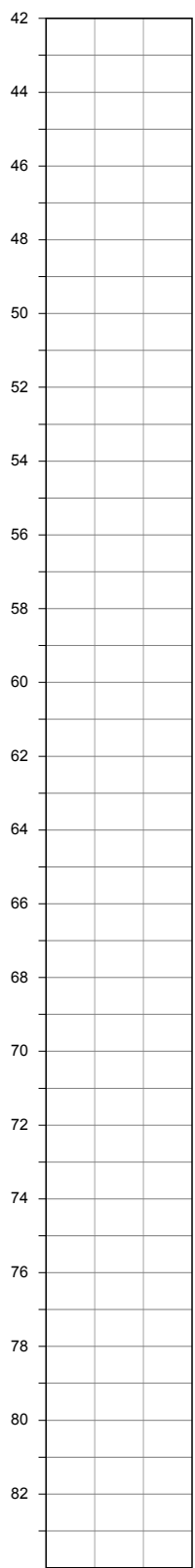
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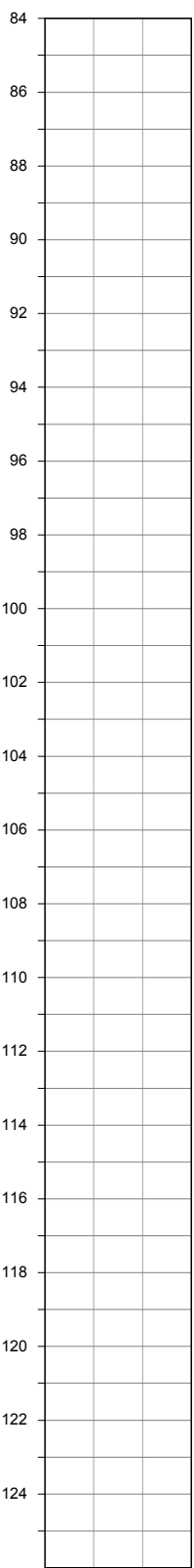
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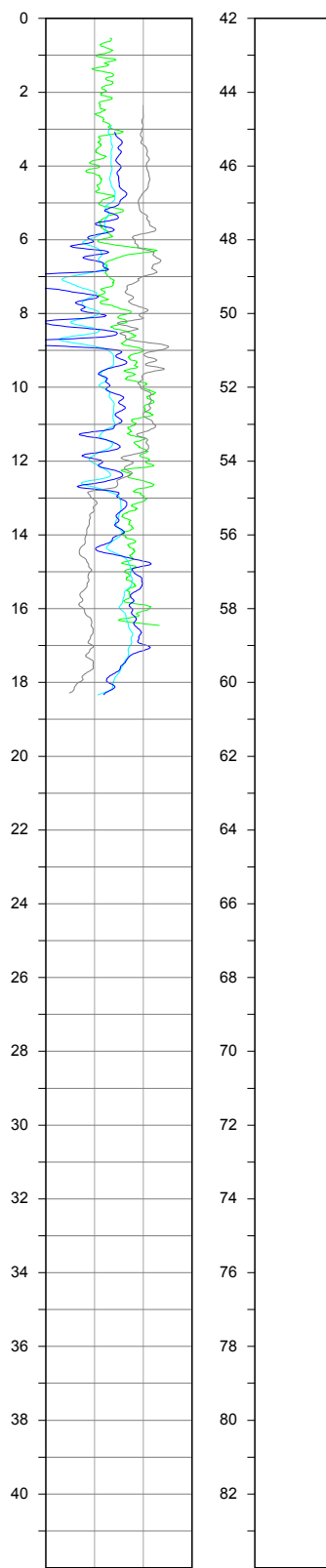
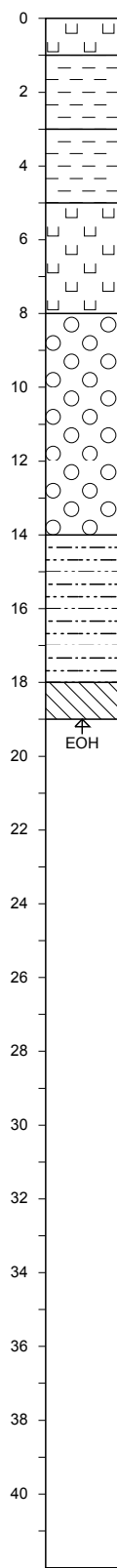
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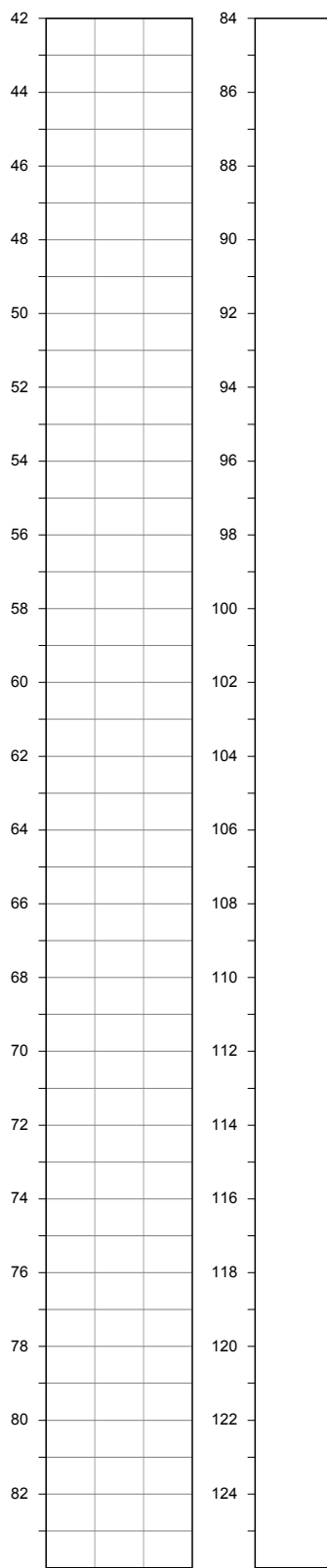
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 1.0 DENB 3.0



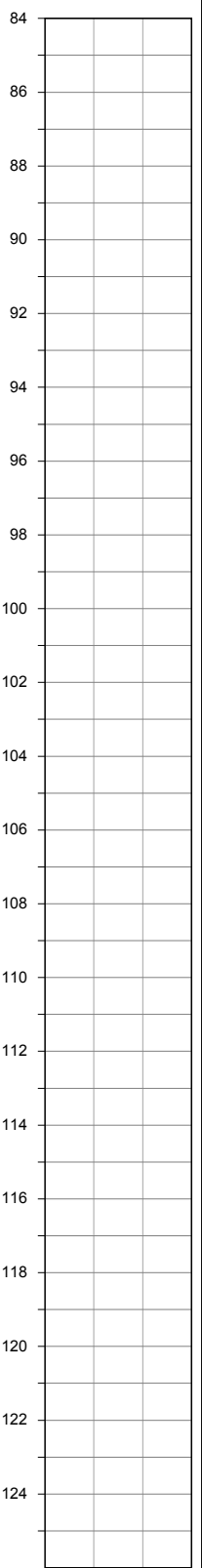
Vickery
 TR0028



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 1.0 DENB 3.0



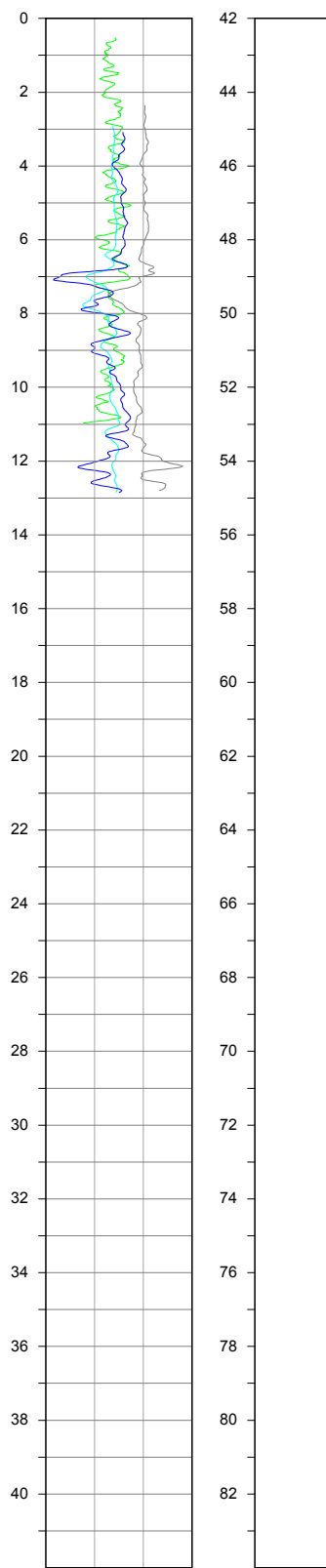
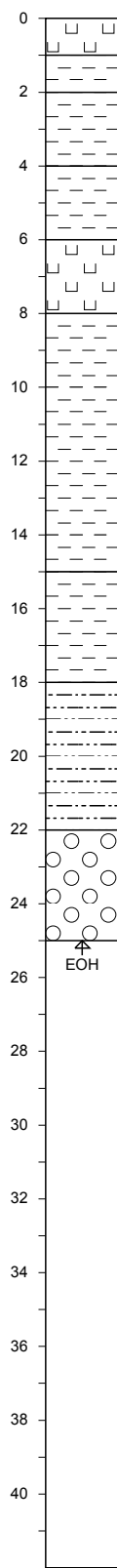
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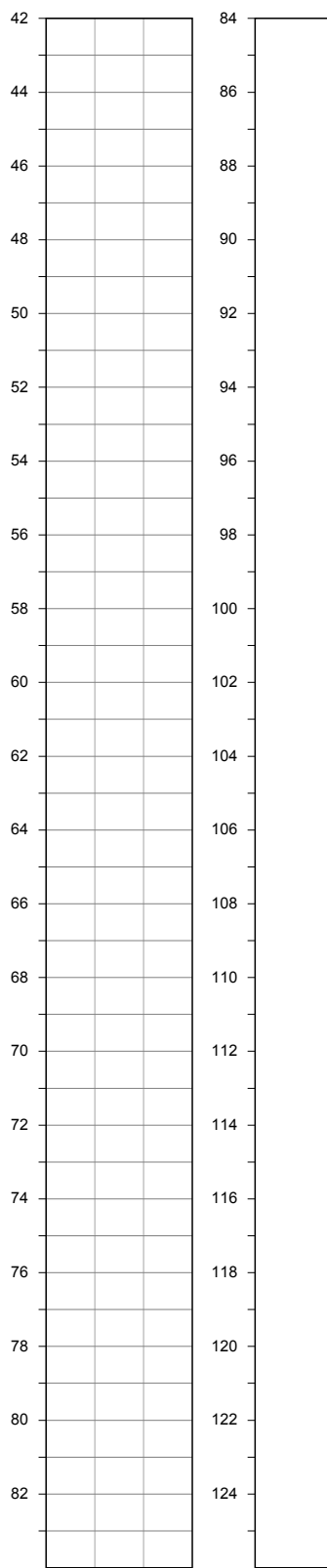
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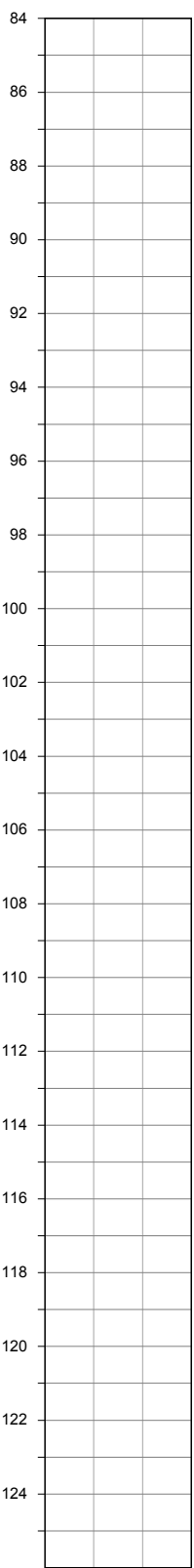
Vickery
 TR0029



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 1.0 DENB 3.0



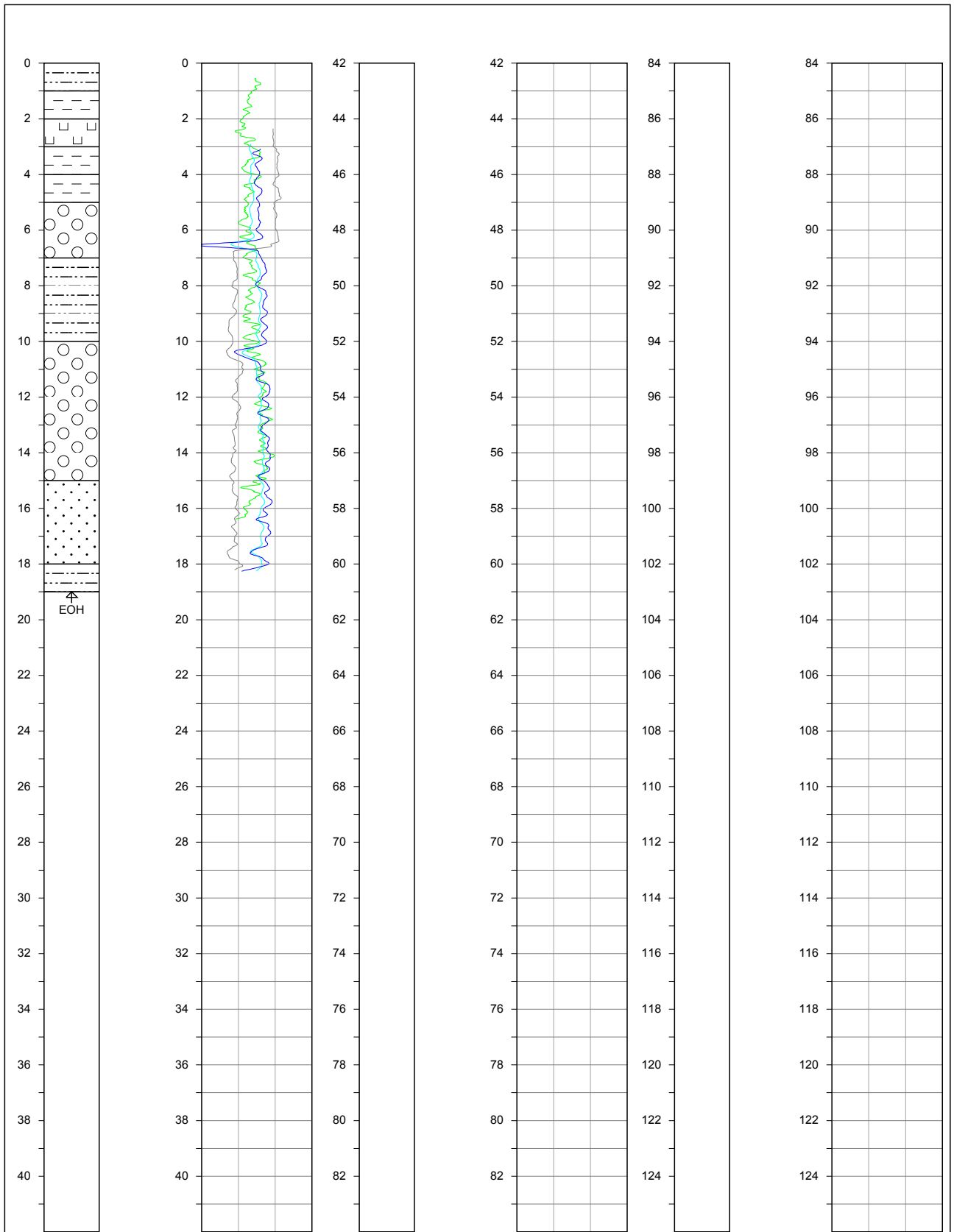
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Vickery
 TR0030



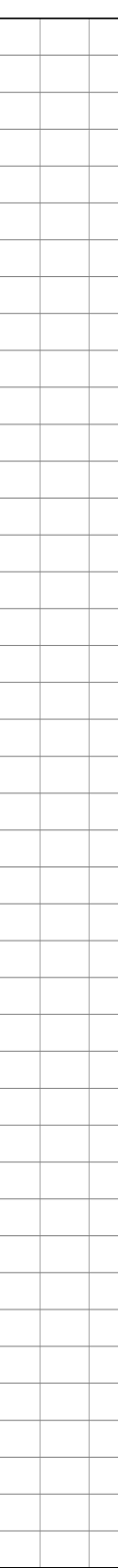
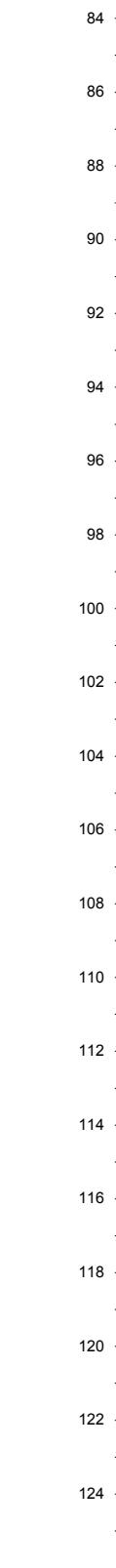
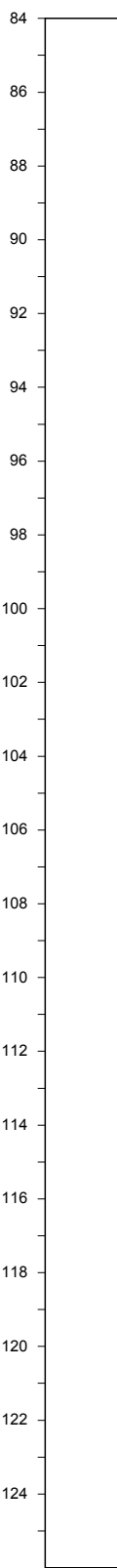
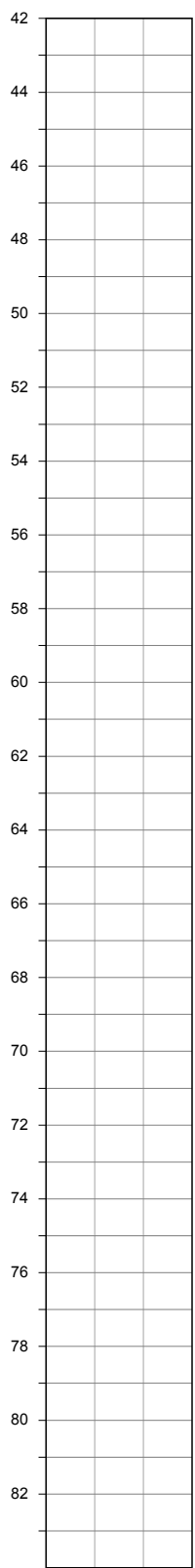
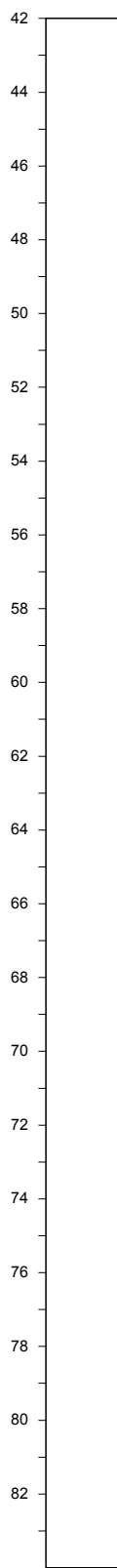
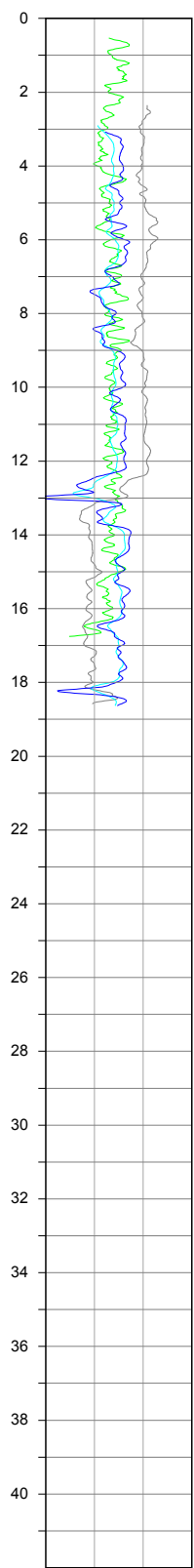
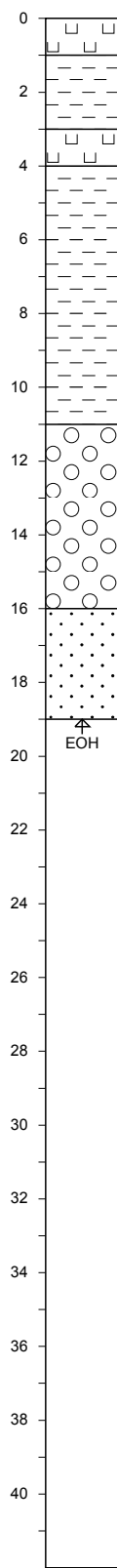
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 1.0 DENB 3.0

0.0 GRDE 275.
 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0

0.0 GRDE 275.
 90.0 CADE 200.
 1.0 DENL 3.0
 1.0 DENB 3.0



Vickery
 TR0032



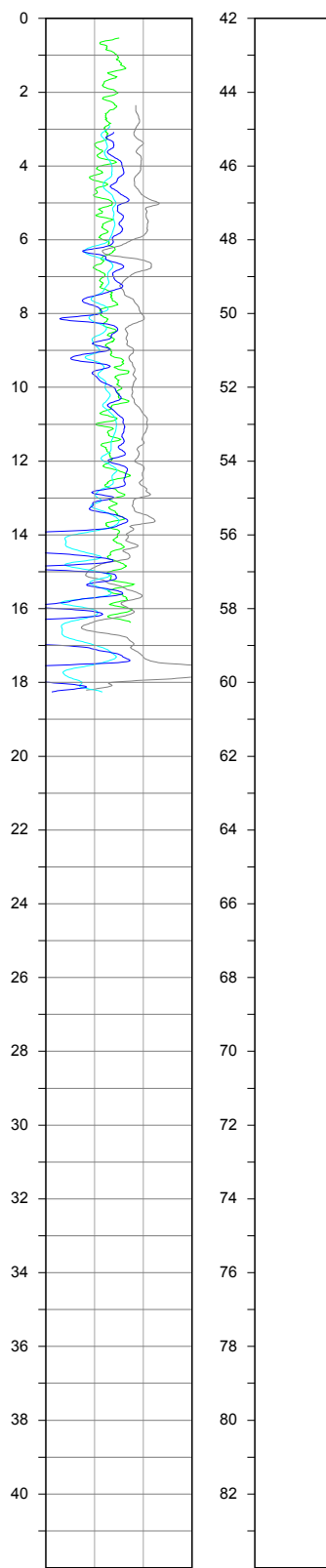
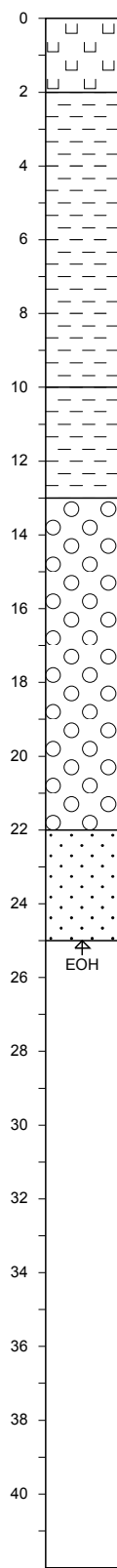
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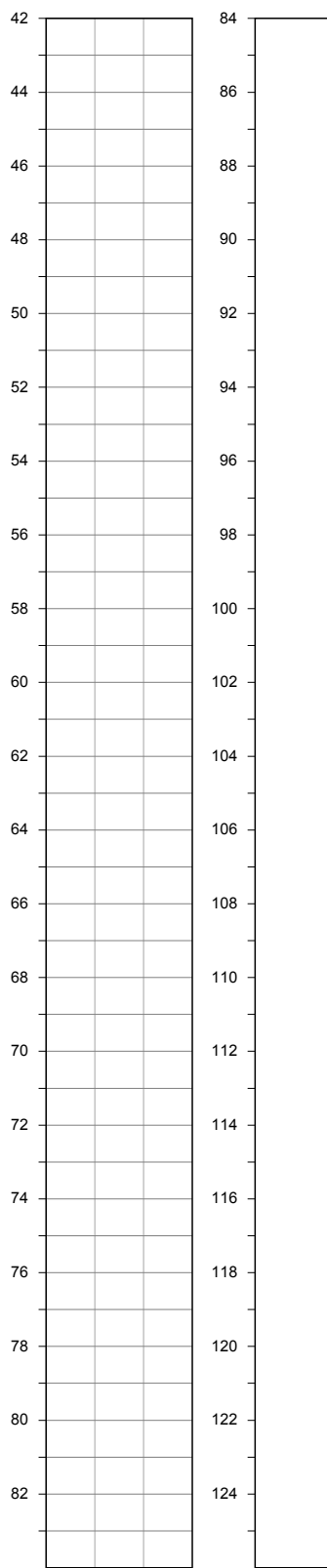
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 1.0 DENB 3.0



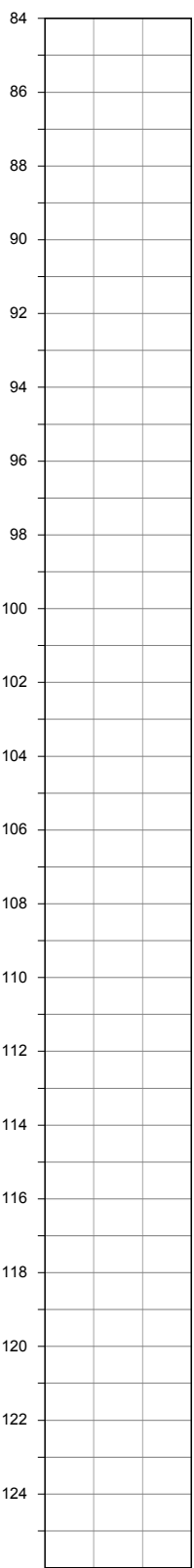
Vickery
 TR0033



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 1.0 DENB 3.0



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0.0 GRDE 275.
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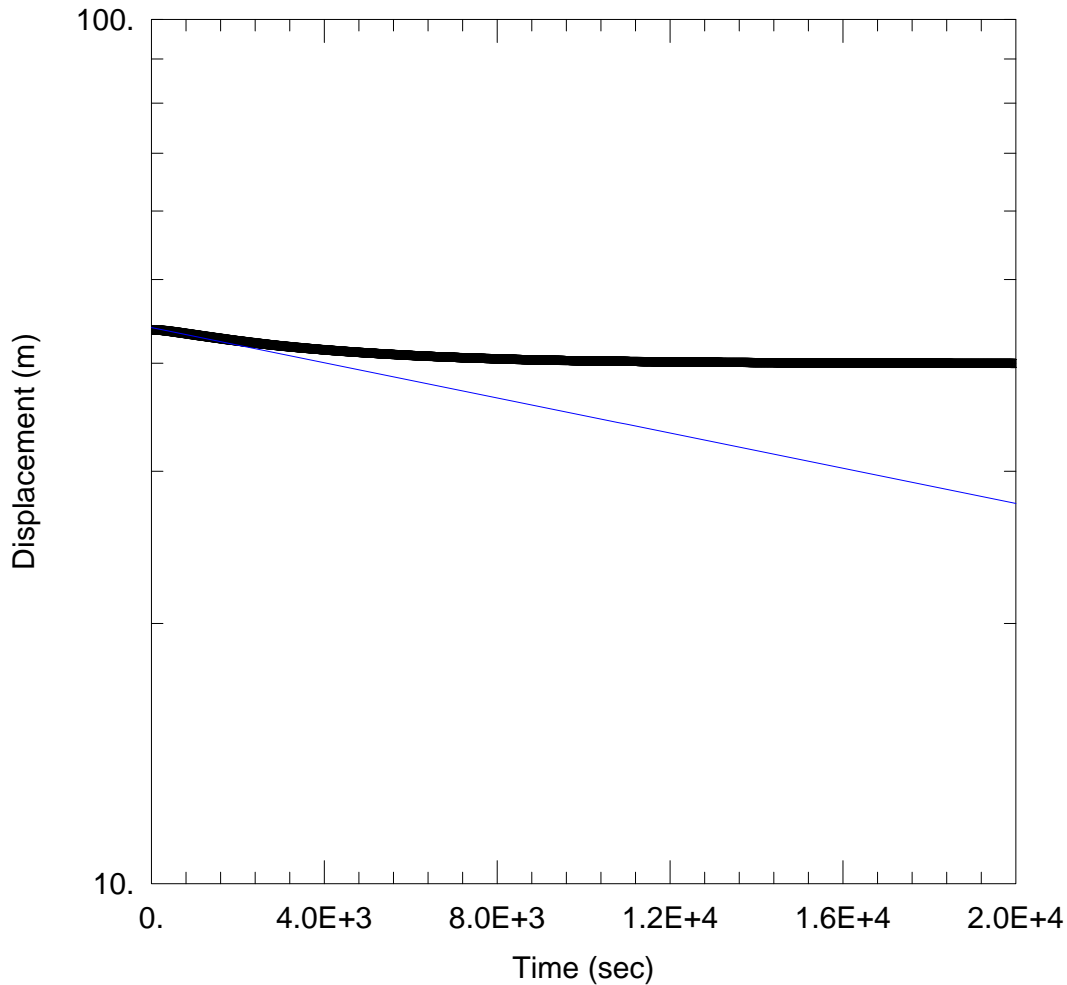


Vickery
 TR0034

Table C1: Upper Namoi Alluvium / Weathered Permian Strata Drillhole Details

Hole ID	Easting (MGA)	Northing (MGA)	Bore Depth (m)
TR1	232431	6589763	24
TR3	232435	6589563	36
TR4	232427	6589811	21
TR05	232432.49	6589953.56	31
TR06	232435.56	6590044.62	25
TR07	232933.73	6589766.46	36
TR08	233429.58	6589765.81	42
TR09	233431.1	6589977.15	37
TR10	233430.72	6590094.33	25
TR11	233426.72	6590333.85	19
TR12	232939.05	6590100.65	30
TR13	232942.97	6590279.33	19
TR14	232942.12	6589979.23	37
TR15	232939.49	6589895.91	31
TR16	233434.71	6590213.8	31
TR17	233426	6590390	19
TR18	233430.14	6590029.2	23
TR20	232940	6590600	25
TR21	233461	6590687	31
TR22	233460	6590795	25
TR23	232441	6590633	13
TR24	232475	6590405	13
TR25	232429	6590154	13
TR26	232438.49	6590095.19	19
TR27	234533.5	6590403.81	13
TR28	234499.4	6590195.21	19
TR29	234503.44	6590000.66	19
TR30	234502.56	6589797.84	25
TR31	234502.49	6589902.69	25
TR32	234047.87	6590252.88	19
TR33	234162.6	6590228.44	19
TR34	233740.23	6590278.27	25
TR35	233848.31	6590279.05	25

APPENDIX D
HYDRAULIC TEST RESULTS



WELL TEST ANALYSIS

Data Set: F:\Vick\Vickery\VKY35.aqt
 Date: 06/25/12

Time: 16:00:54

PROJECT INFORMATION

Test Well: VKY35

AQUIFER DATA

Saturated Thickness: 10. m

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (New Well)

Initial Displacement: 3.7 m
 Total Well Penetration Depth: 79. m
 Casing Radius: 0.025 m

Static Water Column Height: 39. m
 Screen Length: 10. m
 Well Radius: 0.025 m

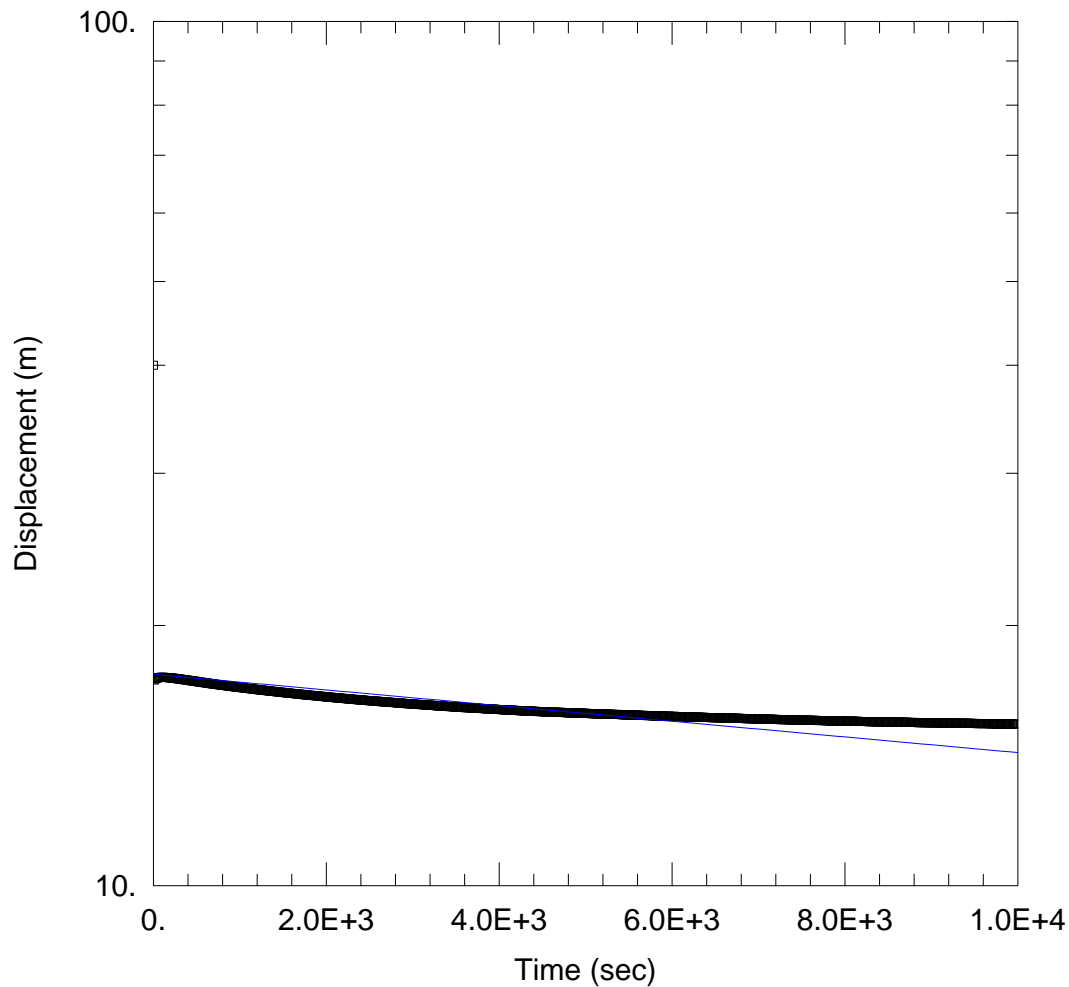
SOLUTION

Aquifer Model: Confined

Solution Method: Hvorslev

K = 3.882E-9 m/sec

y0 = 44. m



WELL TEST ANALYSIS

Data Set: C:\Andy\Jobs\Vickery\Data\Hydraulic Tests\VKY34.aqt
 Date: 08/01/12 Time: 08:11:30

PROJECT INFORMATION

Company: GES
 Client: Whitehave
 Location: Vickery
 Test Well: VKY34

AQUIFER DATA

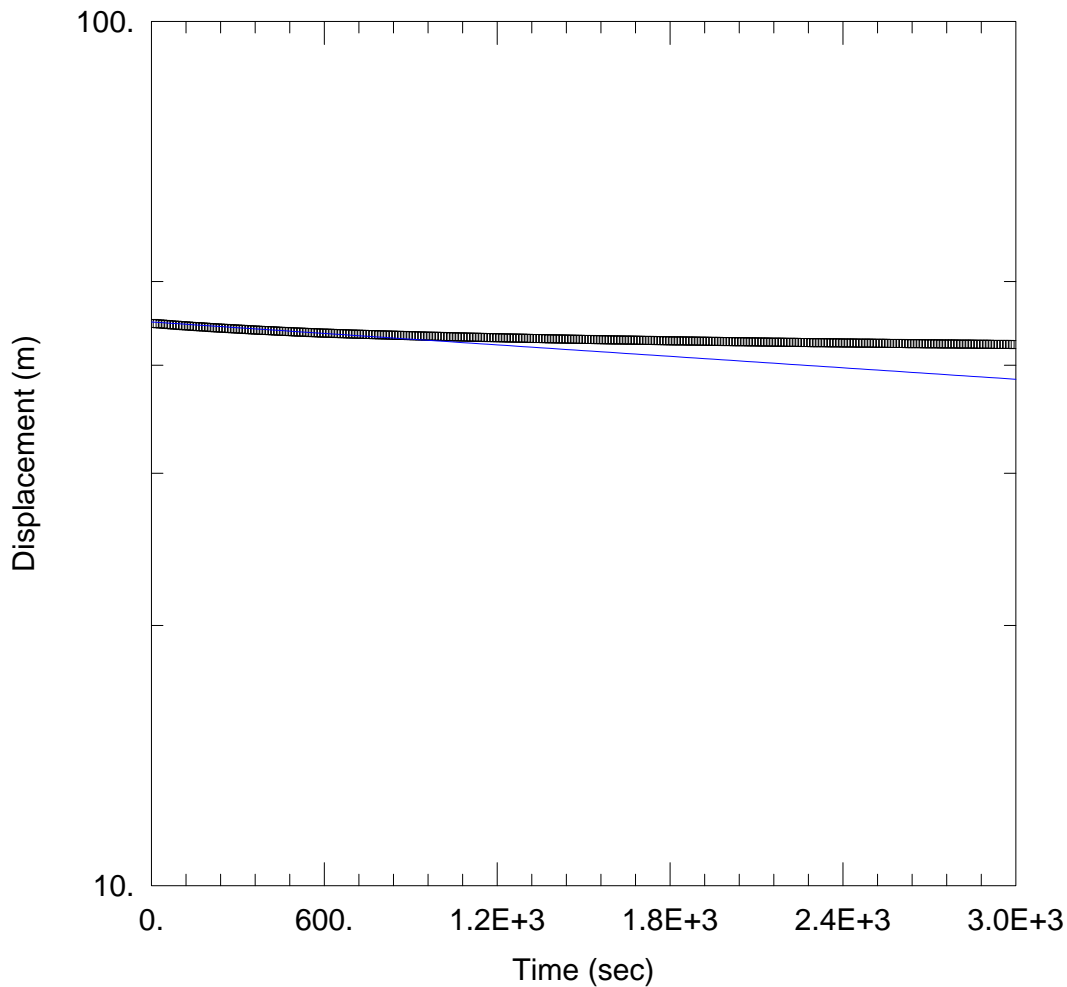
Saturated Thickness: 100. m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (New Well)

Initial Displacement: 40. m Static Water Column Height: 100. m
 Total Well Penetration Depth: 140. m Screen Length: 20. m
 Casing Radius: 0.025 m Well Radius: 0.025 m

SOLUTION

Aquifer Model: Confined Solution Method: Hvorslev
 K = 2.406E-9 m/sec y0 = 17.56 m



WELL TEST ANALYSIS

Data Set: F:\Vick\Vickery\VKY36.aqt
 Date: 06/25/12

Time: 16:02:06

PROJECT INFORMATION

Test Well: VKY36

AQUIFER DATA

Saturated Thickness: 10. m

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (VKY36)

Initial Displacement: 4.4 m
 Total Well Penetration Depth: 117. m
 Casing Radius: 0.025 m

Static Water Column Height: 77. m
 Screen Length: 10. m
 Well Radius: 0.025 m

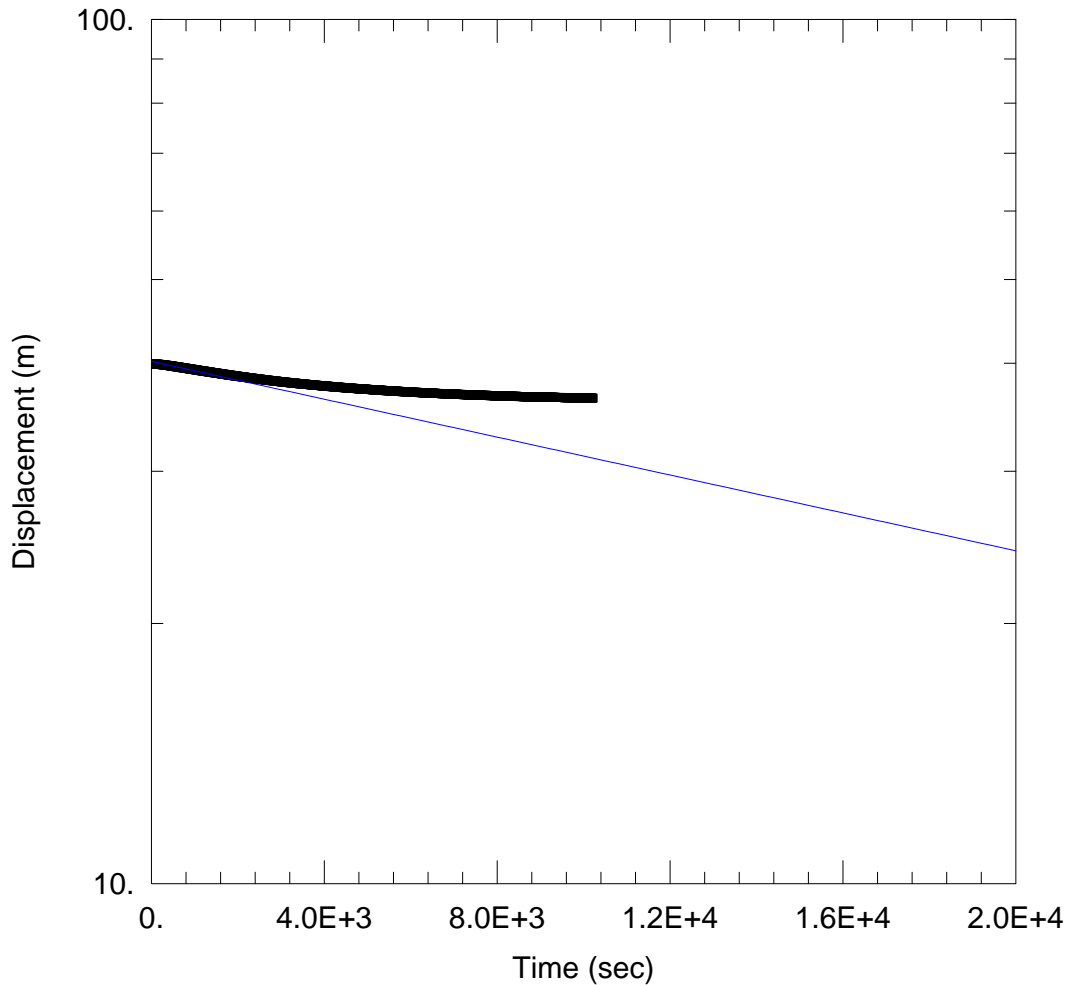
SOLUTION

Aquifer Model: Confined

Solution Method: Hvorslev

K = 8.42E-9 m/sec

y0 = 44.89 m



WELL TEST ANALYSIS

Data Set: F:\Vick\Vickery\VKY42.aqt
 Date: 06/25/12

Time: 16:02:34

PROJECT INFORMATION

Test Well: VKY42

AQUIFER DATA

Saturated Thickness: 20. m

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (New Well)

Initial Displacement: 3.5 m
 Total Well Penetration Depth: 110. m
 Casing Radius: 0.025 m

Static Water Column Height: 70. m
 Screen Length: 20. m
 Well Radius: 0.025 m

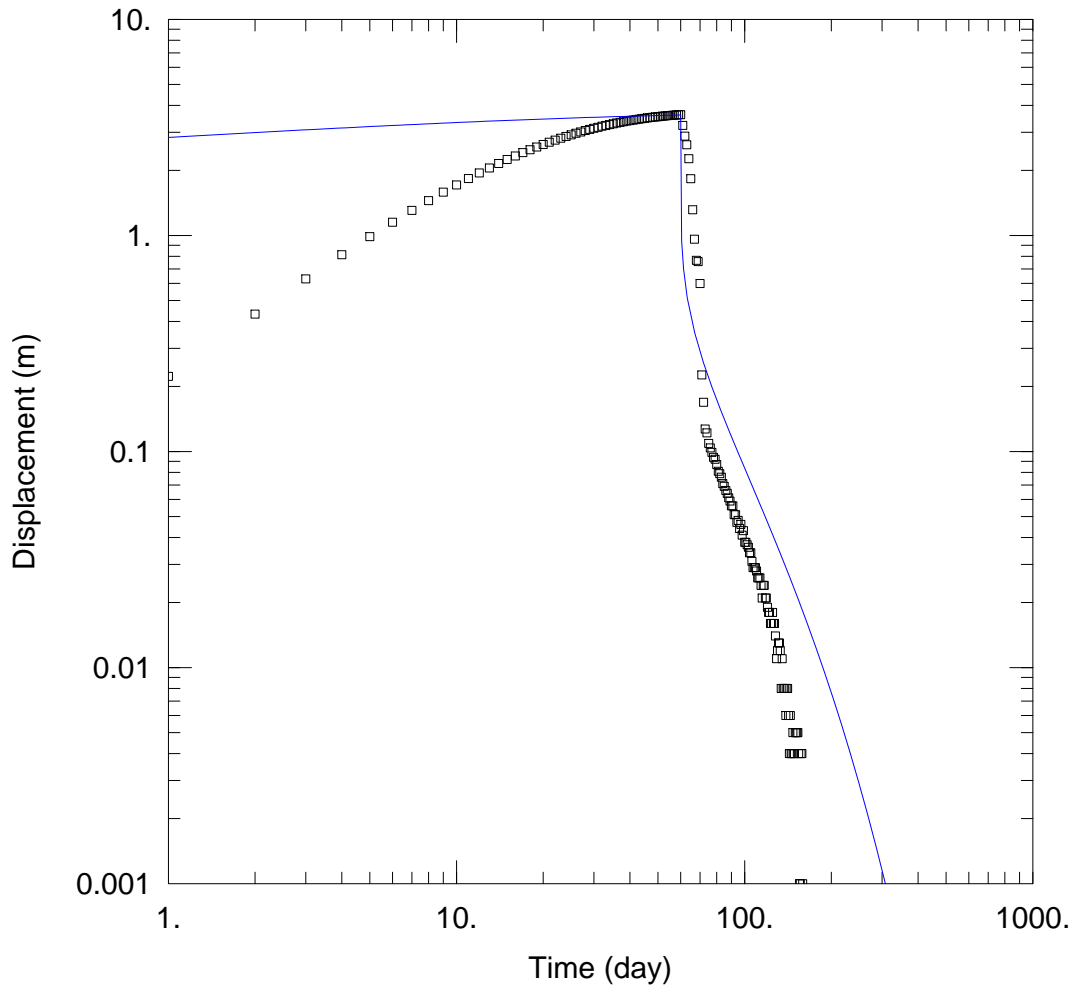
SOLUTION

Aquifer Model: Confined

Solution Method: Hvorslev

K = 2.09E-9 m/sec

y0 = 40.21 m



WELL TEST ANALYSIS

Data Set: C:\Andy\Jobs\Vickery\Data\Hydraulic Tests\TR7 Recovery.aqt
 Date: 07/31/12 Time: 11:51:40

PROJECT INFORMATION

Company: GES
 Client: Whitehaven
 Location: Vickery
 Test Well: TR7

WELL DATA

Pumping Wells

Well Name	X (m)	Y (m)
New Well	0	0

Observation Wells

Well Name	X (m)	Y (m)
□ New Well	0	0

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush-Jacob

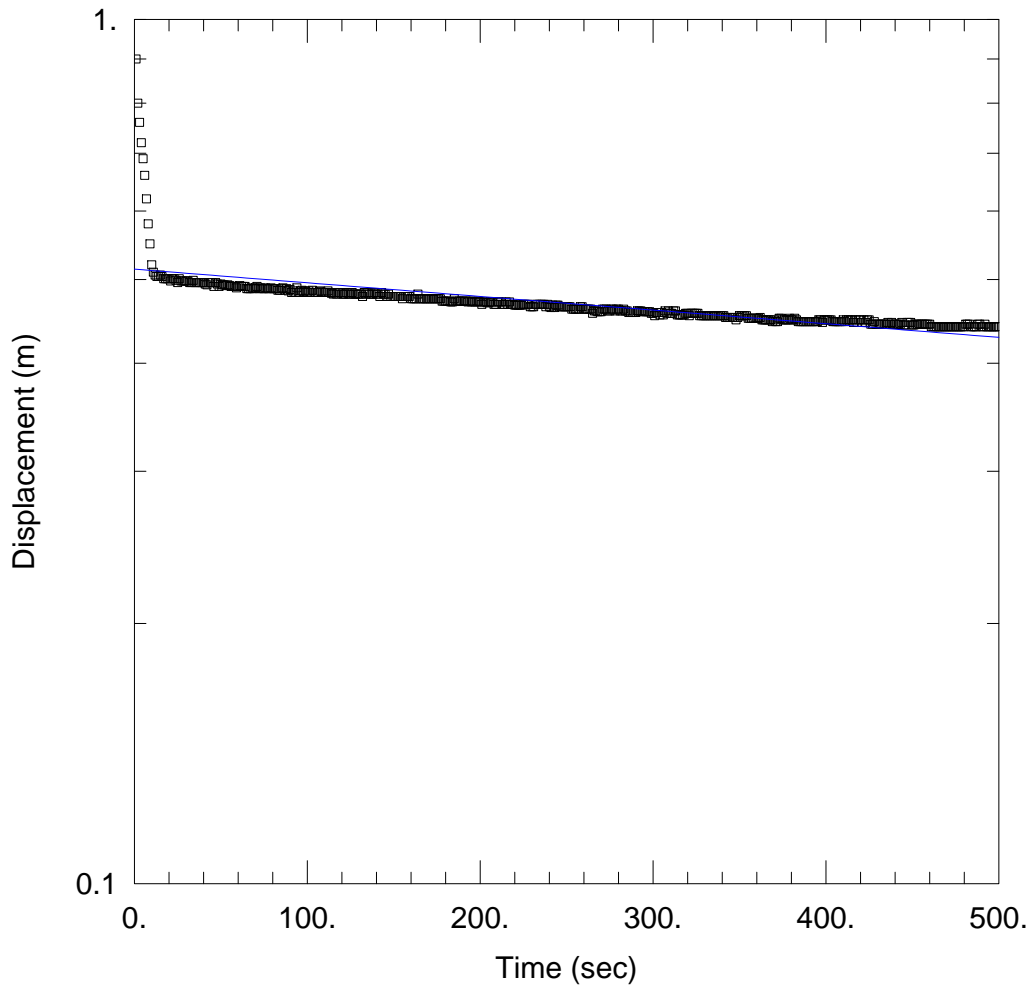
T = 7.089 m²/day

S = 0.08023

r/B = 0.0003162

Kz/Kr = 0.3633

b = 8. m



WELL TEST ANALYSIS

Data Set: C:\Andy\Jobs\Vickery\Data\Hydraulic Tests\TR18.aqt
 Date: 07/31/12 Time: 14:59:42

PROJECT INFORMATION

Company: GES
 Client: Whitehaven
 Location: Vickery
 Test Well: TR18

AQUIFER DATA

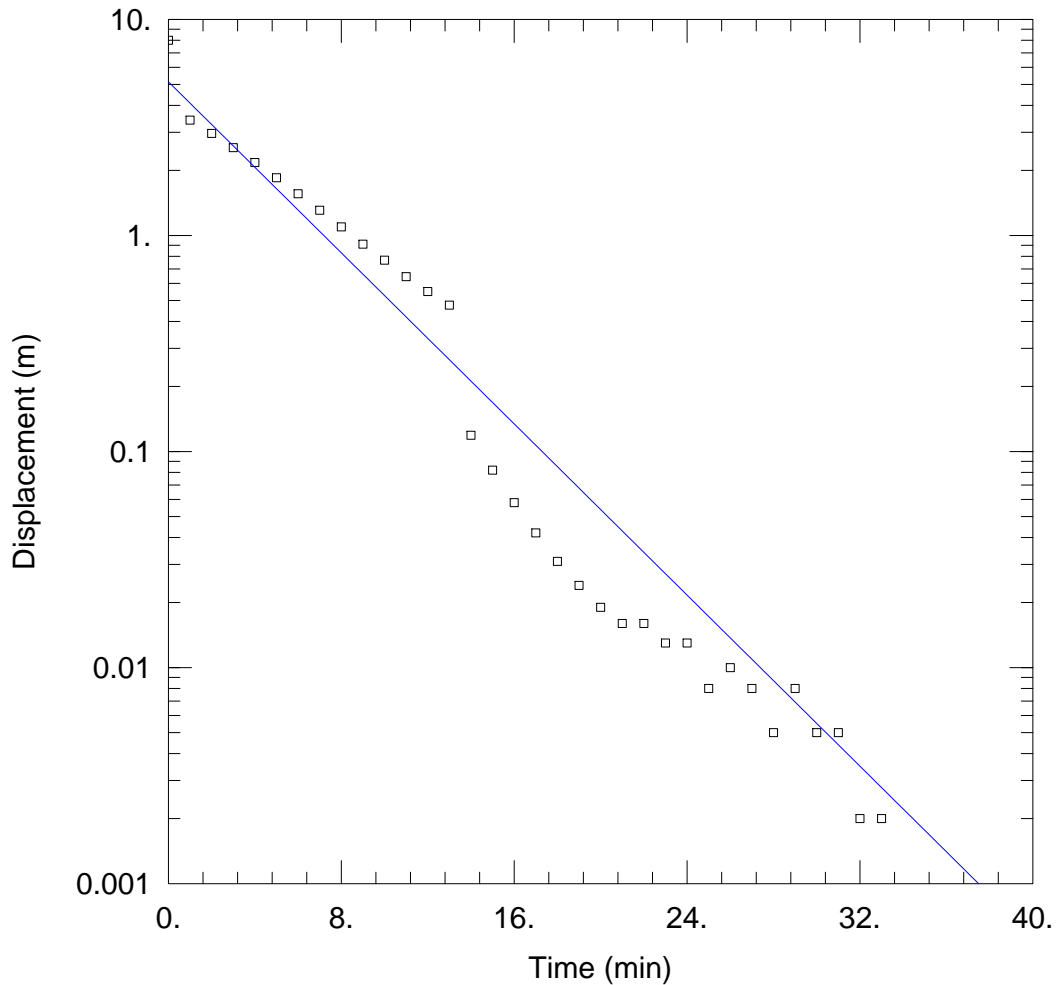
Saturated Thickness: 9. m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (New Well)

Initial Displacement: 13. m Static Water Column Height: 9. m
 Total Well Penetration Depth: 22. m Screen Length: 3. m
 Casing Radius: 0.025 m Well Radius: 0.025 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice
 K = 1.859E-7 m/sec y0 = 0.514 m



WELL TEST ANALYSIS

Data Set: C:\Andy\Jobs\Vickery\Data\Hydraulic Tests\TR26b.aqt
 Date: 07/31/12 Time: 12:21:57

PROJECT INFORMATION

Test Well: TR26

AQUIFER DATA

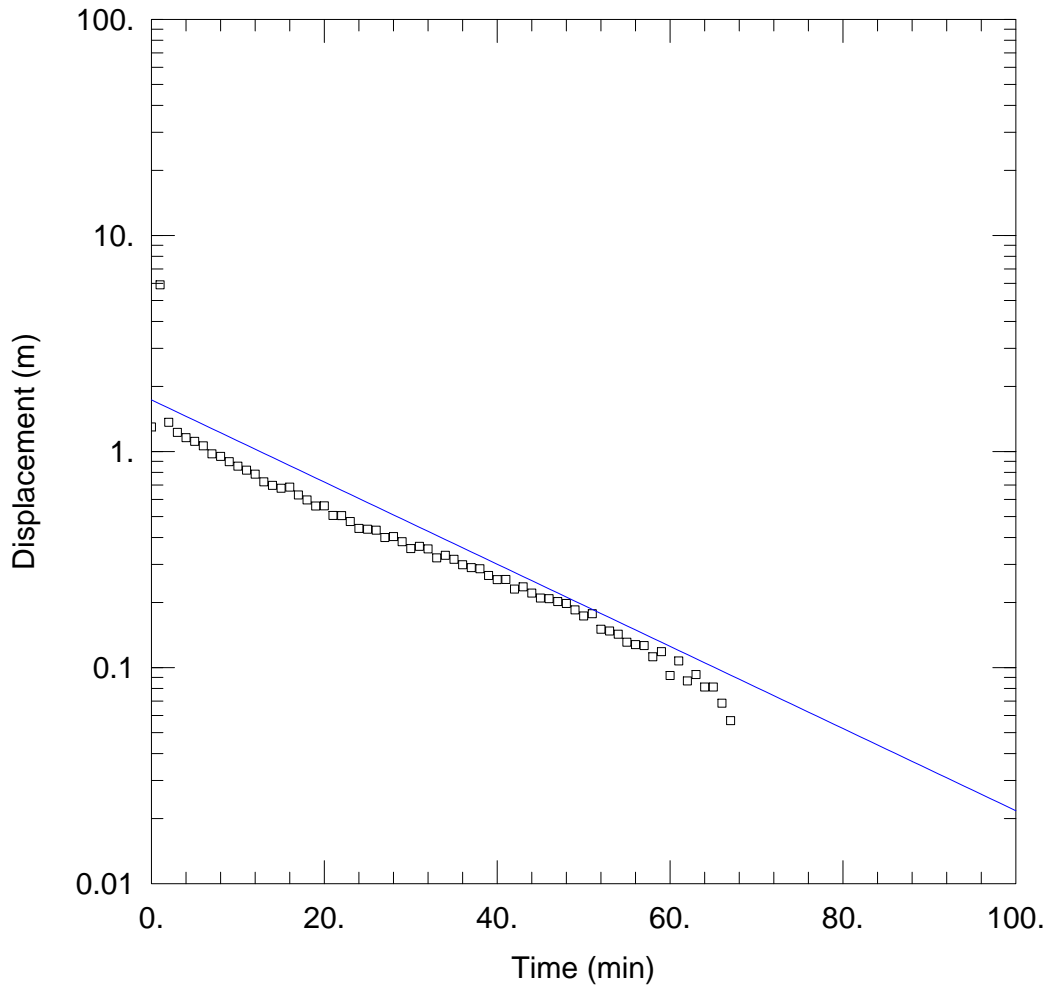
Saturated Thickness: 16. m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (New Well)

Initial Displacement: 8. m Static Water Column Height: 10. m
 Total Well Penetration Depth: 18. m Screen Length: 3. m
 Casing Radius: 0.025 m Well Radius: 0.025 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice
 K = 0.000114 m/min y0 = 5.144 m



WELL TEST ANALYSIS

Data Set: C:\Andy\Jobs\Vickery\Data\Hydraulic Tests\TRT35 Unconfined.aqt
 Date: 07/31/12 Time: 12:23:19

PROJECT INFORMATION

Test Well: TR35

AQUIFER DATA

Saturated Thickness: 5. m Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (New Well)

Initial Displacement: 1.3 m Static Water Column Height: 6. m
 Total Well Penetration Depth: 24. m Screen Length: 5. m
 Casing Radius: 0.025 m Well Radius: 0.025 m

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice
 K = 1.405E-5 m/min y0 = 1.733 m

APPENDIX E PUMPING TEST LICENCE DETAILS



NEW SOUTH WALES
NSW Government

NSW OFFICE OF WATER

**Whitehaven Coal Mining Pty Limited
PO Box 600
GUNNEDAH NSW 2380**

11th July, 2012 .

Dear Sir,

MONITORING BORE LICENCE

Please find enclosed your licence(s). Together with a blank **"Form A" (Particulars of Completed bore)** and map for recording details of the bore. Also attached is a copy of the new procedure for the handling of the **"Form A"**.

Your attention is drawn to the nature and description of the work, terms, limitations and conditions under which the licence(s) is/are issued.

You are able to sink multiple test bores under this Test License.

Please show the licence to the Driller so that he is aware of any conditions affecting the construction of the bore. The driller must have a current Driller's Licence issued by this Department.

Condition (2) of the license applies whether the bore is successful or not. The attached **"Form A"** has been provided for the recording of details of the proposed bore. The driller is required to record the details of the bore, and **provide you** with the **completed form**. The driller is also required to mark the location of the bore site on the attached plan. This sketch is required even though you may have already indicated the site to the Department.

You are required to return the completed "Form A" and map to the contact officer and address shown in this letter. Failure to do so could restrict future actions relating to the licensed work, such as the future conversion from a Water Act license to a Water Management Act approval.

If your monitoring bore is to be converted into a production bore, it will be necessary to apply and obtain a new licence. The following should be kept in mind when selecting the site of the test bore, if it is to be used for any purpose other than stock, domestic and farming:-

The bore must be located:

- ** 200 metres from any boundary of the property
- ** 400 metres from any irrigation bore on an adjoining property
- ** 500 metres from any town water supply bore
- ** 400 metres from any Department Water Monitoring Bore
- ** 40 metres from the nearest bank of any river or creek
- ** 400 metres from any Artesian Bore on an adjoining property
- ** 200 metres from any wetland or other nature/conservation area

And any relevant distance conditions as set out in the Water Sharing Plans for your specific area.

Yours faithfully



Karen Schubert
Licensing Division
Area North
Moree Office

NSW Office of Water

Licensing North
Po Box 550
155-157 Marius Street
Tamworth NSW 2340
Phone: (02) 67019600

BORE LICENSE CERTIFICATE
UNDER SECTION 115 OF THE WATER ACT, 1912

90BL256080



Whitehaven Coal Mining Pty Ltd
Po Box 600
Gunnedah NSW 2380

LICENSE NUMBER
90BL256080
DATE LICENSE VALID FROM
11-Jul-2012
DATE LICENSE VALID TO
PERPETUITY
FEE
\$0.00

ABN 47661556763 GST NIL

LOCATION OF WORKS		
Portion(s) or Lot/Section/DP	PARISH	COUNTY
2//219923	Brentry	Nandewar

TYPE OF WORKS	PURPOSE(S) FOR WHICH WATER MAY BE USED
Bore	Monitoring Bore

CONDITIONS APPLYING TO THIS LICENSE ARE

As shown on the attached Condition Statement

ORIGINAL

NSW Office of Water**CONDITIONS STATEMENT REFERRED TO ON
90BL256080
ISSUED UNDER PART V OF THE WATER ACT, 1912
ON 11-Jul-2012**

(1) THE LICENCE SHALL LAPSE IF THE WORK IS NOT COMMENCED AND COMPLETED WITHIN THREE YEARS OF THE DATE OF THE ISSUE OF THE LICENCE.

(2) THE LICENSEE SHALL WITHIN TWO MONTHS OF COMPLETION OR AFTER THE ISSUE OF THE LICENSE IF THE WORK IS EXISTING, FURNISH TO NSW OFFICE OF WATER:-

(A) DETAILS OF THE WORK SET OUT IN THE ATTACHED FORM "A" (MUST BE COMPLETED BY A DRILLER).

(B) A PLAN SHOWING ACCURATELY THE LOCATION OF THE WORK, IN RELATION TO PORTION AND PROPERTY BOUNDARIES.

(C) A ONE LITRE WATER SAMPLE FOR ALL LICENCES OTHER THAN THOSE FOR STOCK, DOMESTIC, TEST BORES AND FARMING PURPOSES.

(D) DETAILS OF ANY WATER ANALYSIS AND/OR PUMPING TESTS.

(3) THE LICENSEE SHALL ALLOW NSW OFFICE OF WATER OR ANY PERSON AUTHORISED BY IT, FULL AND FREE ACCESS TO THE WORKS, EITHER DURING OR AFTER CONSTRUCTION, FOR THE PURPOSE OF CARRYING OUT INSPECTION OR TEST OF THE WORKS AND ITS FITTINGS AND SHALL CARRY OUT ANY WORK OR ALTERATIONS DEEMED NECESSARY BY THE DEPARTMENT FOR THE PROTECTION AND PROPER MAINTENANCE OF THE WORKS, OR THE CONTROL OF THE WATER EXTRACTED AND FOR THE PROTECTION OF THE QUALITY AND THE PREVENTION FROM POLLUTION OR CONTAMINATION OF SUB-SURFACE WATER.

(4) IF DURING THE CONSTRUCTION OF THE WORK, SALINE OR POLLUTED WATER IS ENCOUNTERED ABOVE THE PRODUCING AQUIFER, SUCH WATER SHALL BE SEALED OFF BY:-

(A) INSERTING THE APPROPRIATE LENGTH(S) OF CASING TO A DEPTH SUFFICIENT TO EXCLUDE THE SALINE OR POLLUTED WATER FROM THE WORK.

(B) CEMENTING BETWEEN THE CASING(S) AND THE WALLS OF THE BORE HOLE FROM THE BOTTOM OF THE CASING TO GROUND LEVEL.

ANY DEPARTURE FROM THESE PROCEDURES MUST BE APPROVED BY THE DEPARTMENT BEFORE UNDERTAKING THE WORK.

(5) (A) THE LICENSEE SHALL NOTIFY NSW OFFICE OF WATER IF A FLOWING SUPPLY OF WATER IS OBTAINED. THE BORE SHALL THEN BE LINED WITH CASING AND CEMENTED AND A SUITABLE CLOSING GEAR SHALL BE ATTACHED TO THE BOREHEAD AS SPECIFIED BY NSW OFFICE OF WATER.

(B) IF A FLOWING SUPPLY OF WATER IS OBTAINED FROM THE WORK, THE LICENSEE SHALL ONLY DISTRIBUTE WATER FROM THE BORE HEAD BY A SYSTEM OF PIPE LINES AND SHALL NOT DISTRIBUTE IT IN DRAINS, NATURAL OR ARTIFICIAL CHANNELS OR DEPRESSIONS.

(6) IF A WORK IS ABANDONED AT ANY TIME THE LICENSEE SHALL NOTIFY NSW OFFICE OF WATER THAT THE WORK HAS BEEN ABANDONED AND SEAL OFF THE AQUIFER BY:-

(A) BACKFILLING THE WORK TO GROUND LEVEL WITH CLAY OR CEMENT AFTER WITHDRAWING THE CASING (LINING); OR

(B) SUCH METHODS AS AGREED TO OR DIRECTED BY NSW OFFICE OF WATER.

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- ANY OTHER PERSONS LAND;
- ANY CROWN LAND;
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- ANY NATIVE VEGETATION AS DESCRIBED UNDER THE NATIVE VEGETATION CONSERVATION ACT 1997;
- ANY WETLANDS OF ENVIRONMENTAL SIGNIFICANCE.

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(9) IF THE BORE AUTHORISED BY THIS LICENSE IS LINED WITH STEEL OR PLASTIC CASING THE INSIDE DIAMETER OF THAT CASING SHALL NOT EXCEED 220 MM.

(10) WATER SHALL NOT BE PUMPED FROM THE BORE AUTHORISED BY THIS LICENSE FOR ANY PURPOSE OTHER THAN GROUNDWATER INVESTIGATION.

(11) SUBJECT TO CONDITION (12) THE LICENSEE SHALL WITHIN TWO MONTHS OF THE DATE OF COMPLETION OF THE BORE AUTHORISED BY THE LICENSE,

(1) BACKFILL IT WITH CLAY OR CEMENT TO GROUND LEVEL, AFTER WITHDRAWING ANY CASING(LINING), OR:-

(2) RENDER IT INEFFECTIVE BY ANY OTHER MEANS ACCEPTABLE TO THE DEPARTMENT.

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(2) THE LICENSEE HAS COMPLETED THE BORE FOR THE PURPOSE OF MEASURING WATER LEVELS OR WATER QUALITY BY THE ADDITION OF CASING WITH A DIAMETER NOT EXCEEDING 220MM.

End Of Conditions



NEW SOUTH WALES
NSW Government

NSW OFFICE OF WATER

**Whitehaven Coal Mining Pty Limited
PO Box 600
GUNNEDAH NSW 2380**

11th July, 2012 .

Dear Sir,

MONITORING BORE LICENCE

Please find enclosed your licence(s). Together with a blank **"Form A" (Particulars of Completed bore)** and map for recording details of the bore. Also attached is a copy of the new procedure for the handling of the **"Form A"**.

Your attention is drawn to the nature and description of the work, terms, limitations and conditions under which the licence(s) is/are issued.

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BORE LICENSE CERTIFICATE
UNDER SECTION 115 OF THE WATER ACT, 1912

90BL256079



Whitehaven Coal Mining Pty Ltd
Po Box 600
Gunnedah NSW 2380

LICENSE NUMBER
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DATE LICENSE VALID FROM
11-Jul-2012
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PERPETUITY
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\$0.00
ABN 47661556763 GST NIL

LOCATION OF WORKS

<u>Portion(s) or Lot/Section/DP</u>	<u>PARISH</u>	<u>COUNTY</u>
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<u>TYPE OF WORKS</u>	<u>PURPOSE(S) FOR WHICH WATER MAY BE USED</u>
Bore	Monitoring Bore

CONDITIONS APPLYING TO THIS LICENSE ARE

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NSW Office of Water

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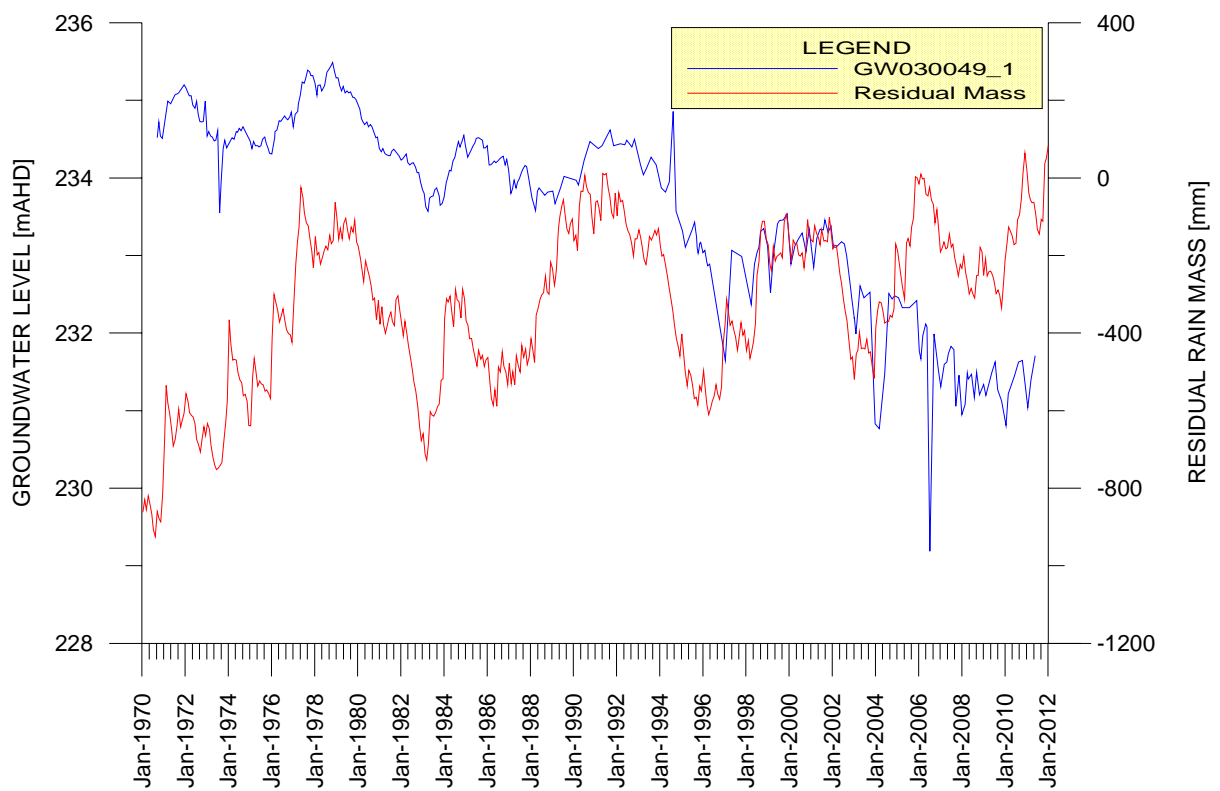
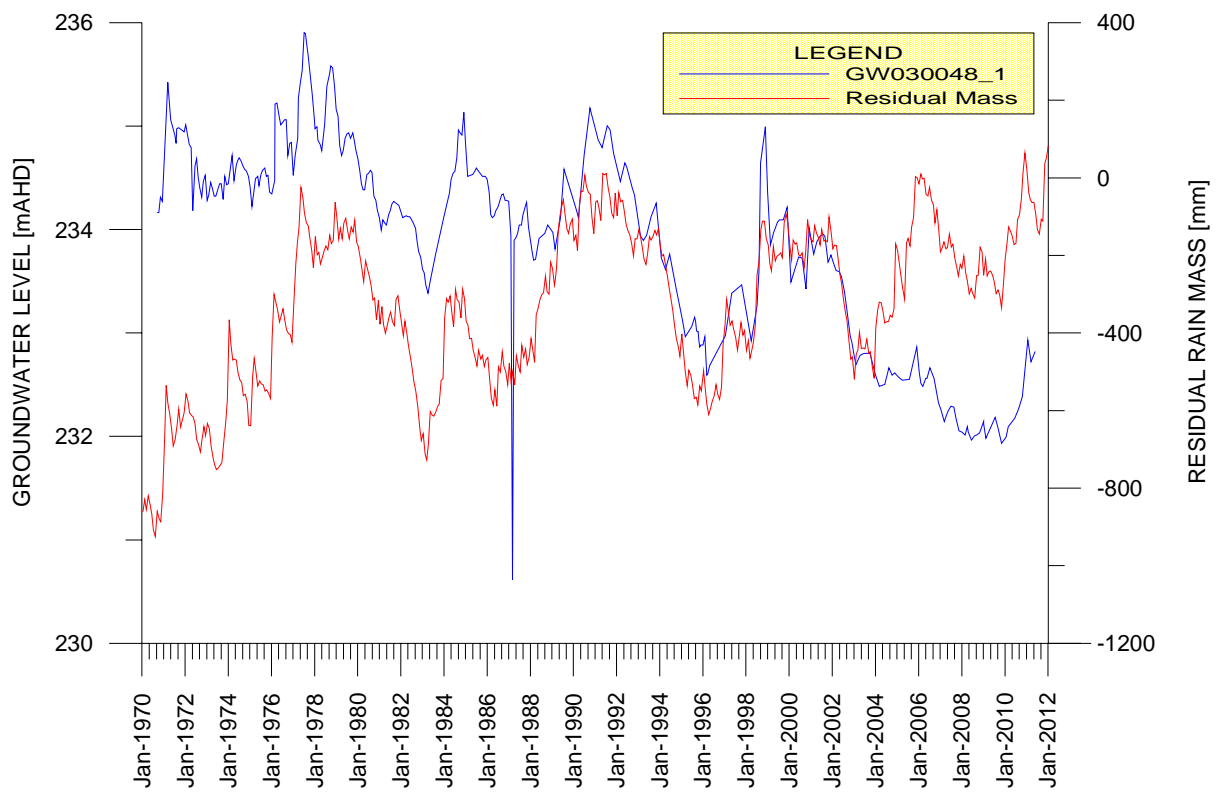
End Of Conditions

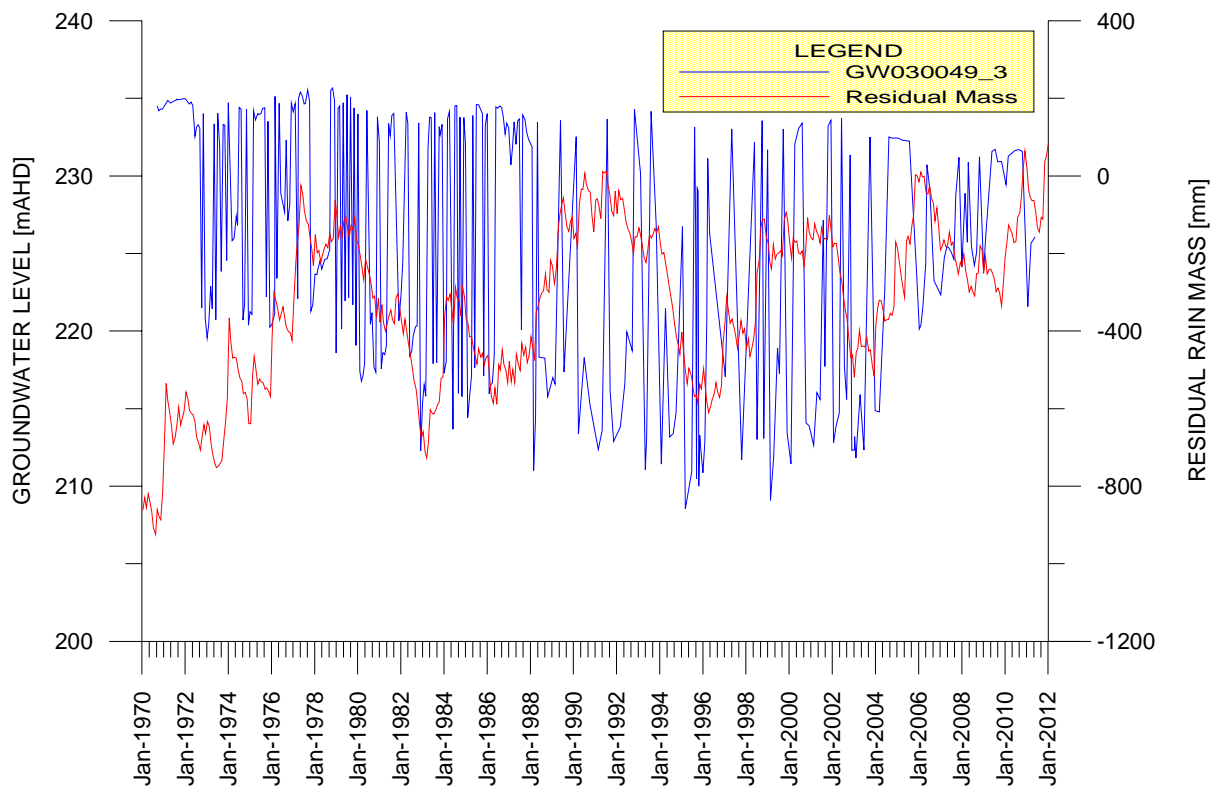
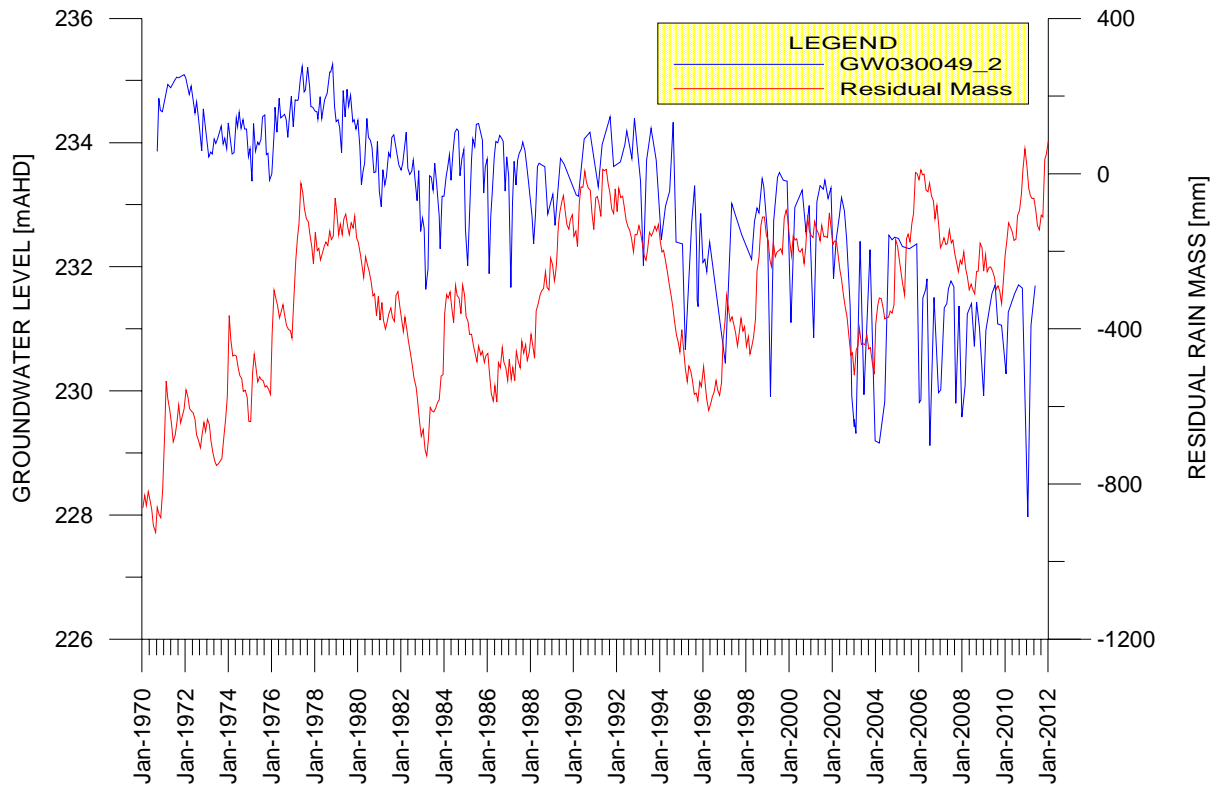
APPENDIX F
BORE VKY3092 GROUNDWATER CHEMISTRY RESULTS

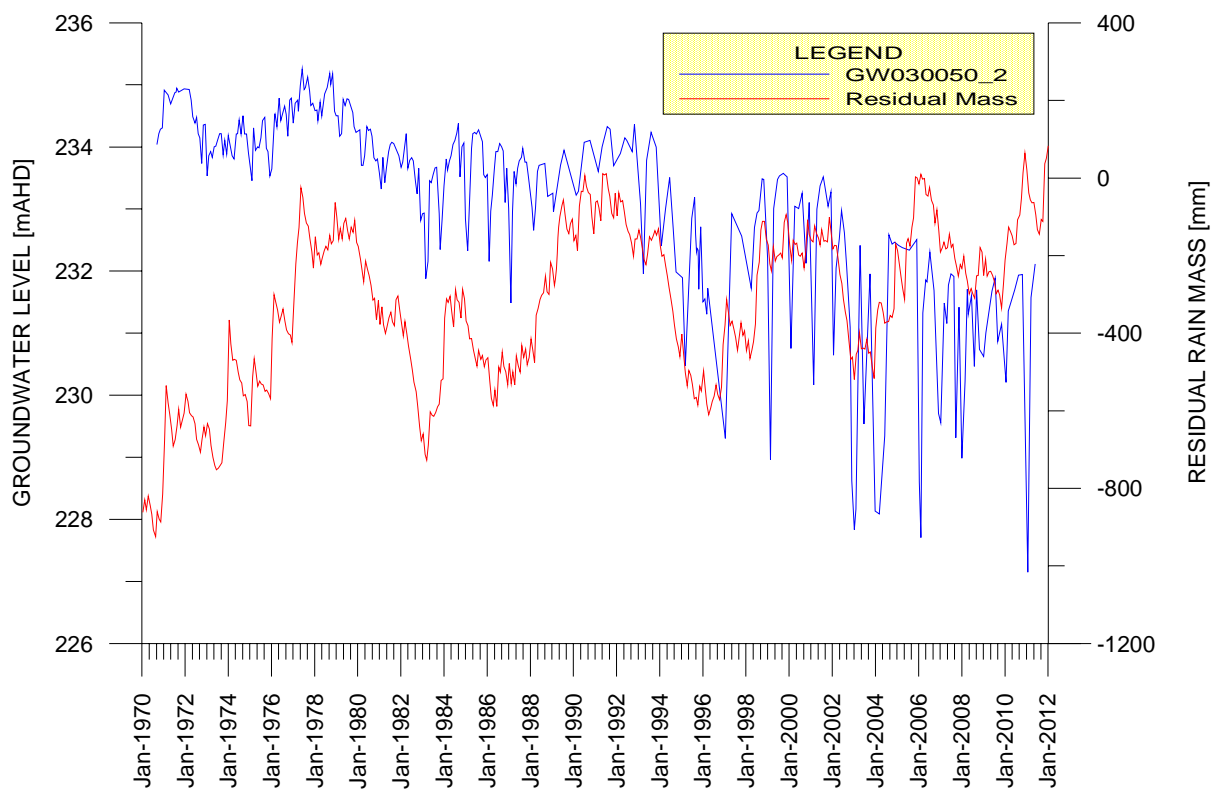
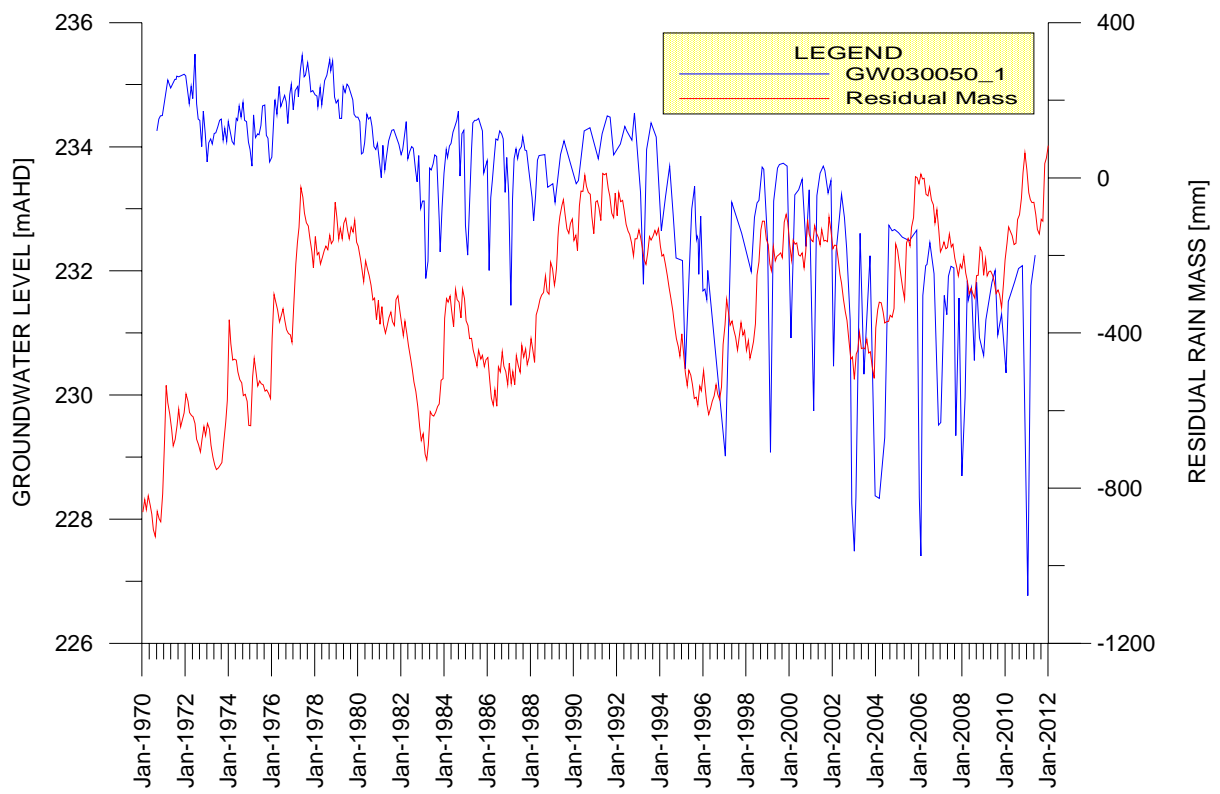
Sample		VKY3092 A	VKY3092B	VKY3092C
Date	Unit	16/08/2012	16/08/2012	16/08/2012
Electrical Conductivity	µS/cm	18600	18600	17300
pH (Field)		7.25	7.21	7.16
pH(Lab)		7.61	7.59	7.73
Calcium	mg/L	378	390	348
Magnesium	mg/L	537	558	493
Sodium	mg/L	3740	3840	2960
Potassium	mg/L	20	21	22
Chloride	mg/L	6250	6420	5680
Sulfate as SO4	mg/L	757	752	704
Hydroxide Alkalinity as CaCO3	mg/L	<1	<1	<1
Carbonate Alkalinity as CaCO3	mg/L	<1	<1	<1
Bicarbonate Alkalinity as CaCO3	mg/L	816	811	782
Total Alkalinity as CaCO3	mg/L	816	811	782
Arsenic	mg/L	<0.001	<0.001	<0.001
Cadmium	mg/L	<0.0001	<0.0001	<0.0001
Chromium	mg/L	<0.001	<0.001	<0.001
Copper	mg/L	0.032	0.032	0.037
Lead	mg/L	0.001	0.001	0.002
Nickel	mg/L	<0.001	<0.001	<0.001
Zinc	mg/L	0.155	0.189	0.319
Iron	mg/L	<0.05	<0.05	<0.05
Mercury	mg/L	<0.0001	<0.0001	<0.0001
Total Anions	meq/L	208	213	190
Total Cations	meq/L	226	233	187
Ionic Balance	%	4.1	4.47	0.87

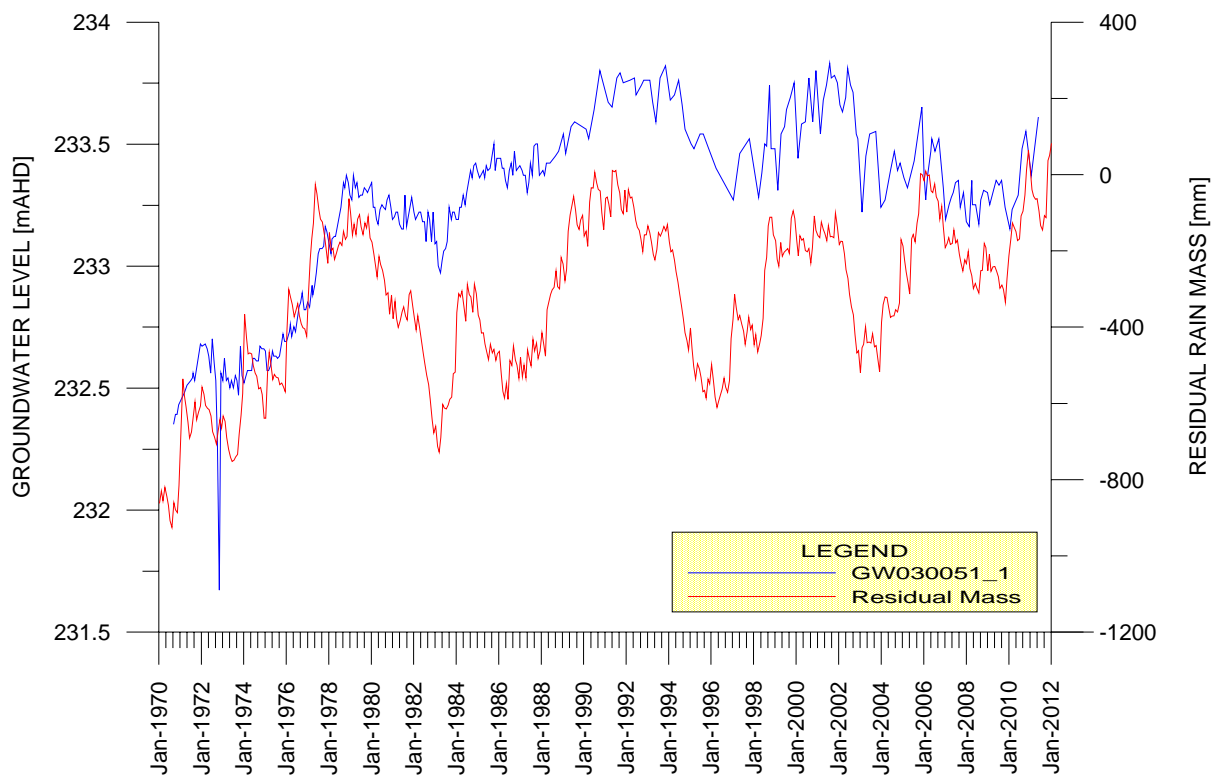
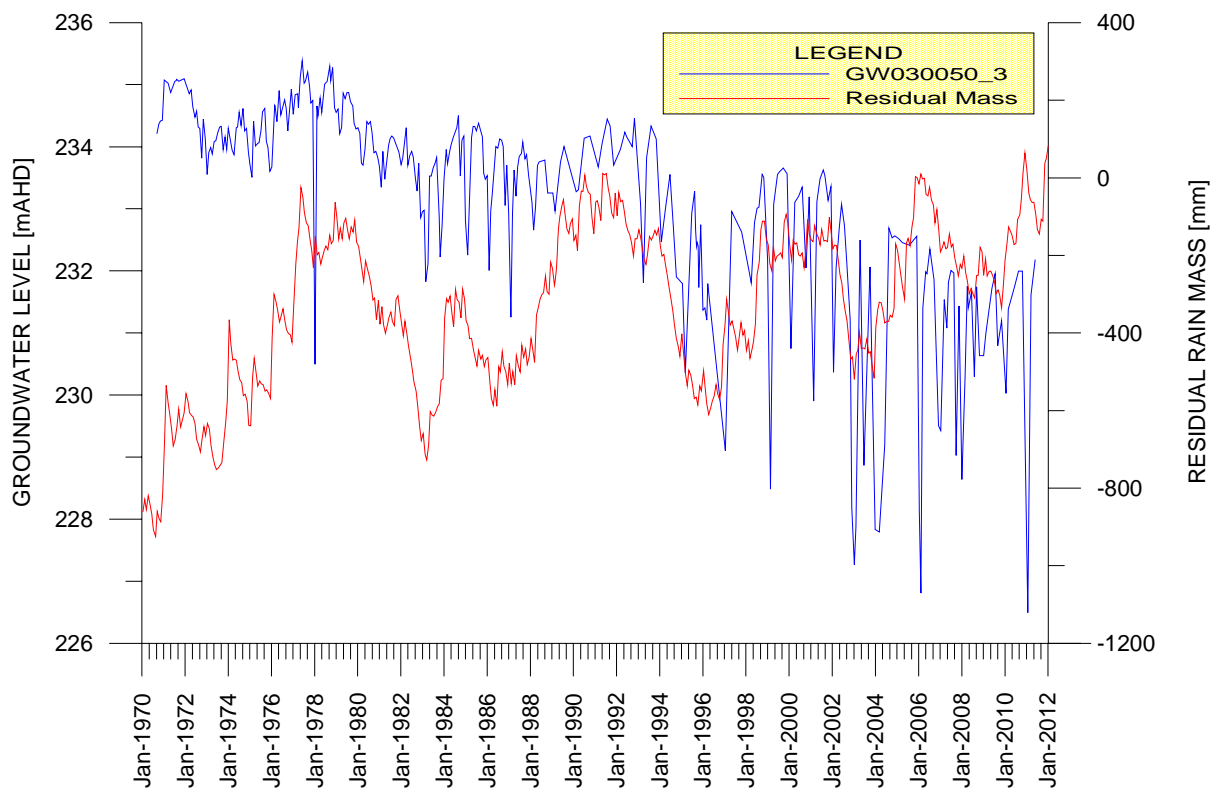
ATTACHMENT AB

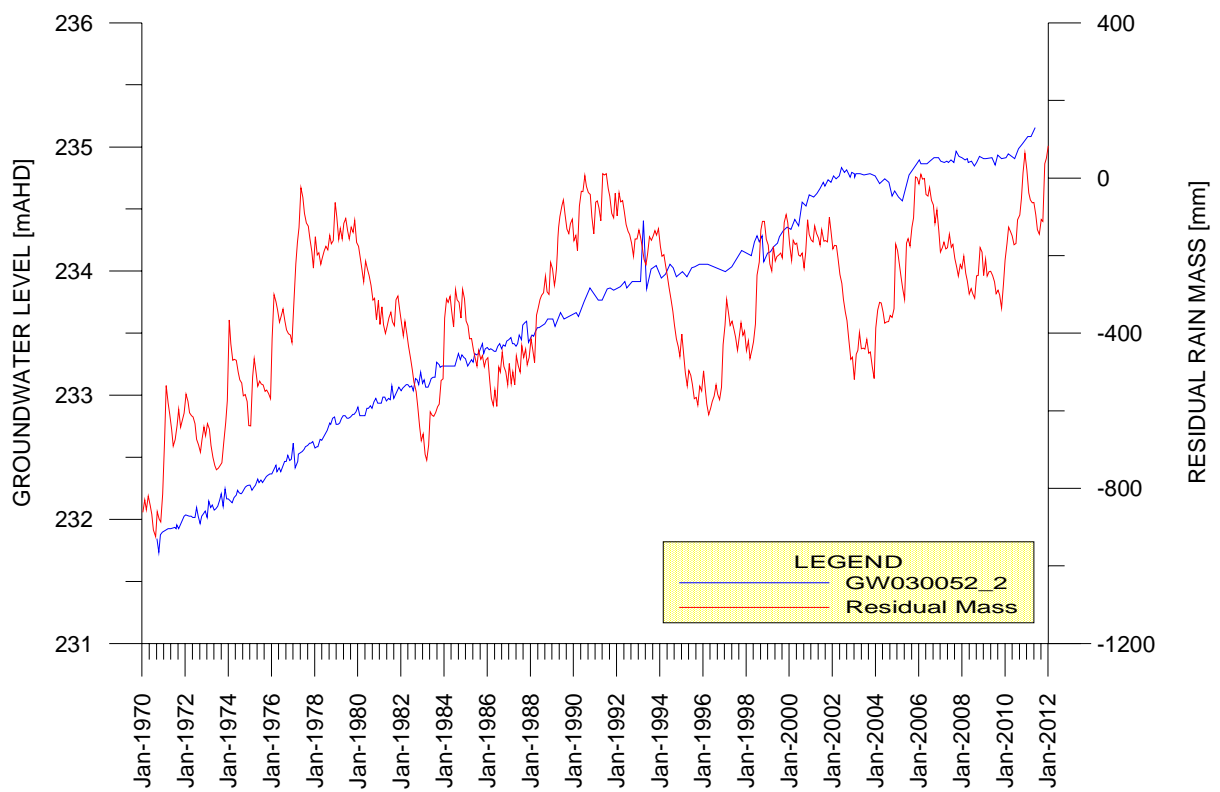
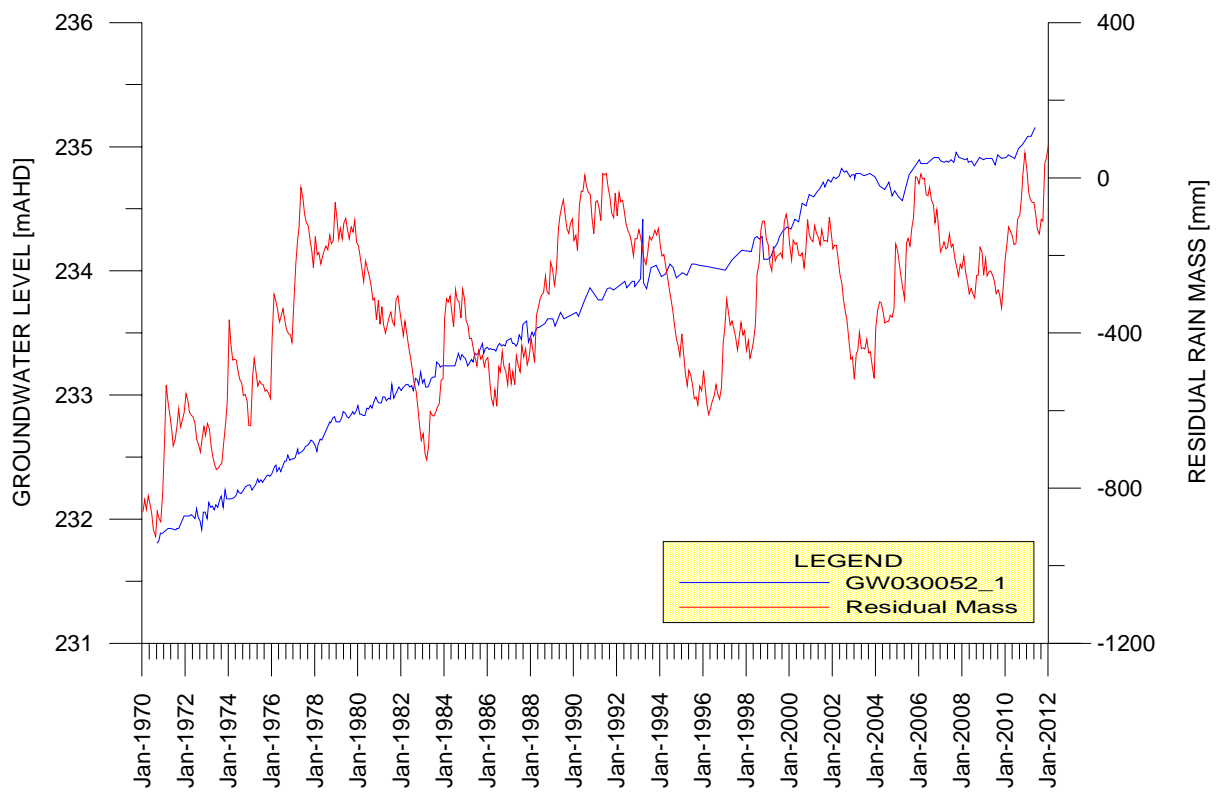
Groundwater Hydrographs for NOW Bores located in Zone 4 and Residual Rain Mass

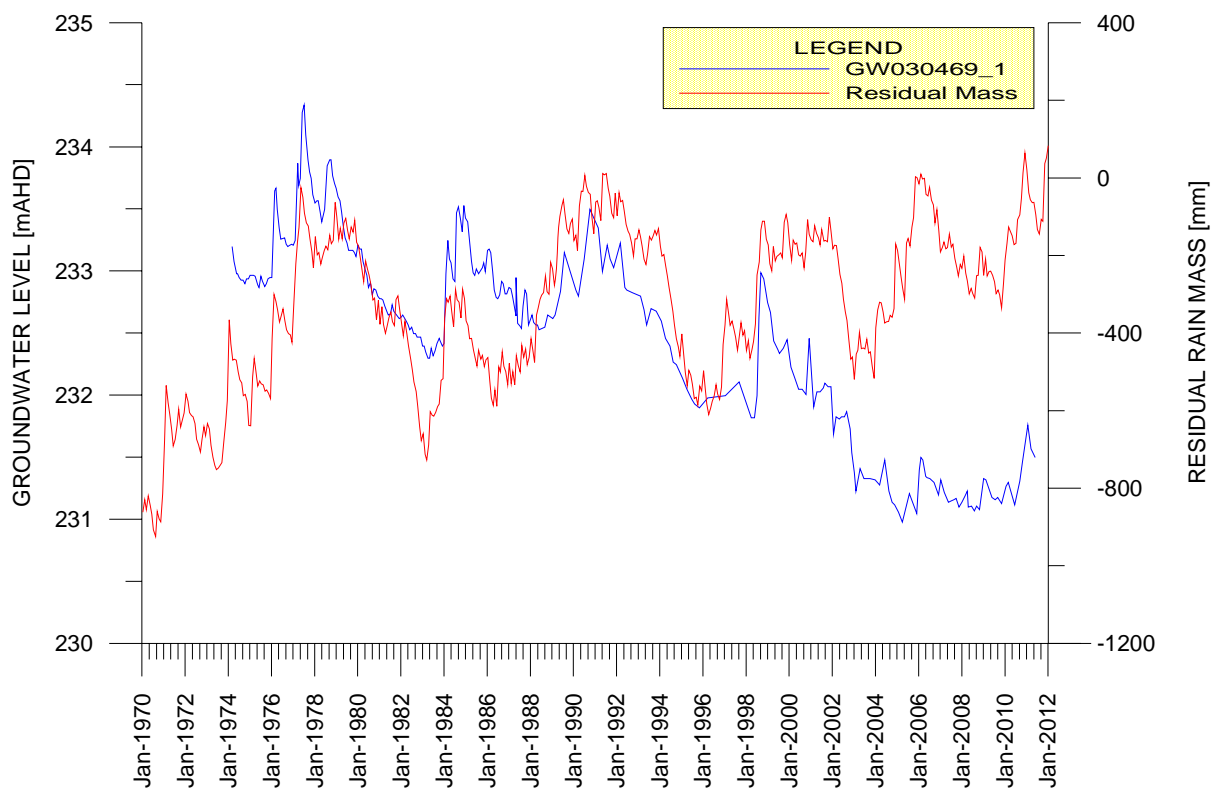
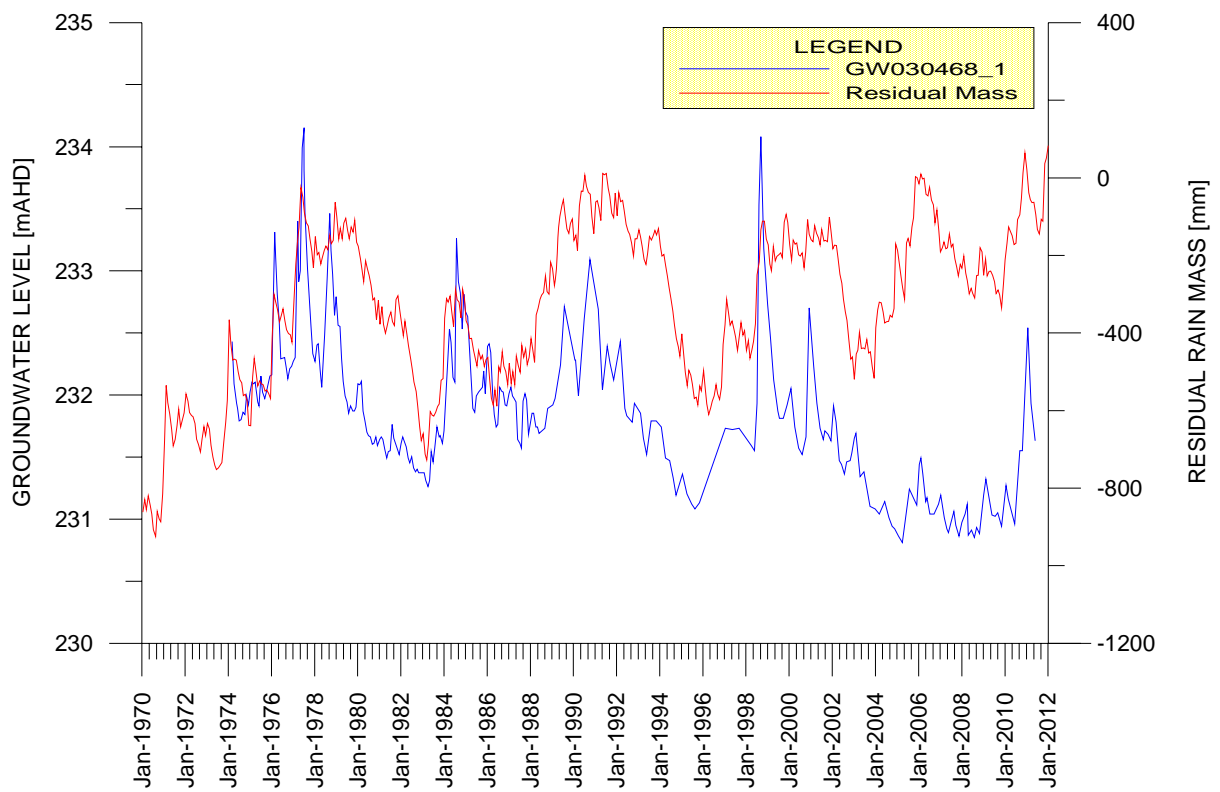


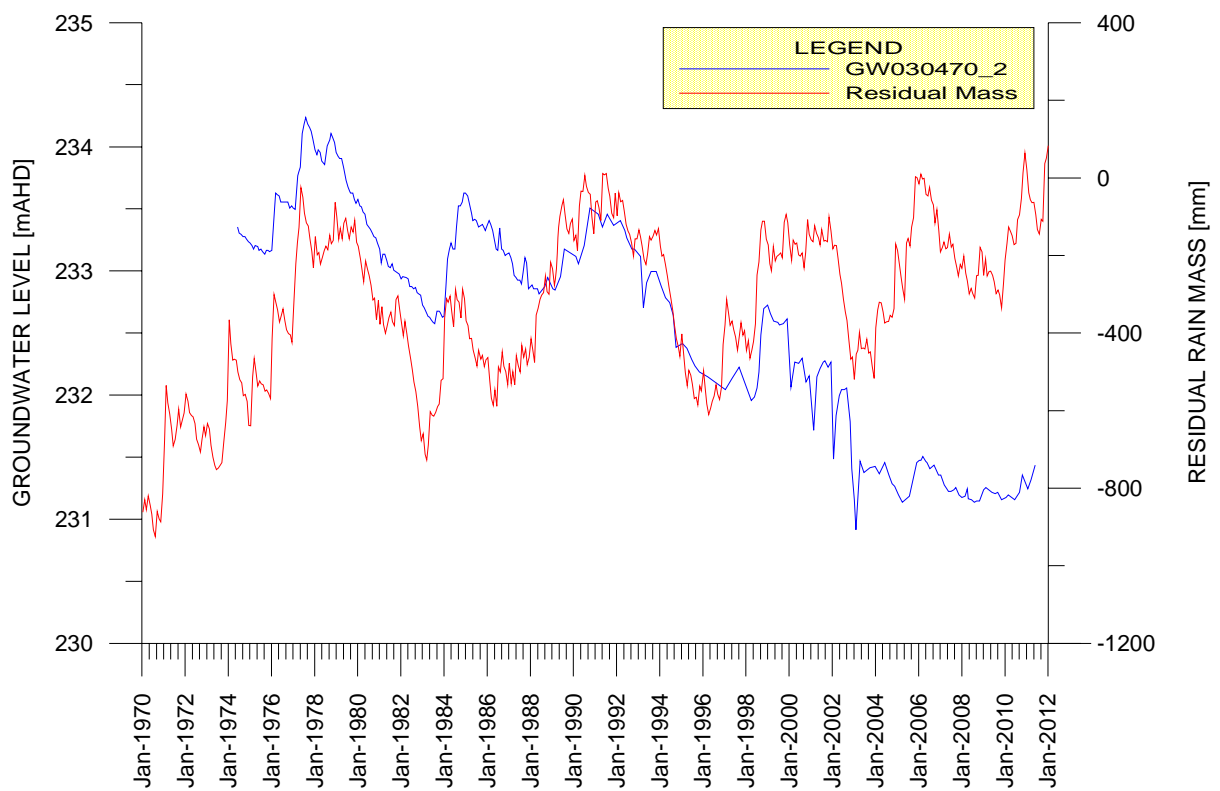
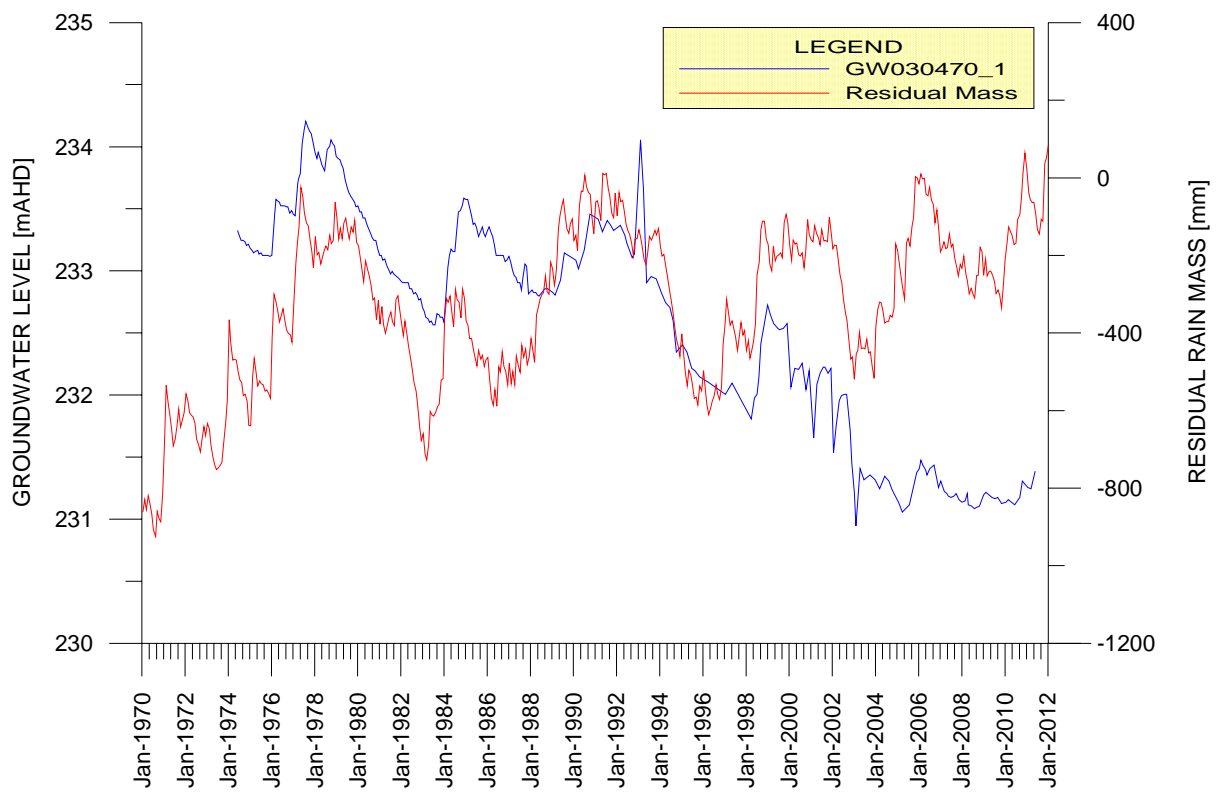


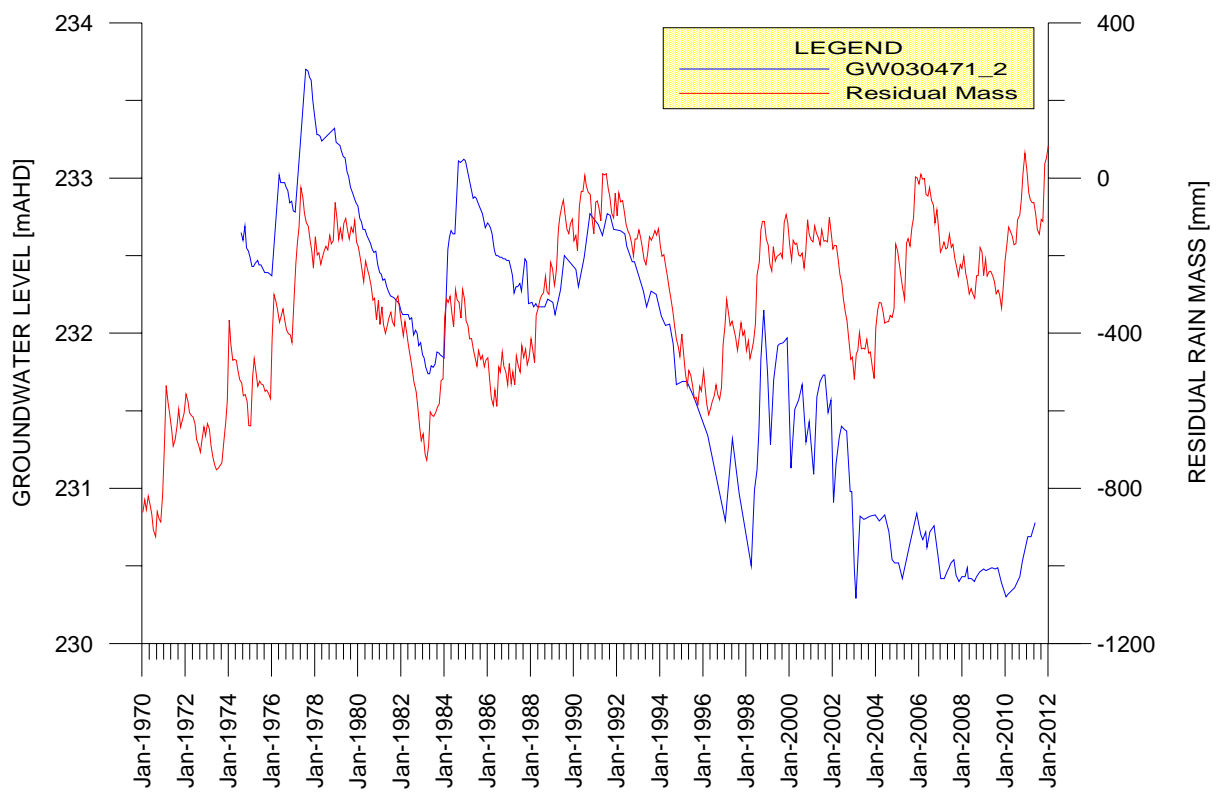
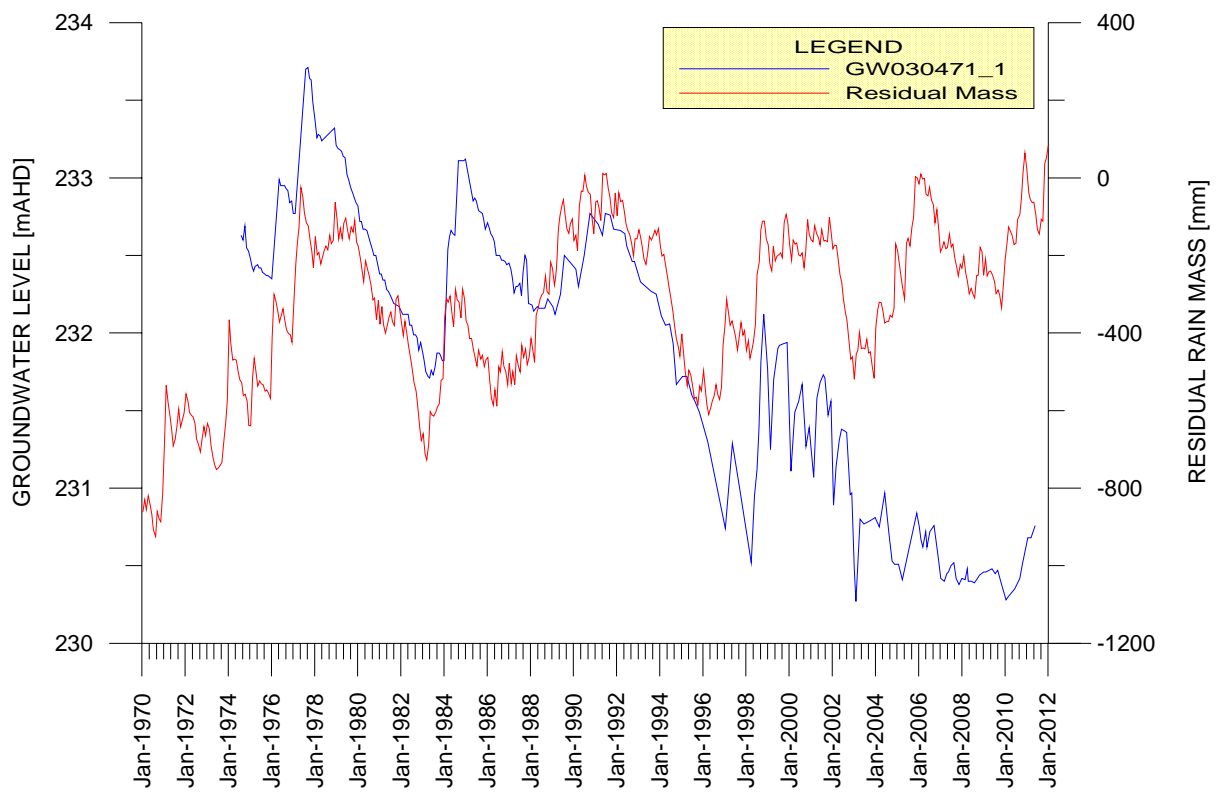


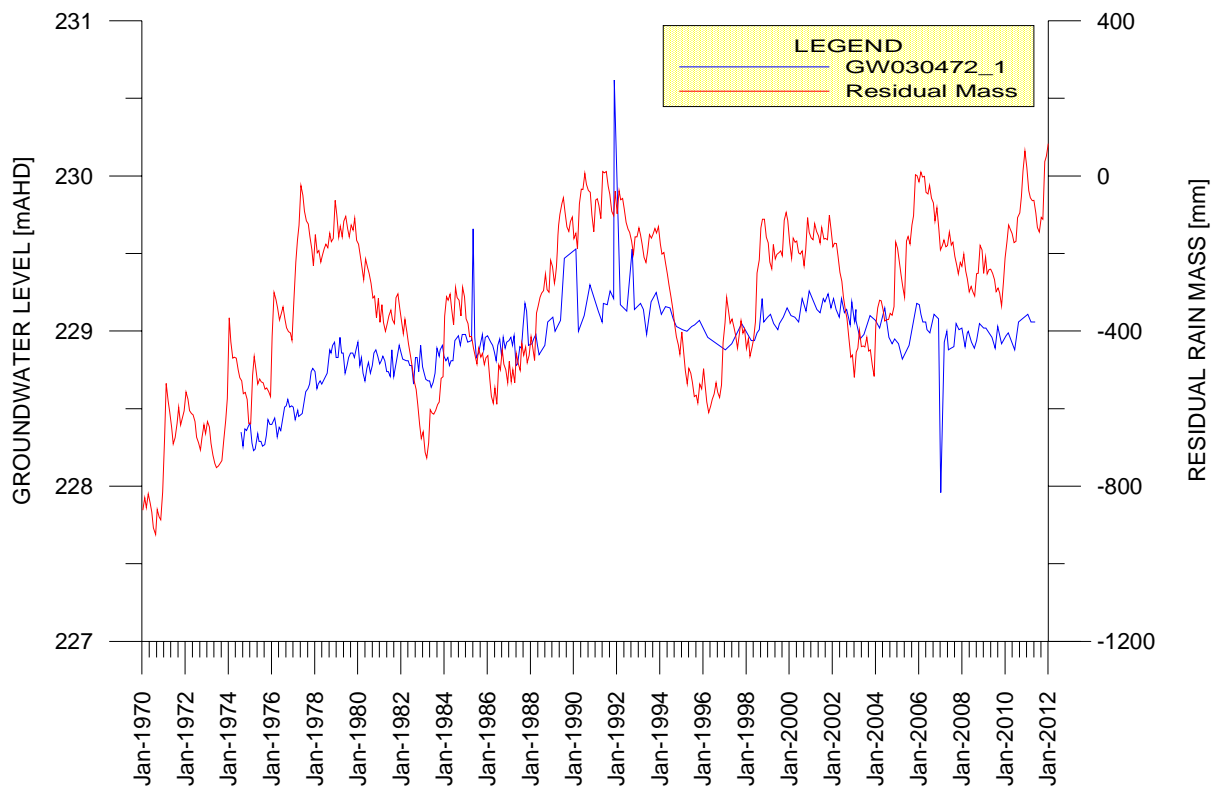
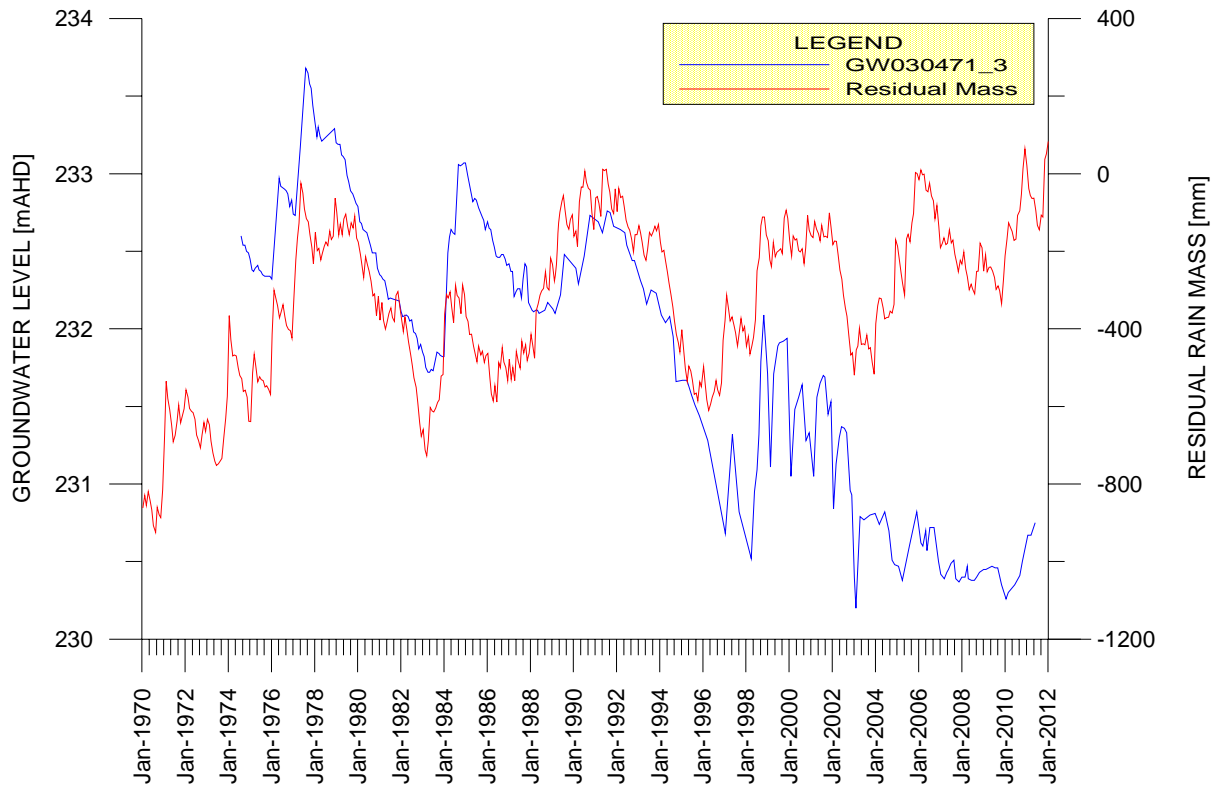


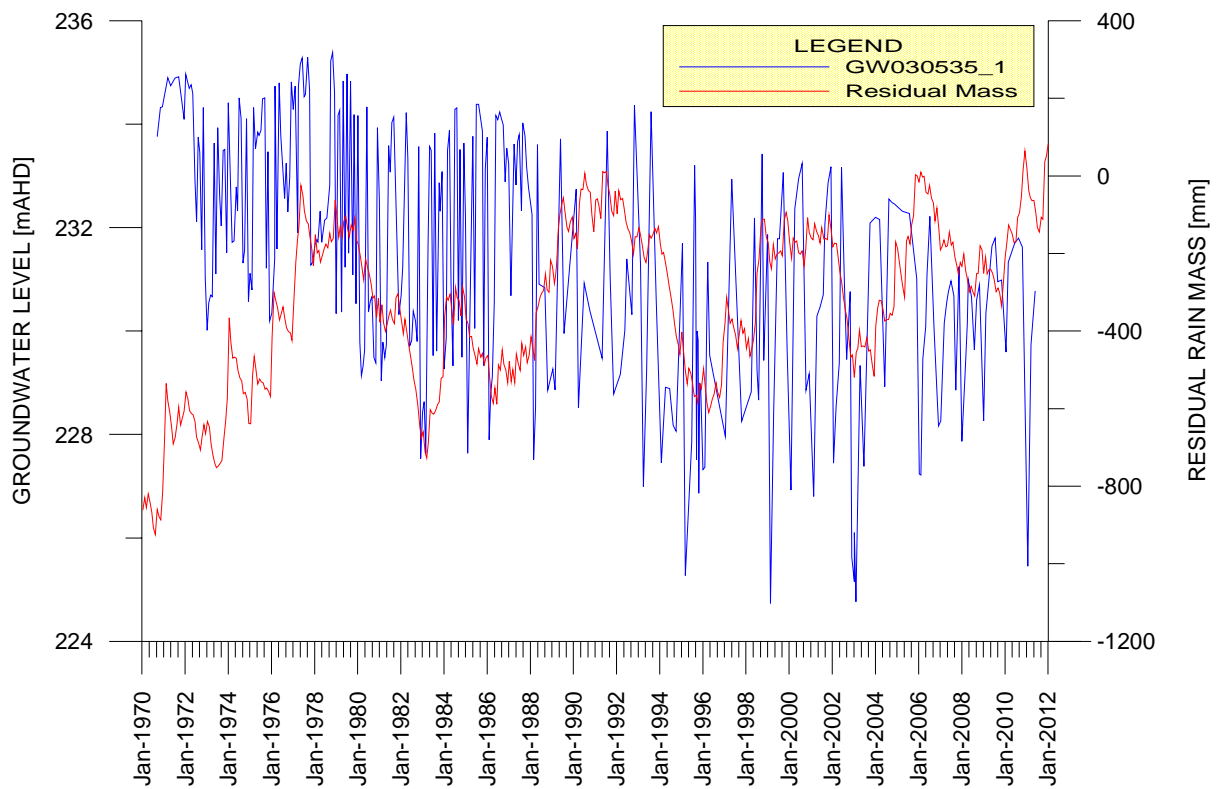
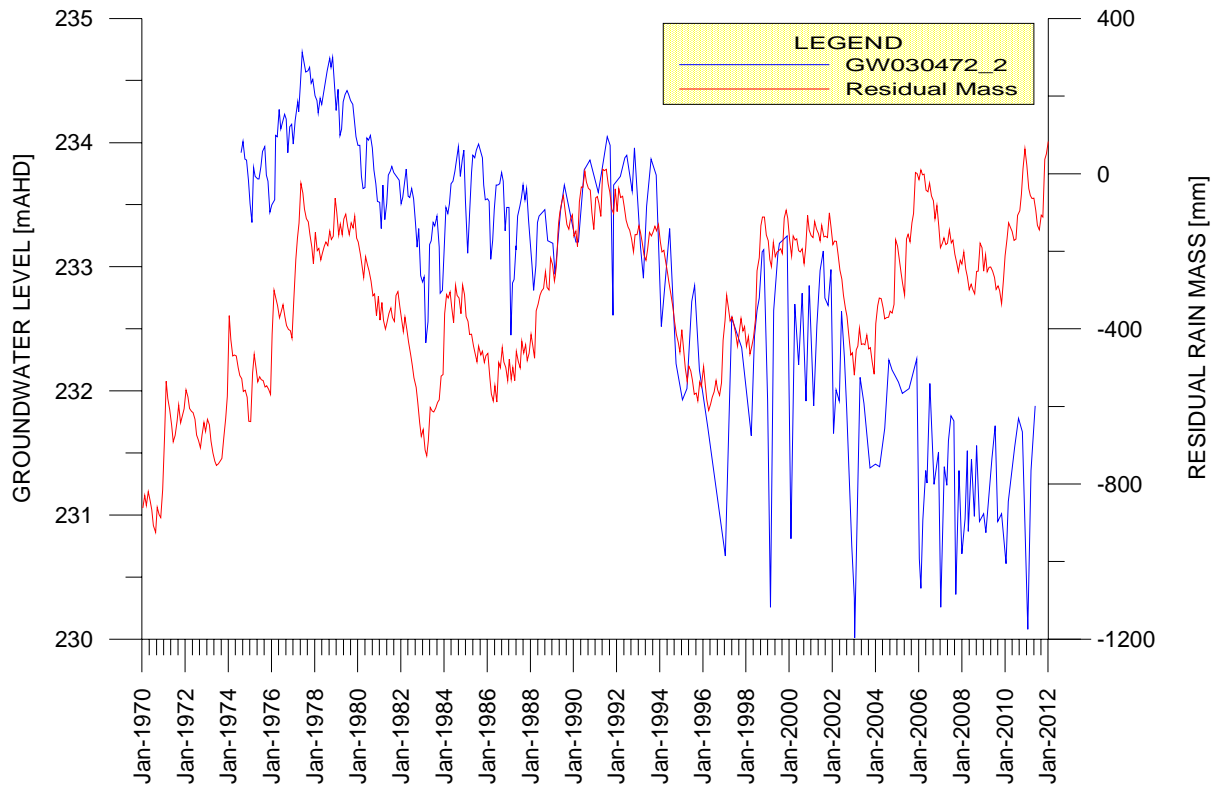


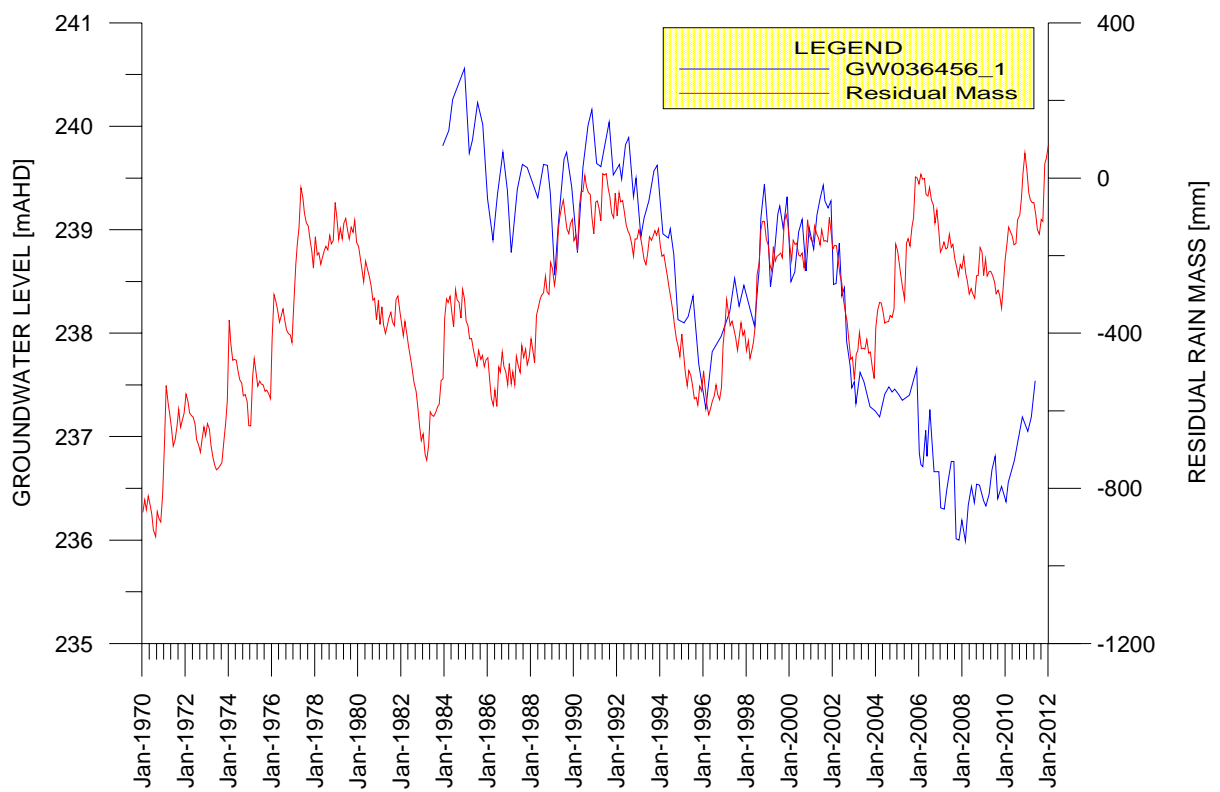
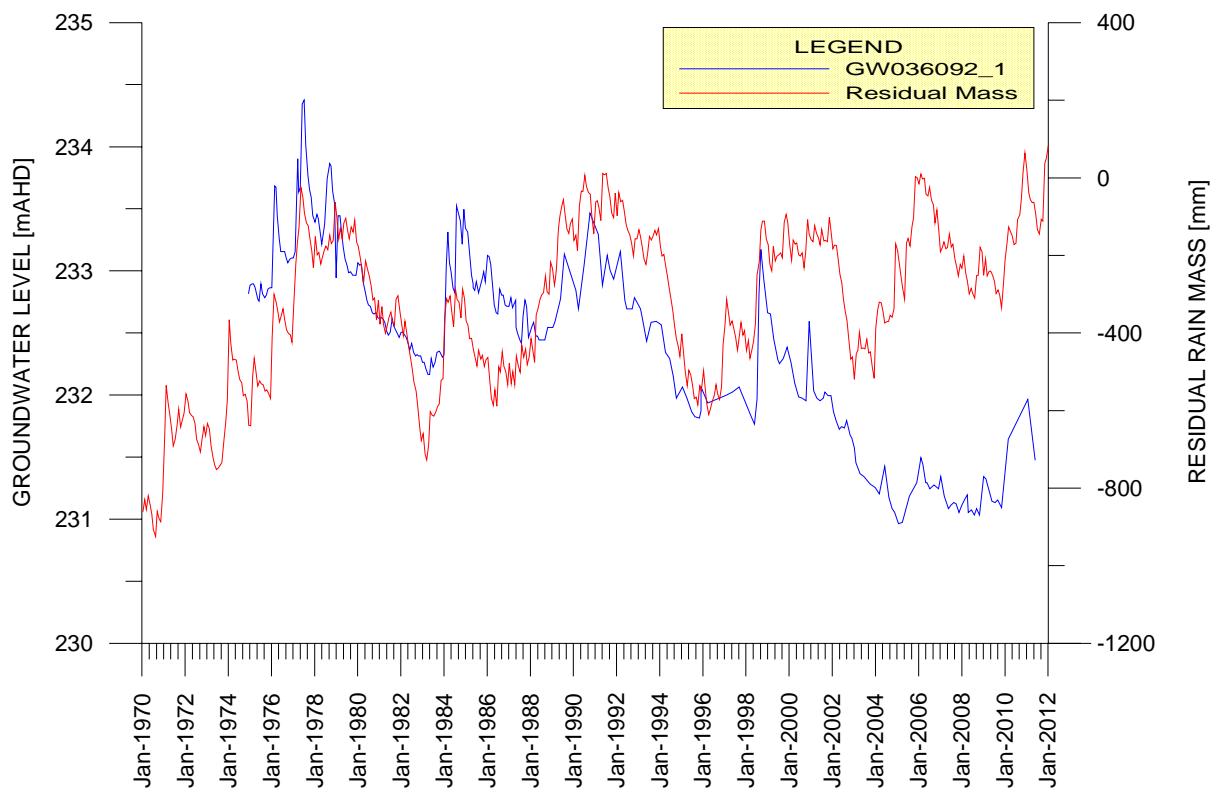


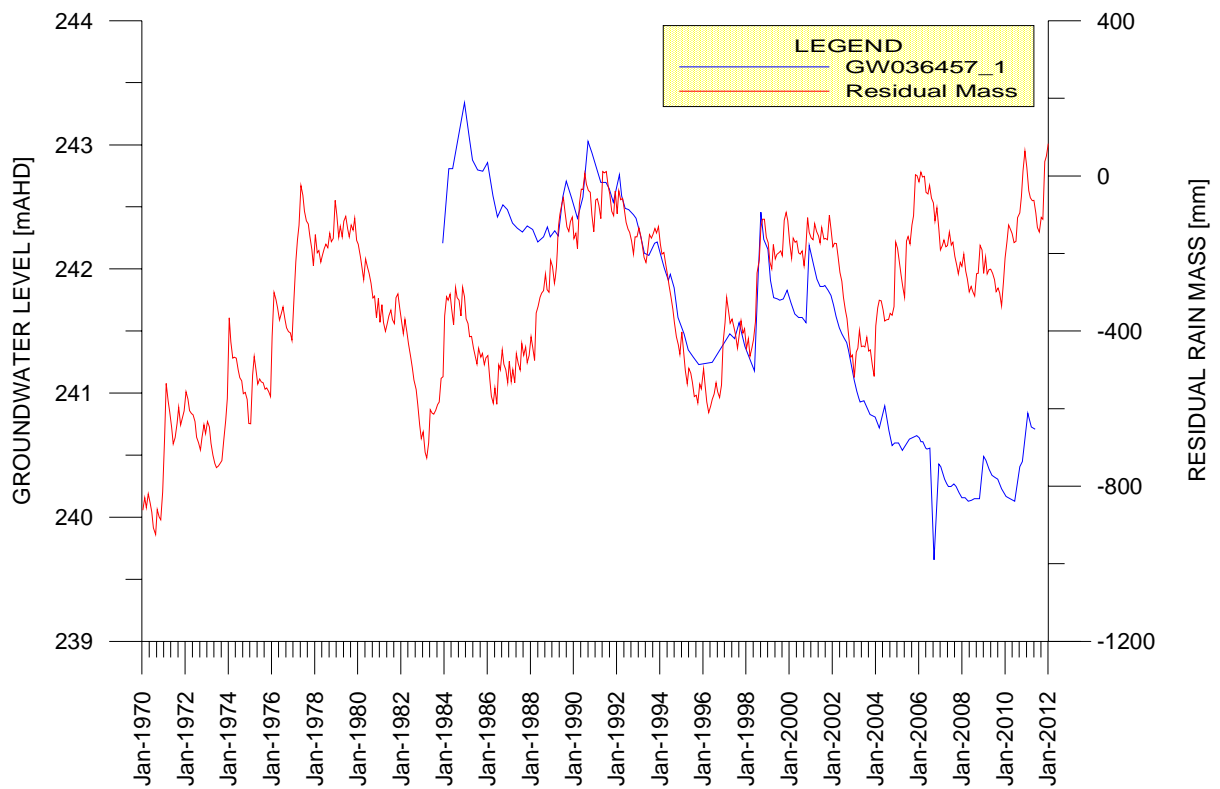
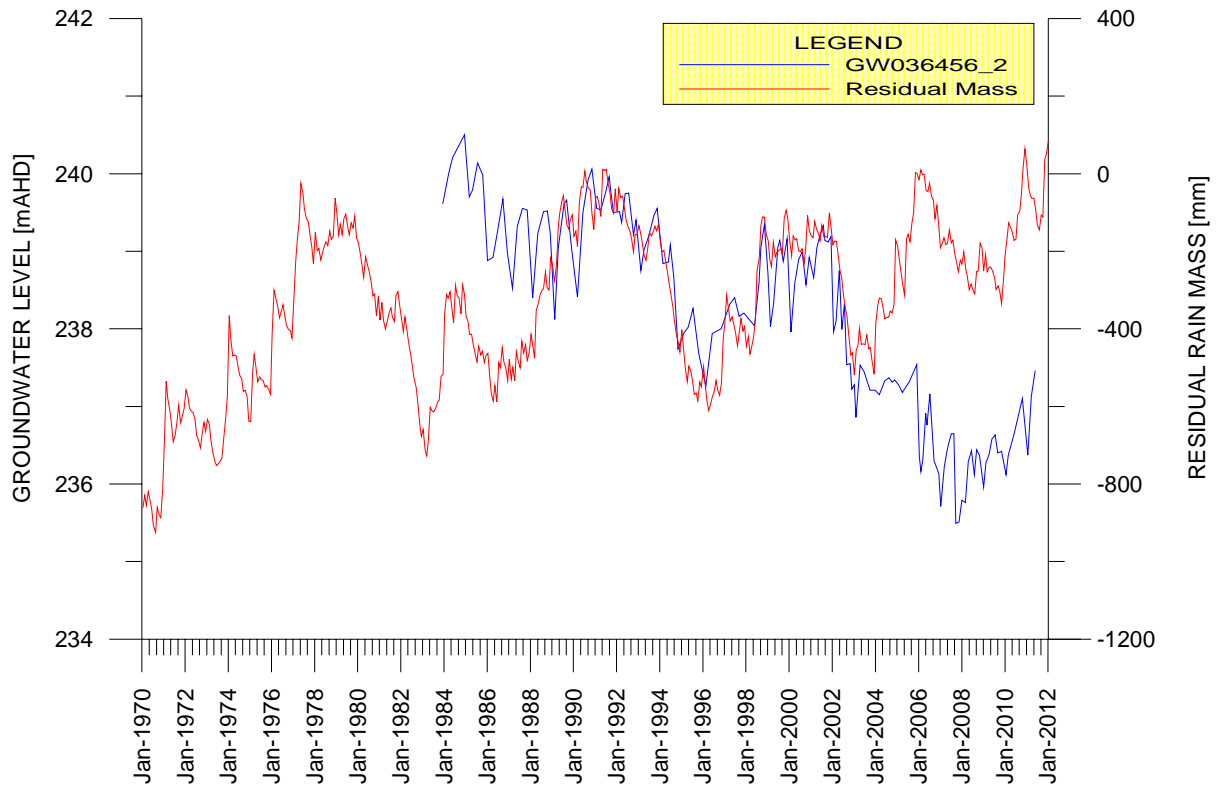


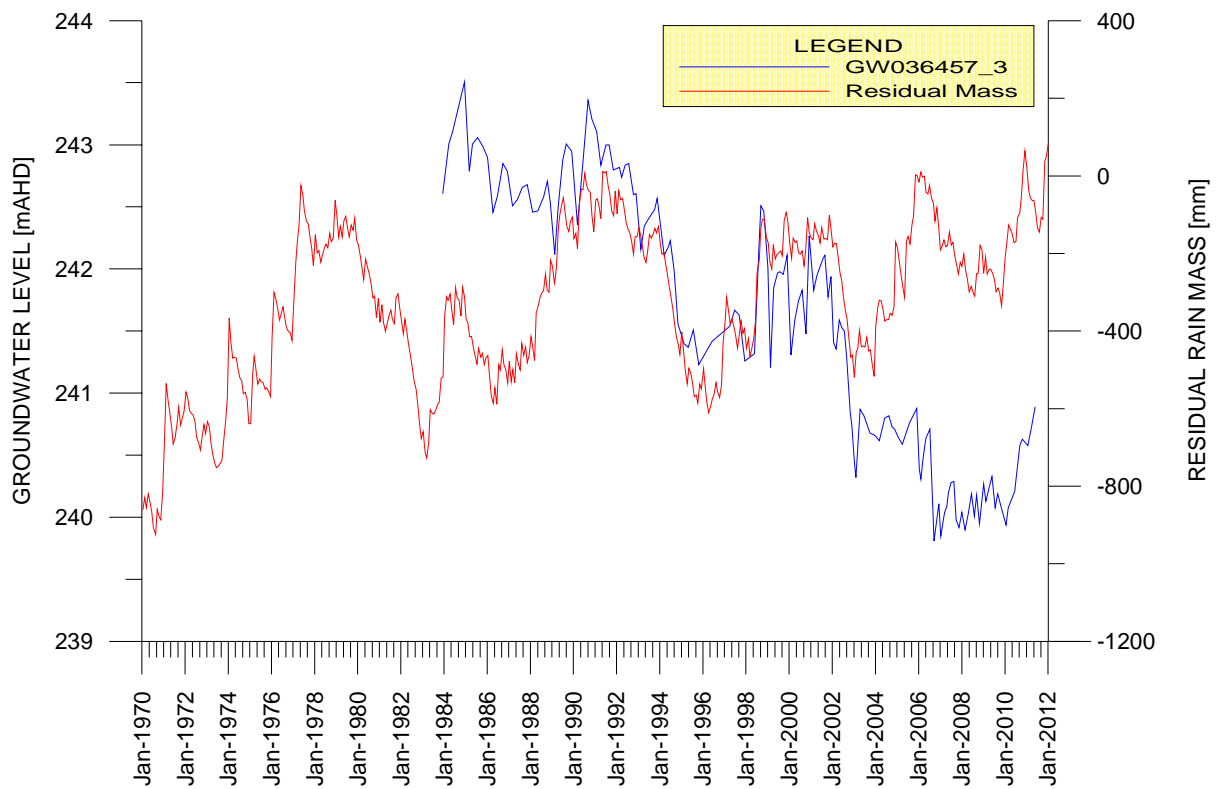
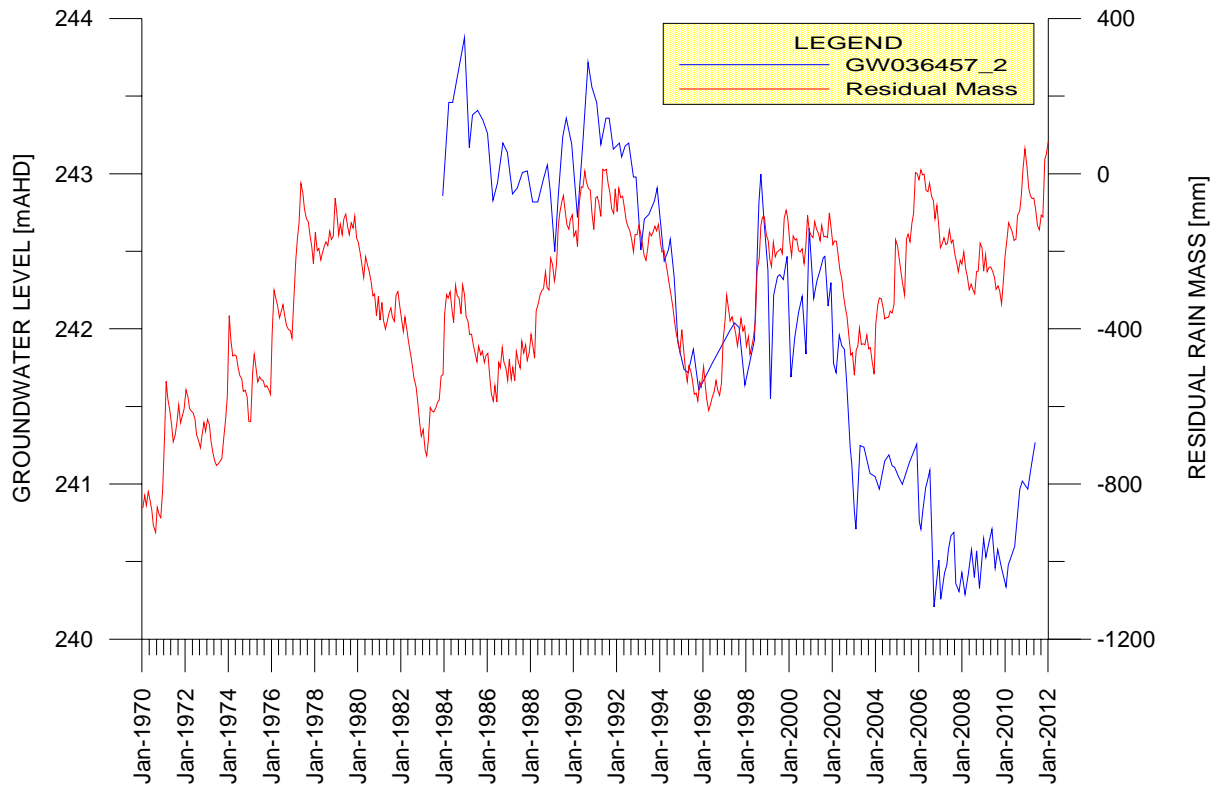


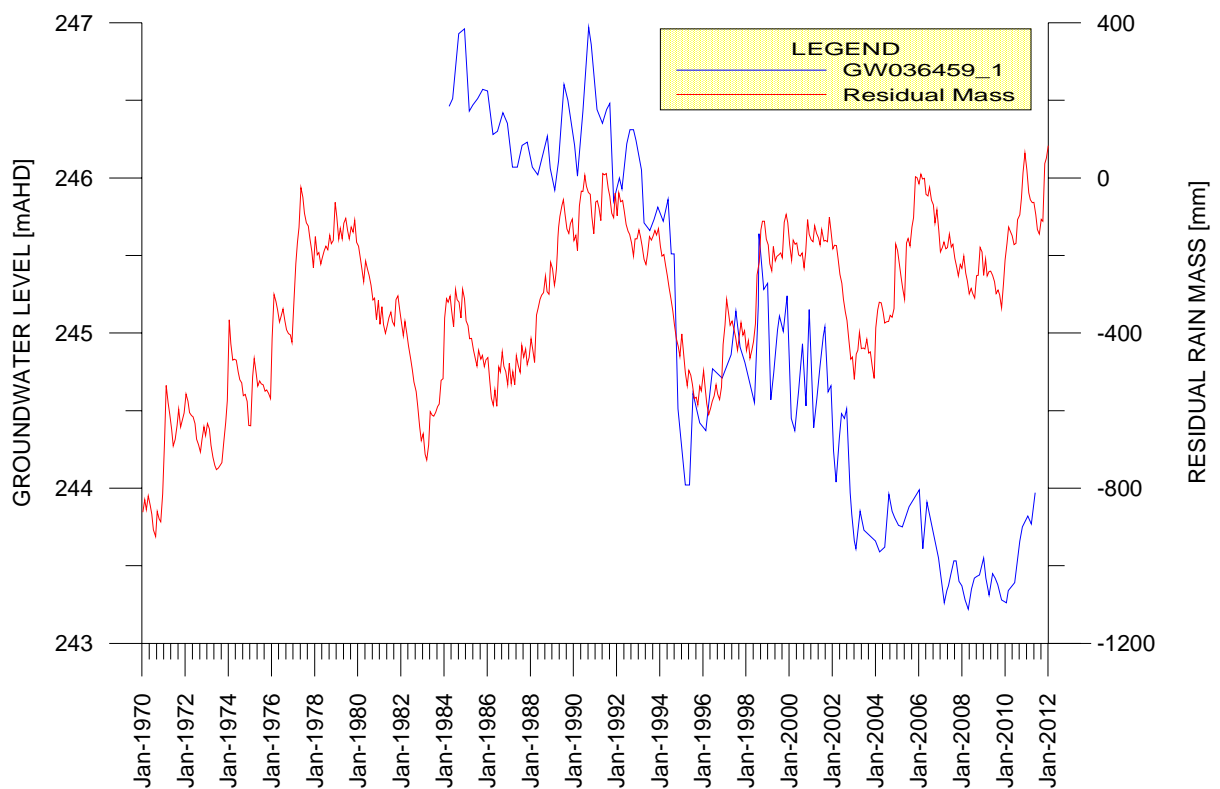
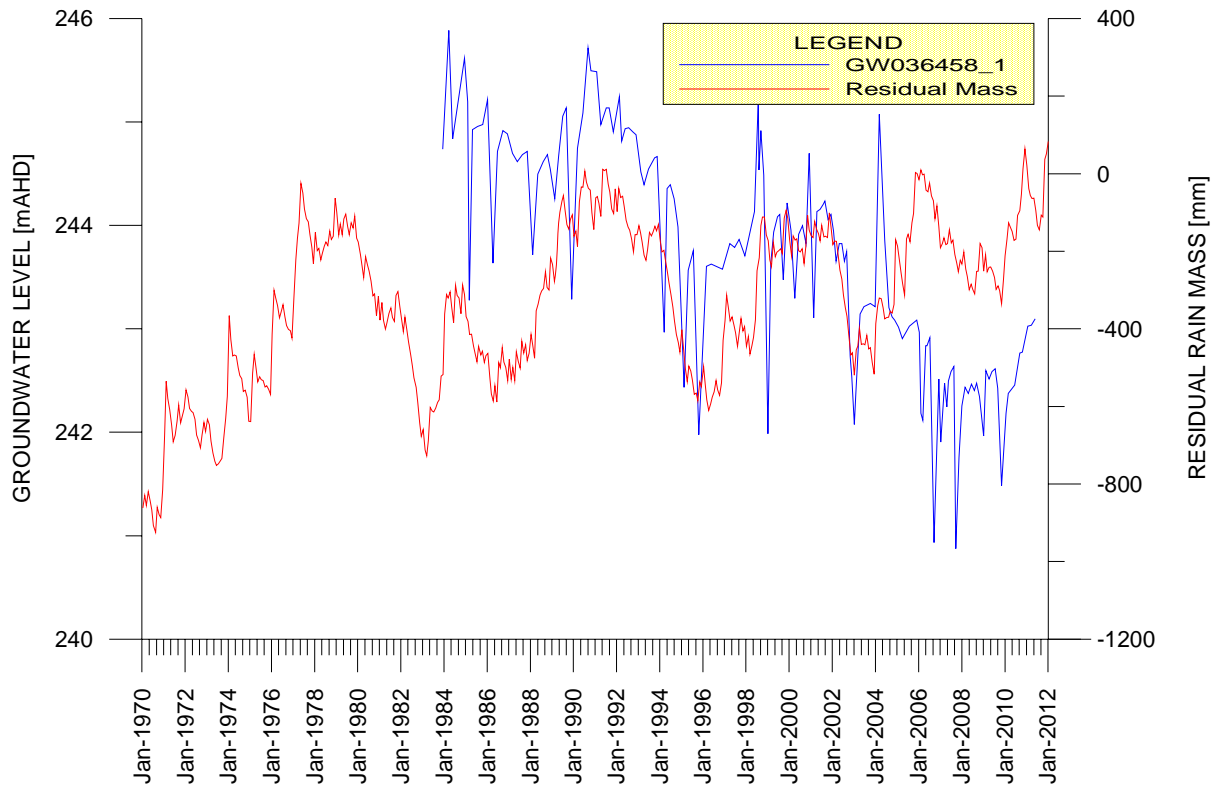


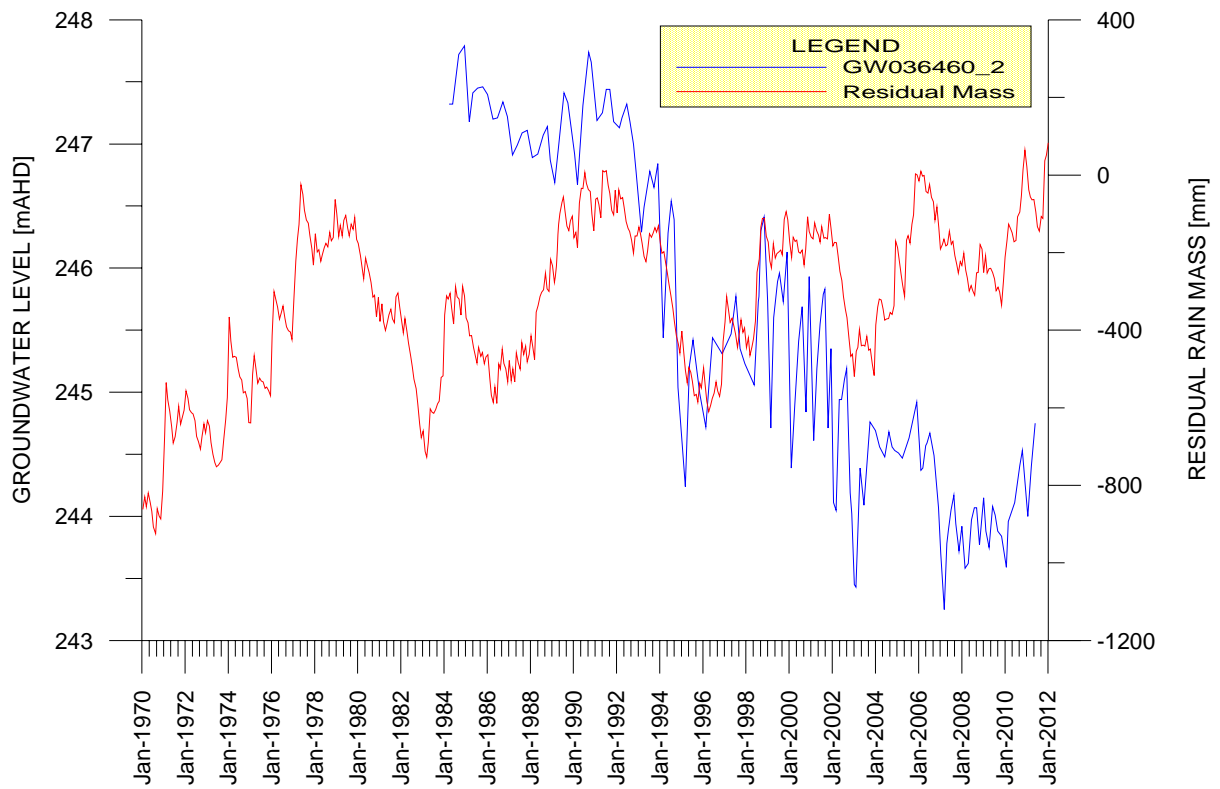
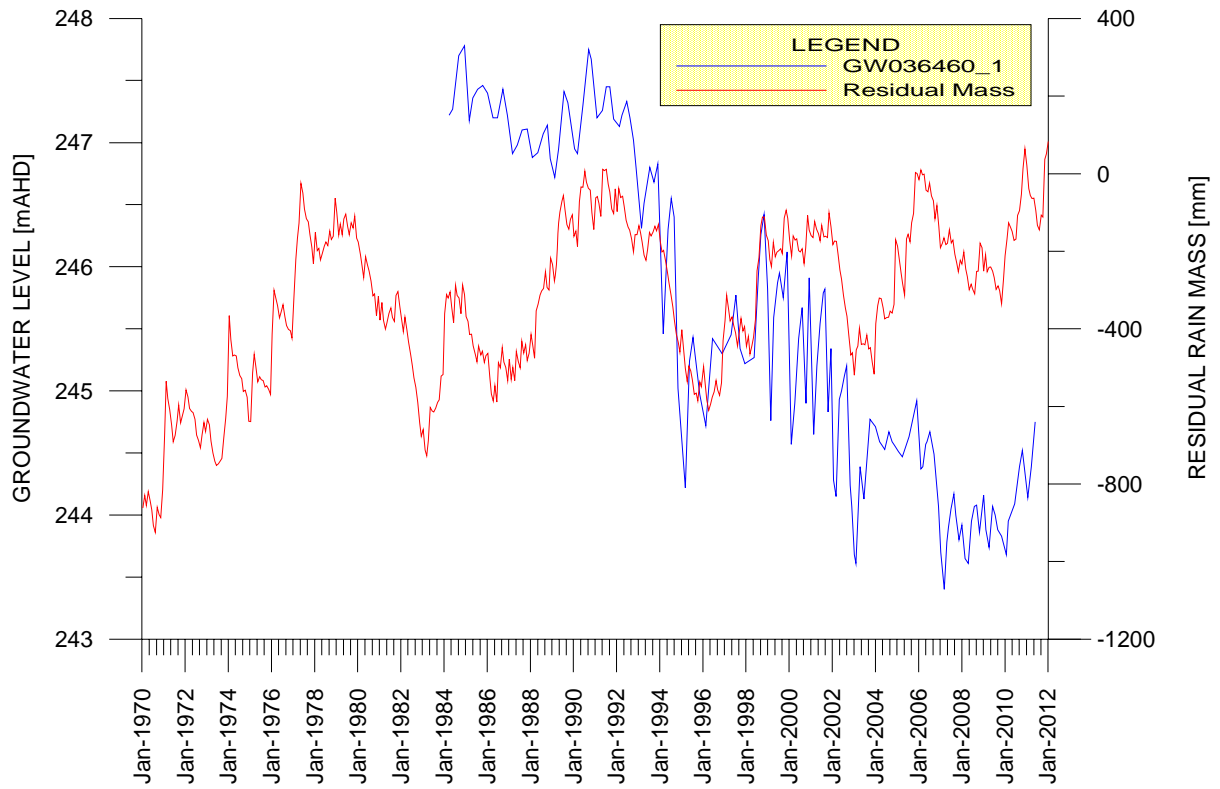


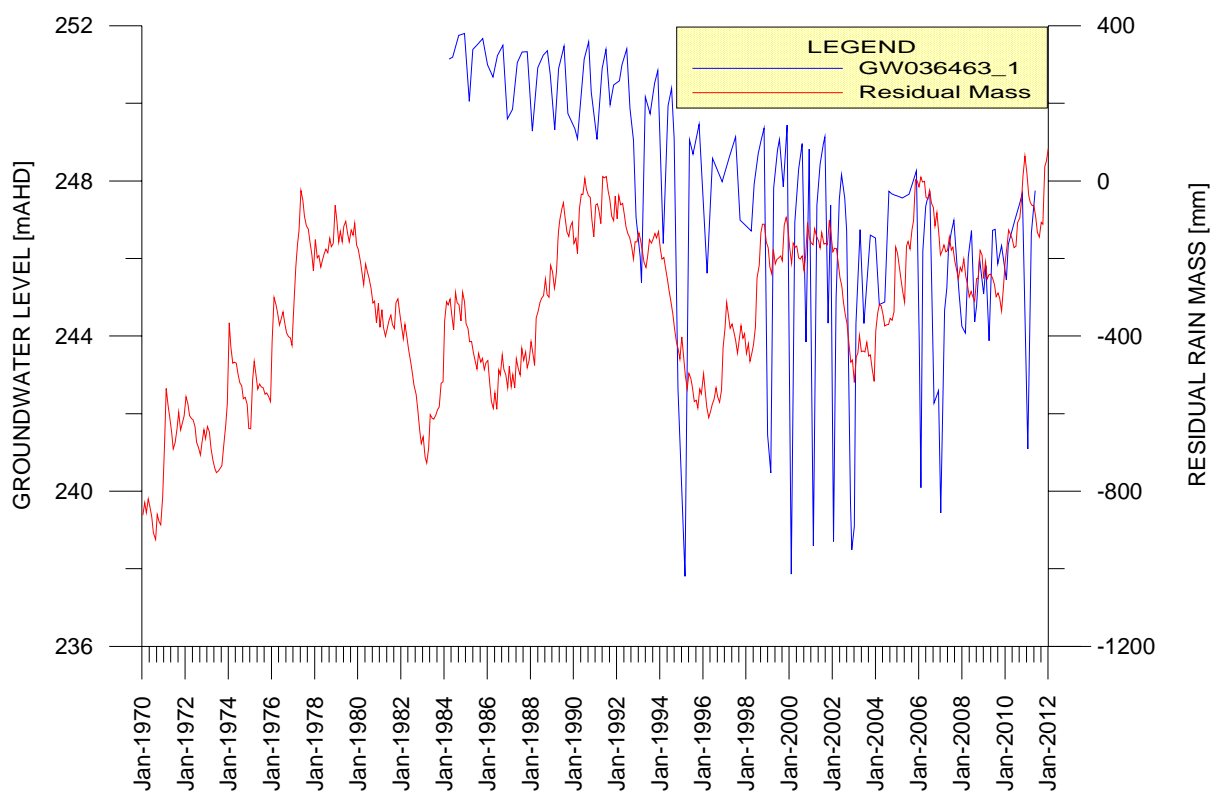
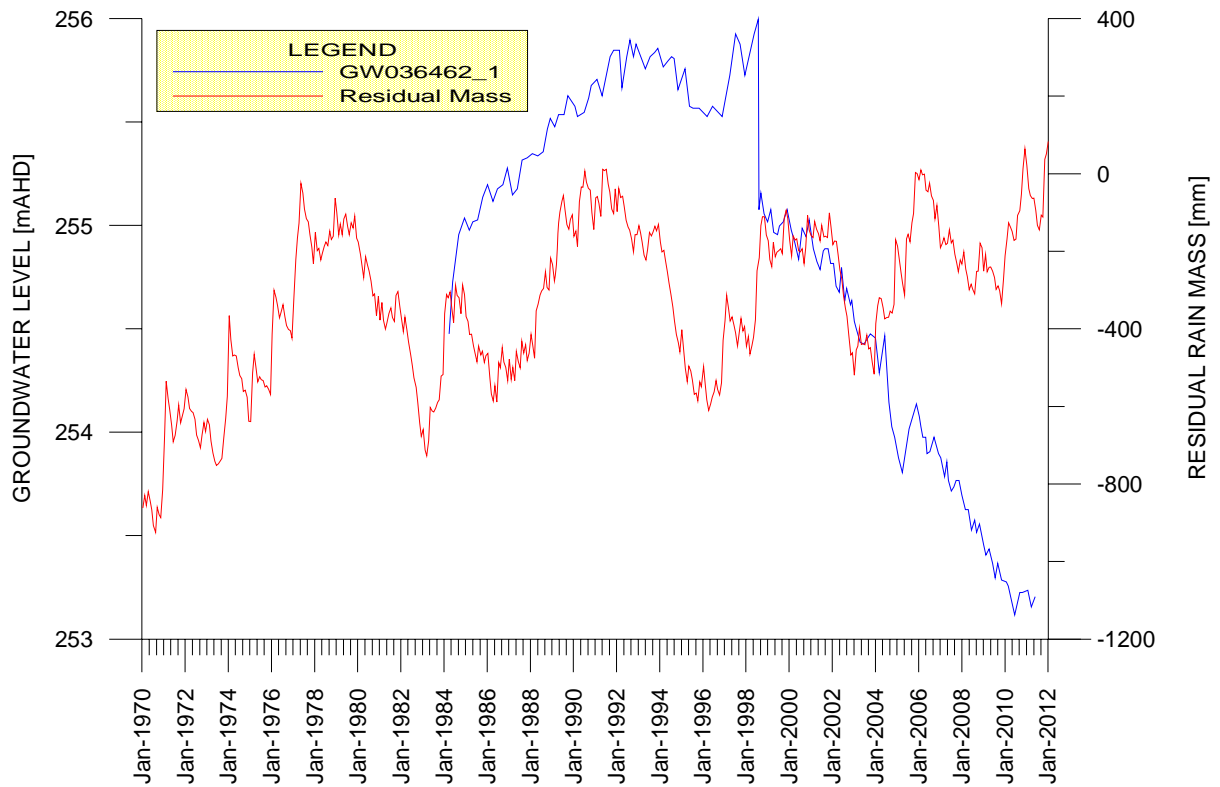


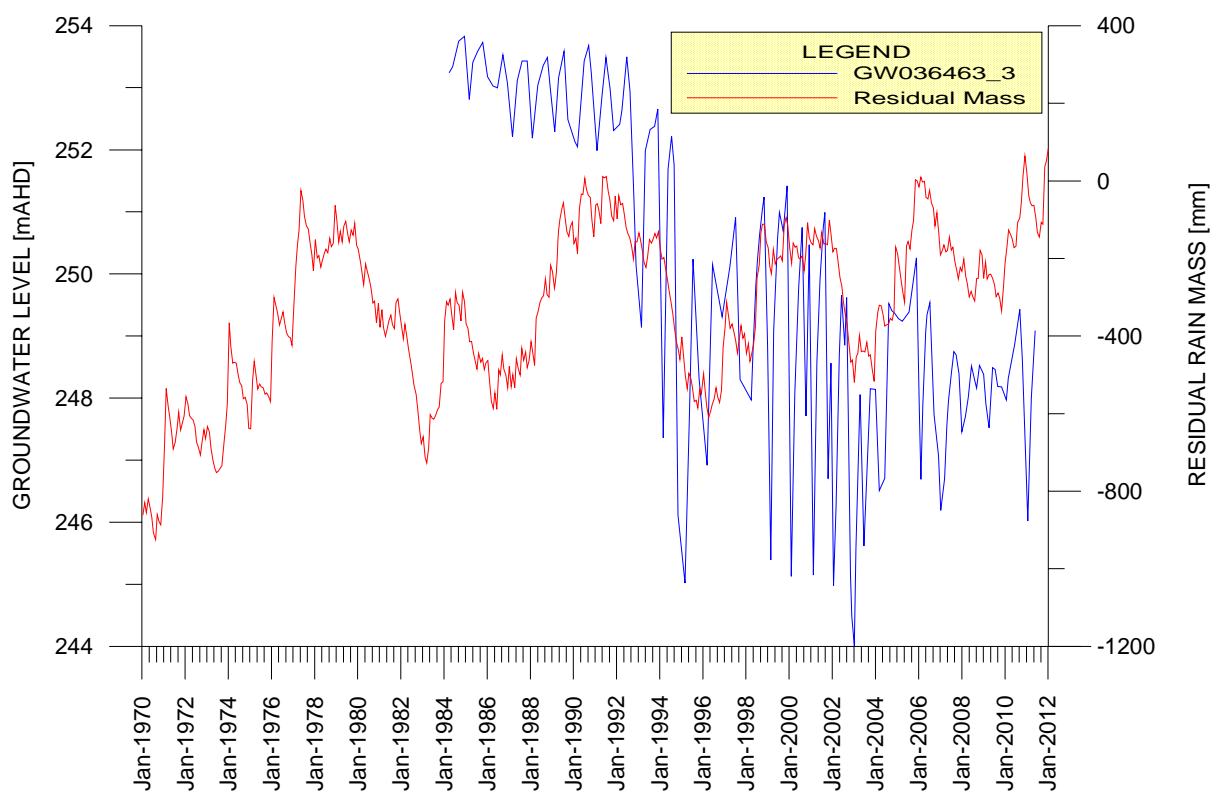
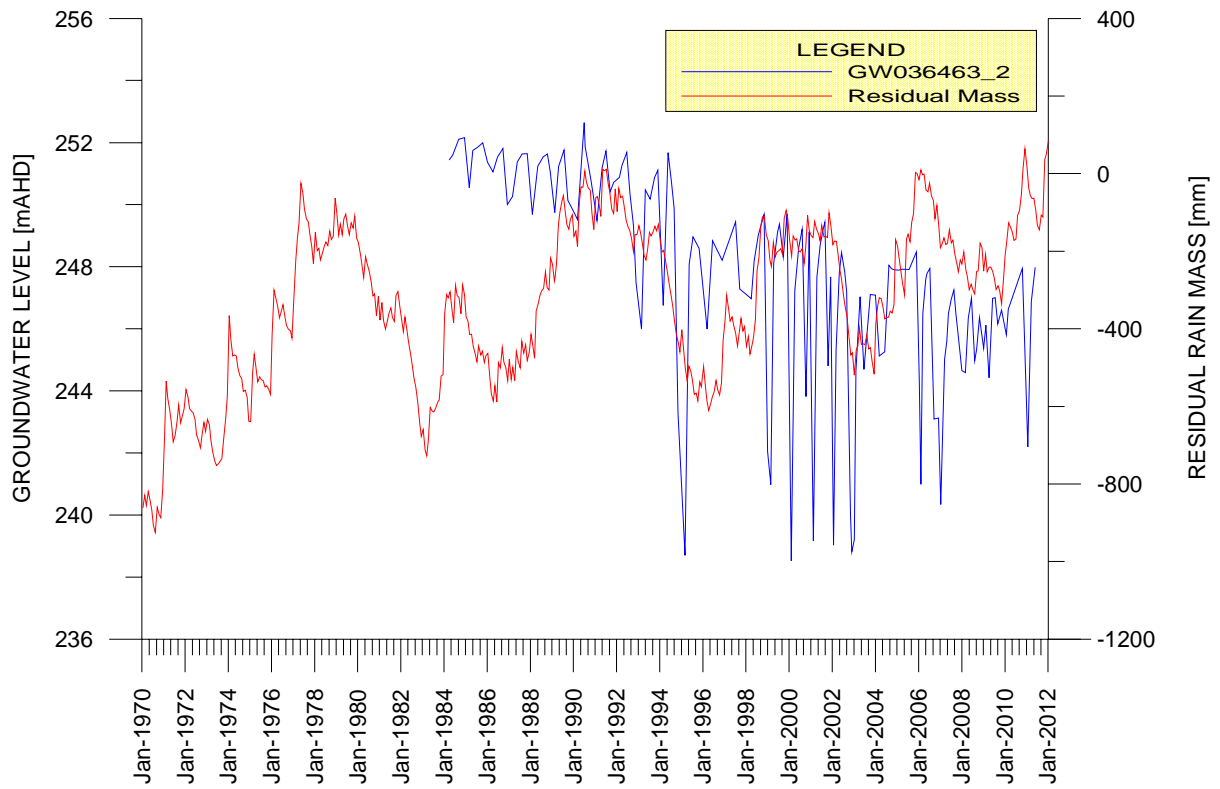


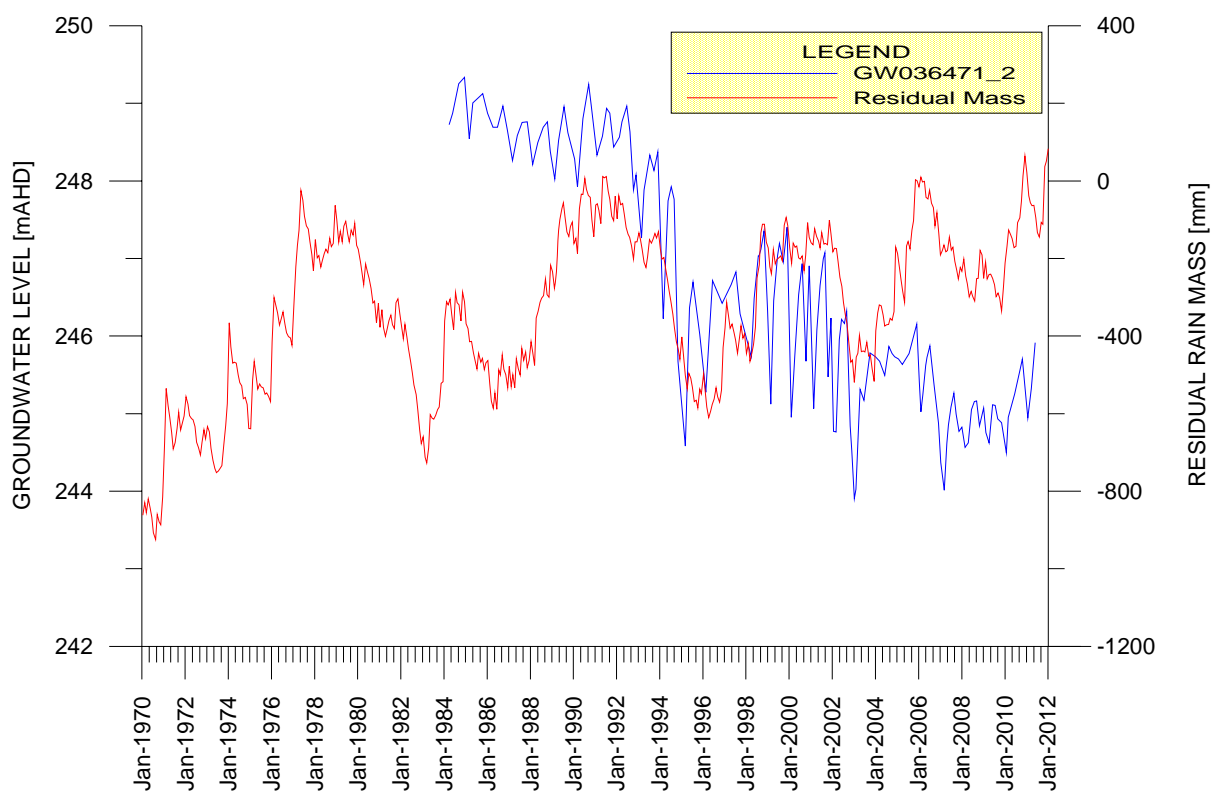
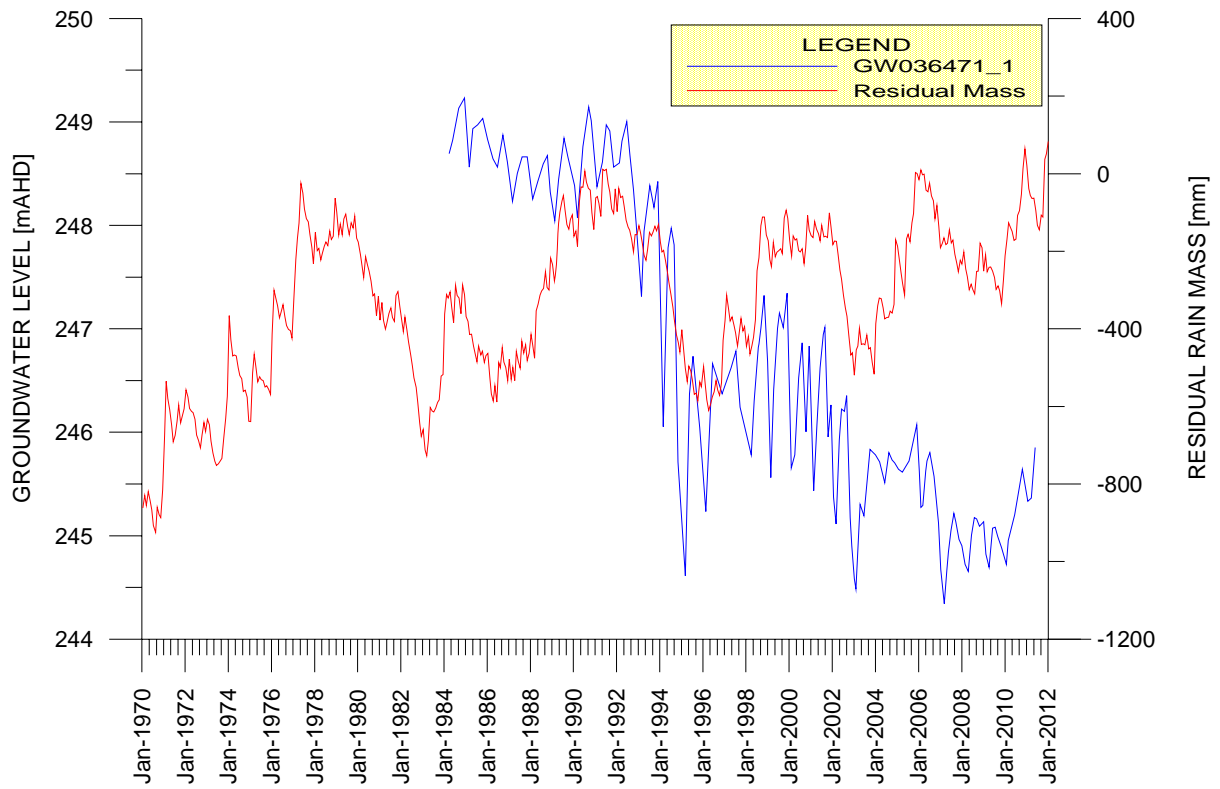


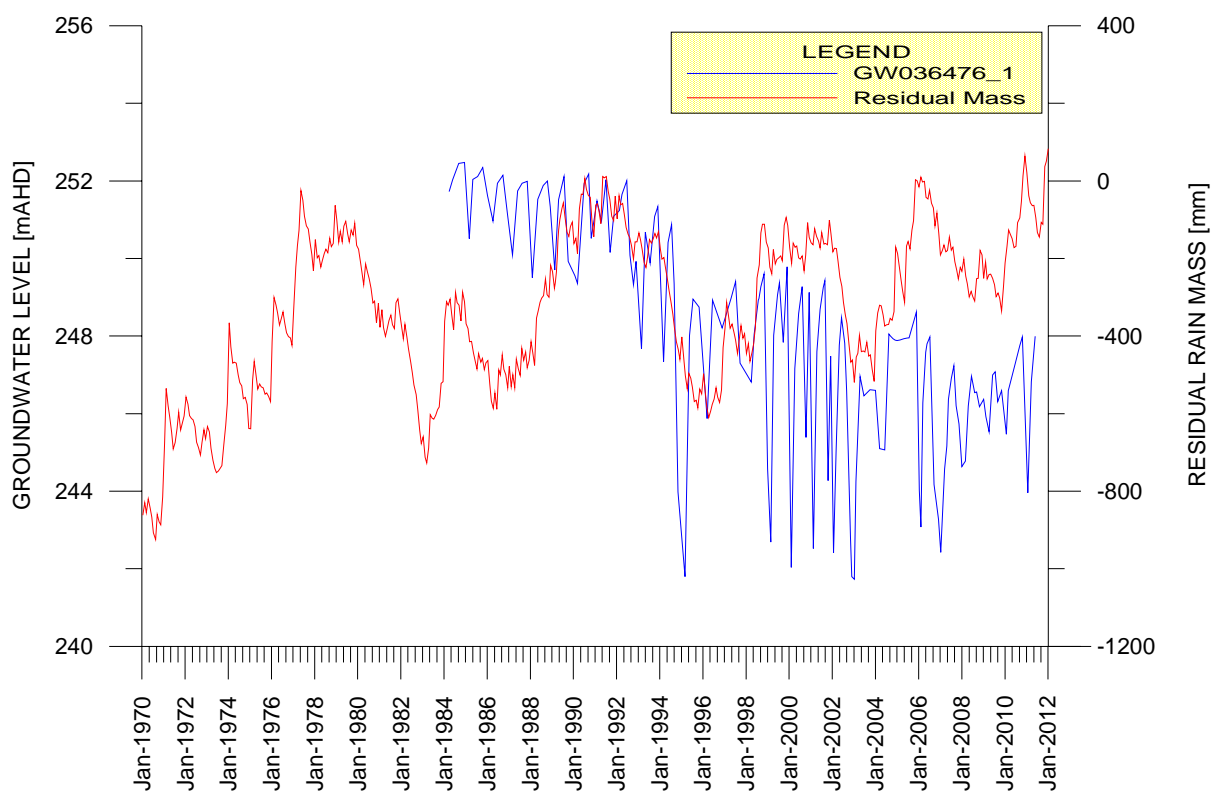
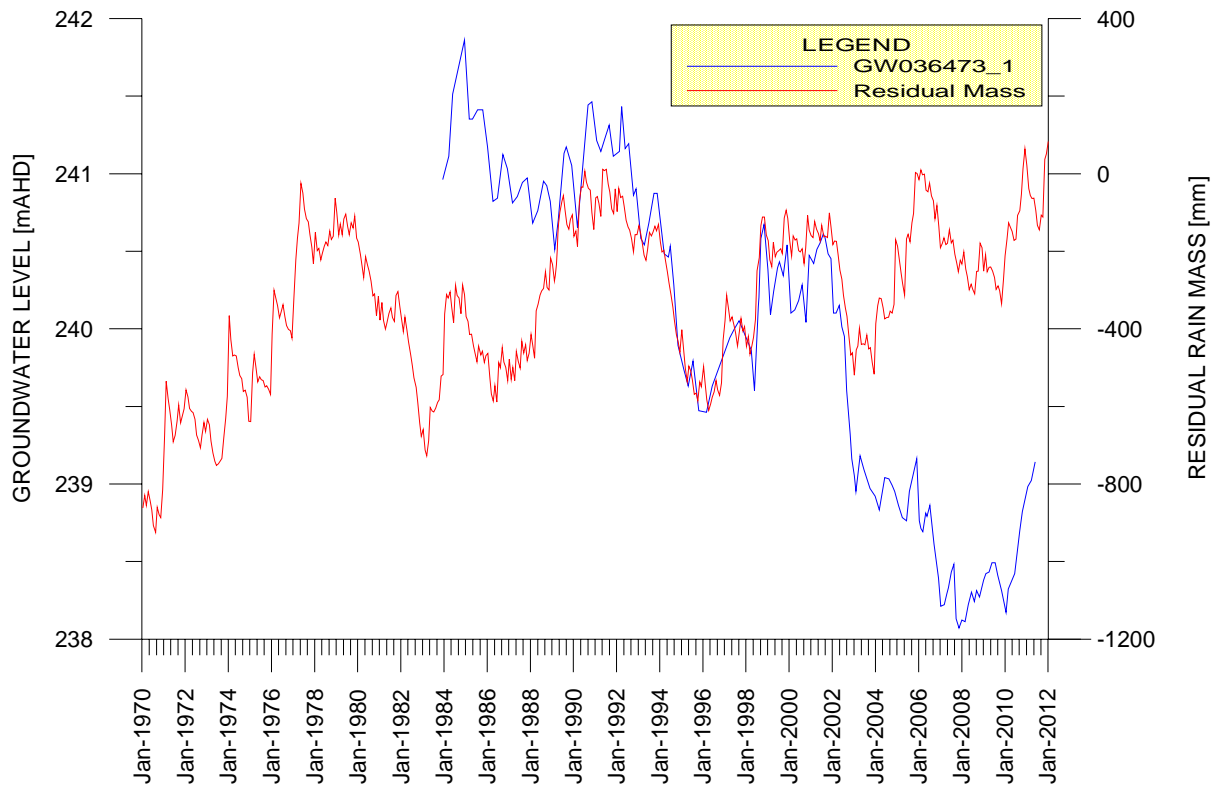


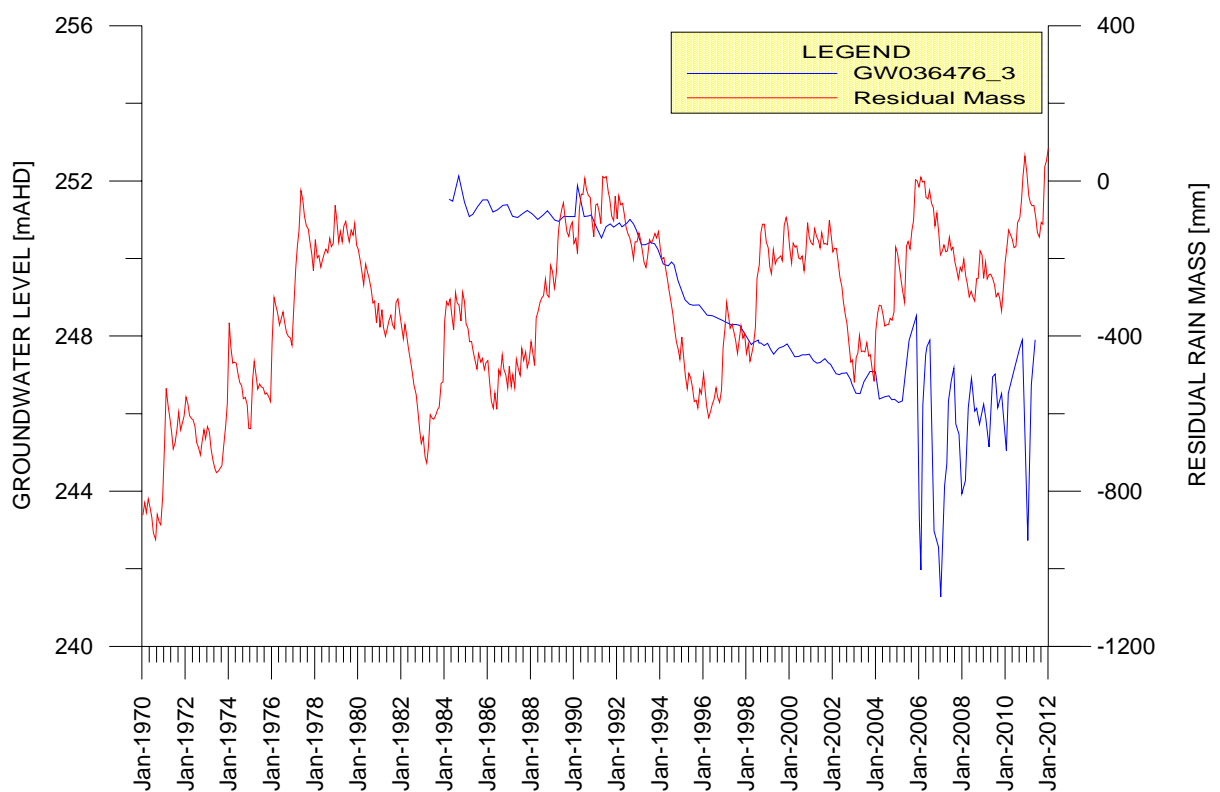
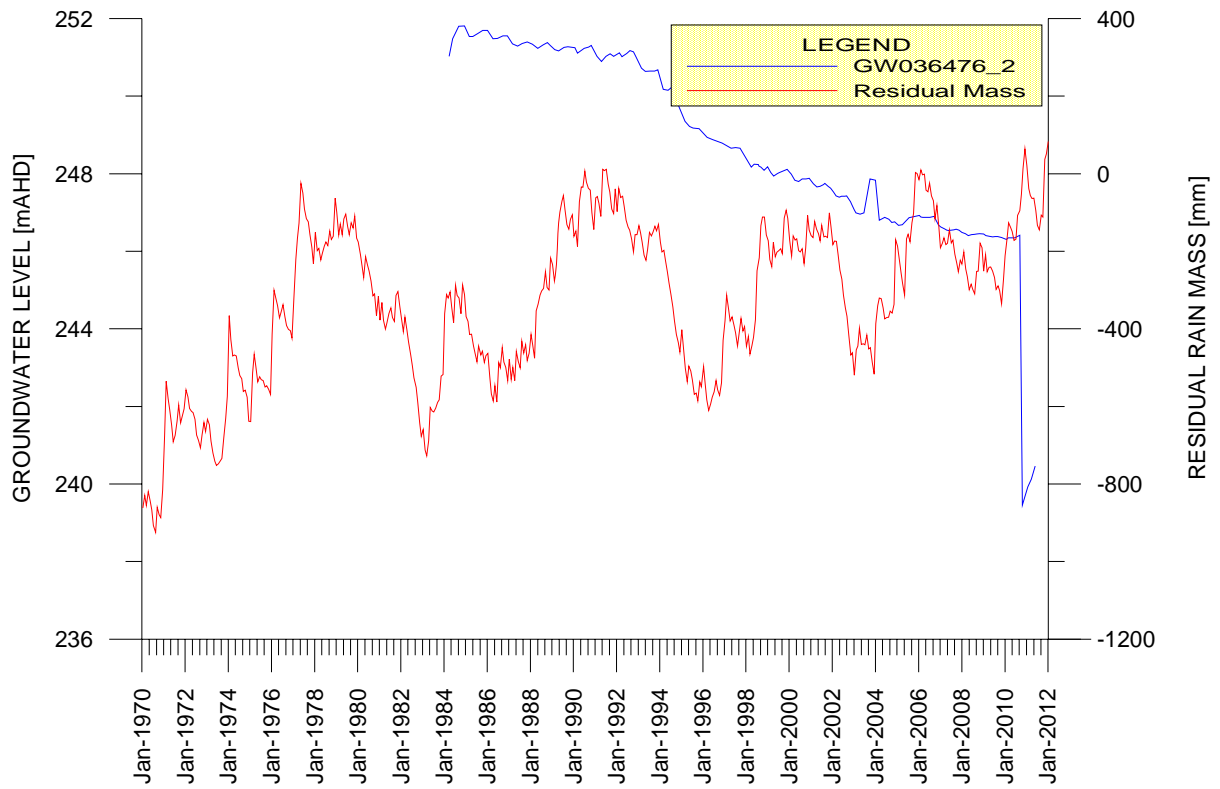


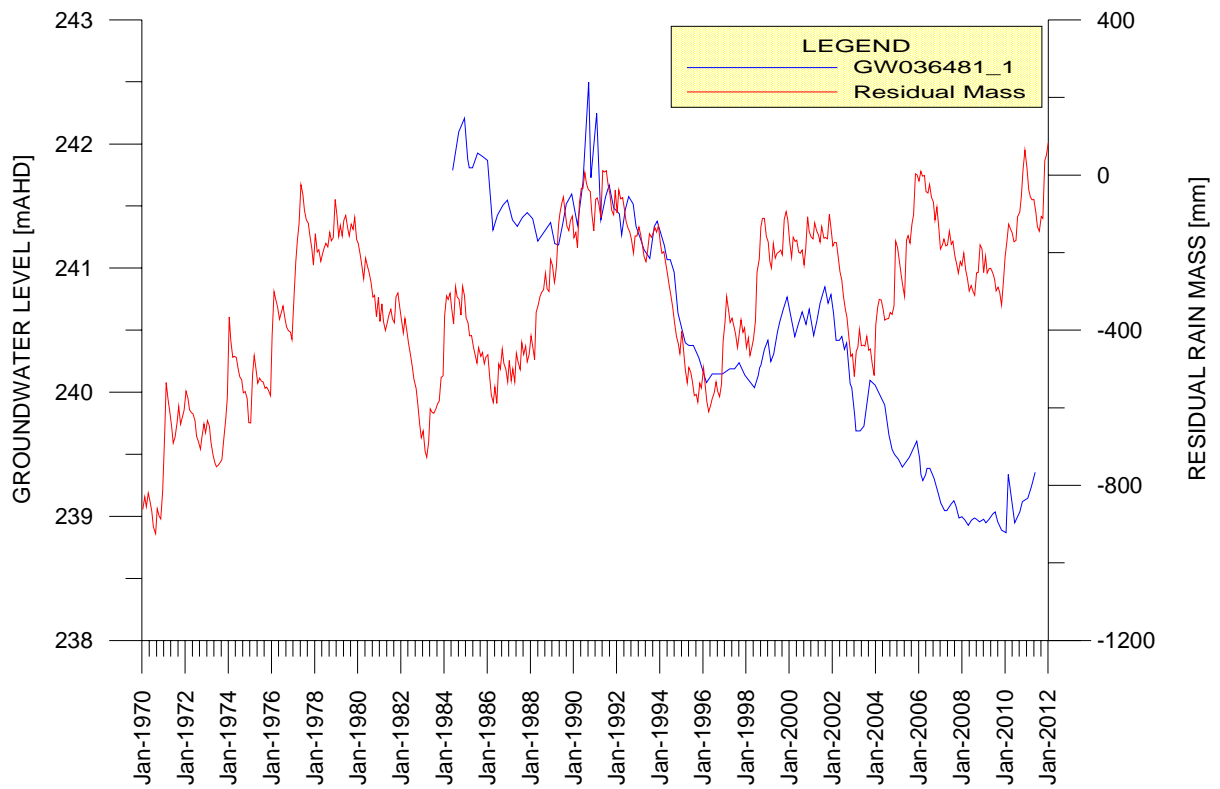
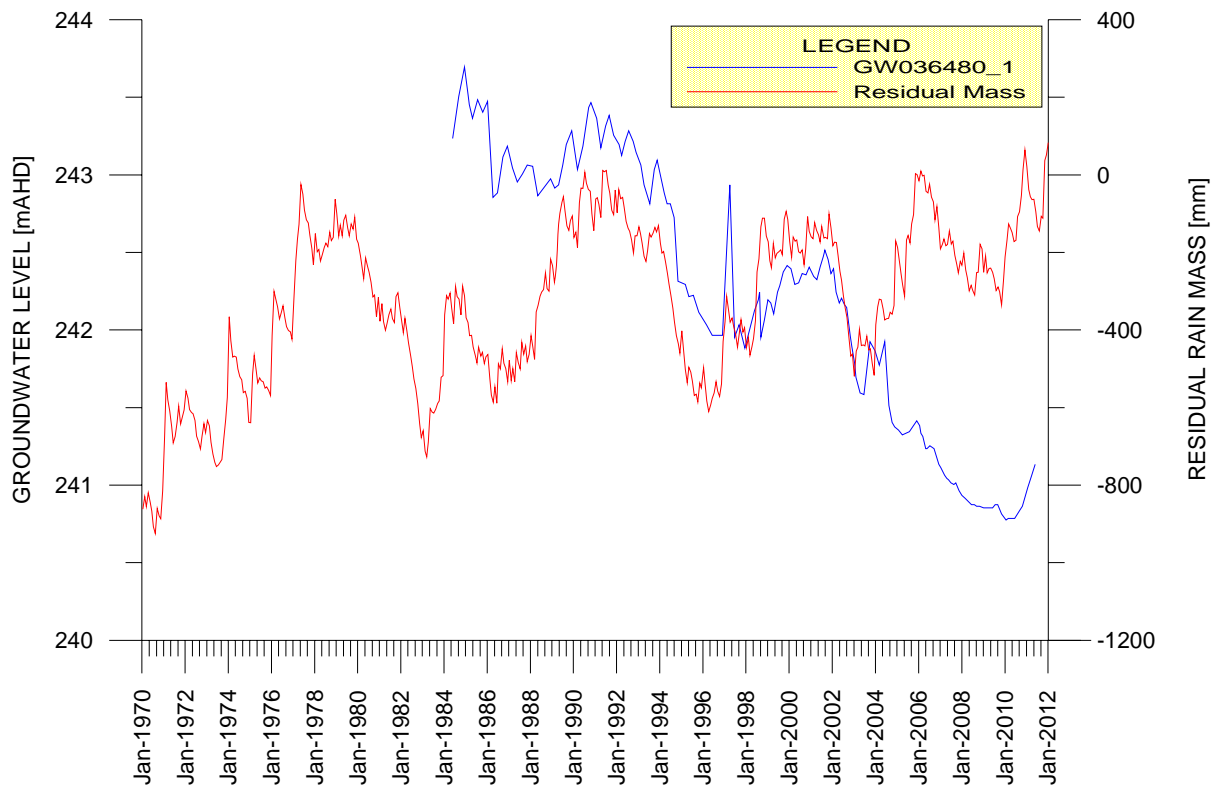


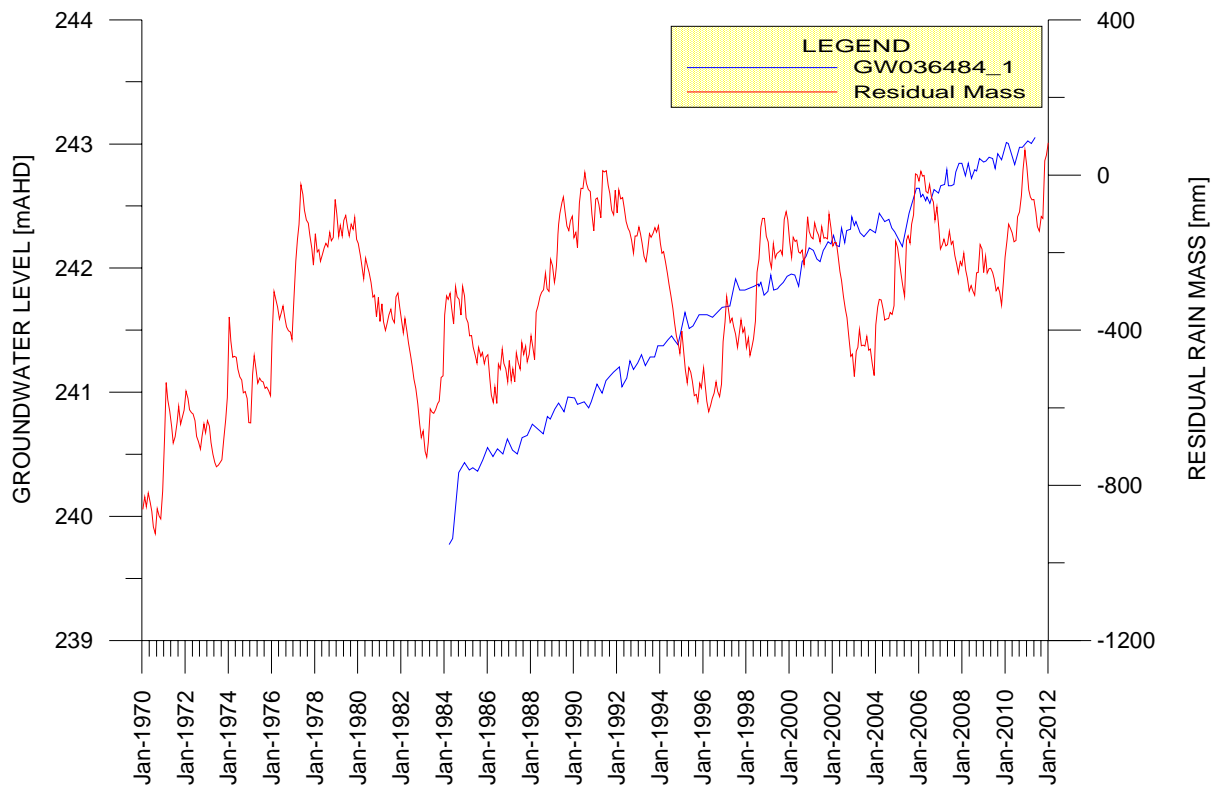
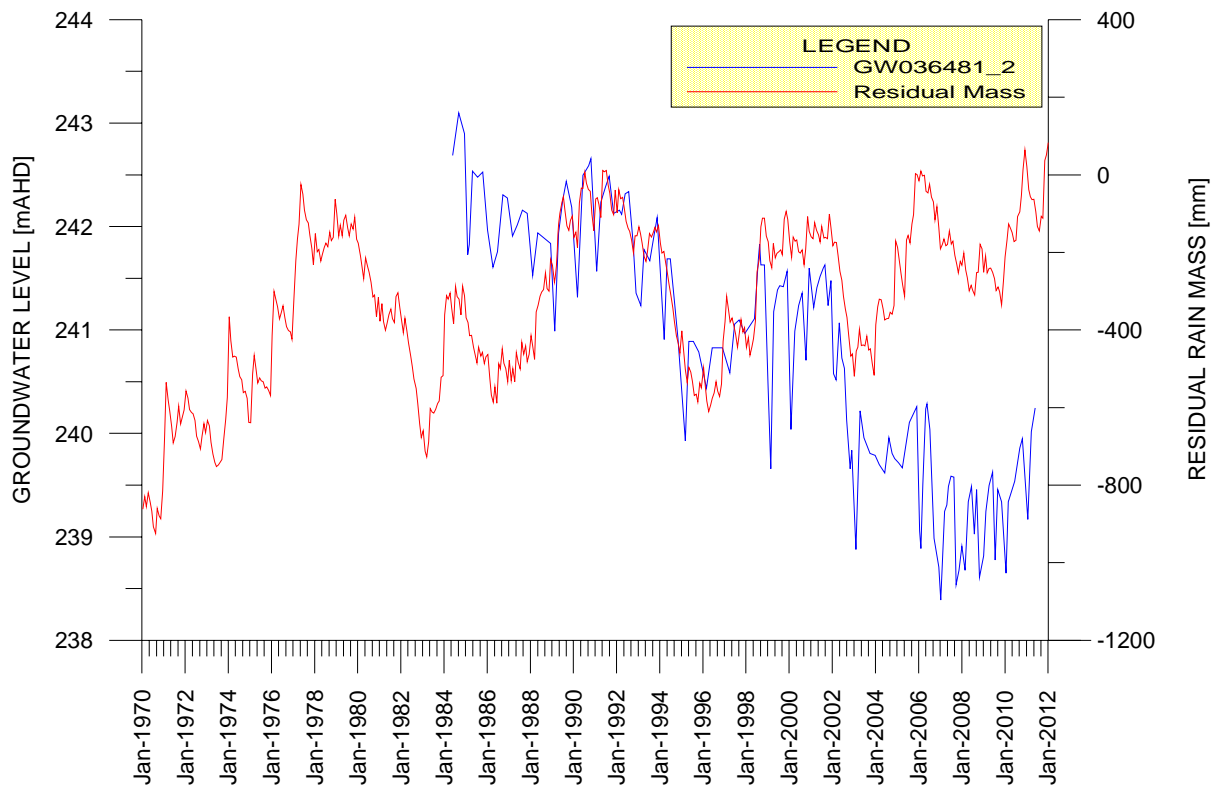


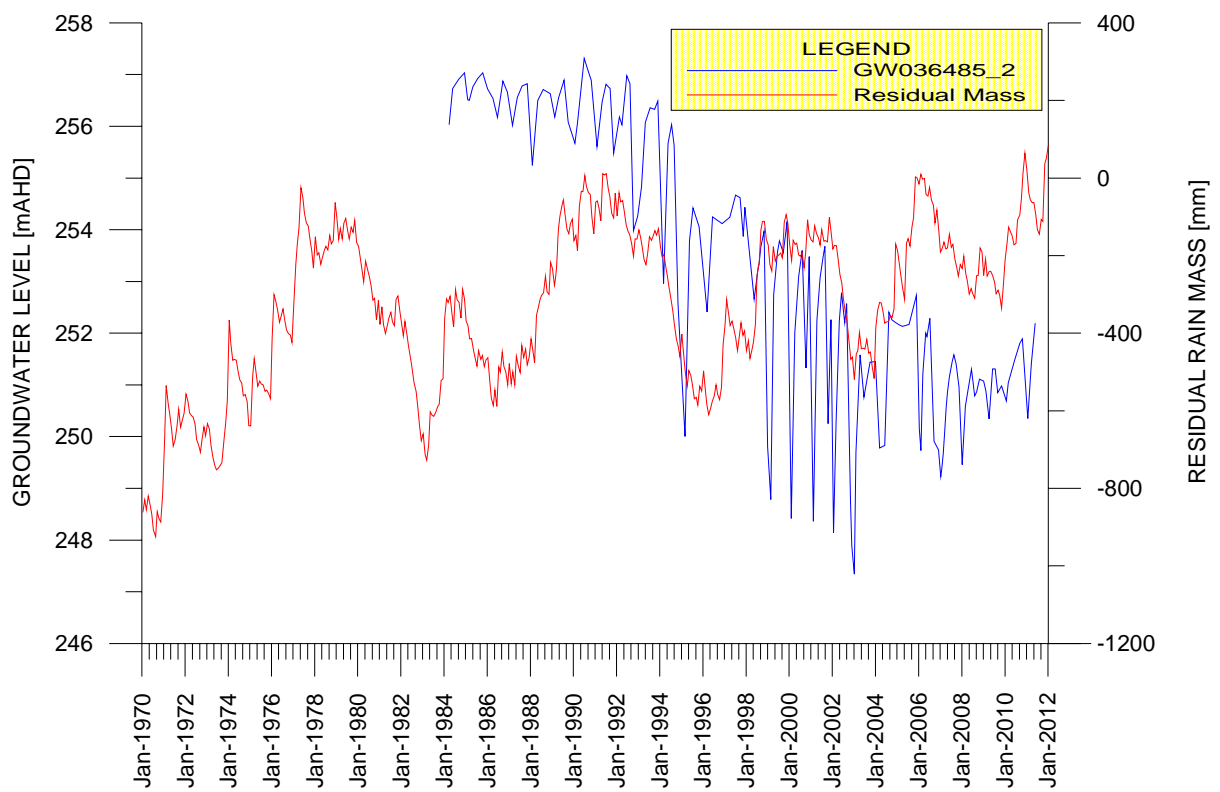
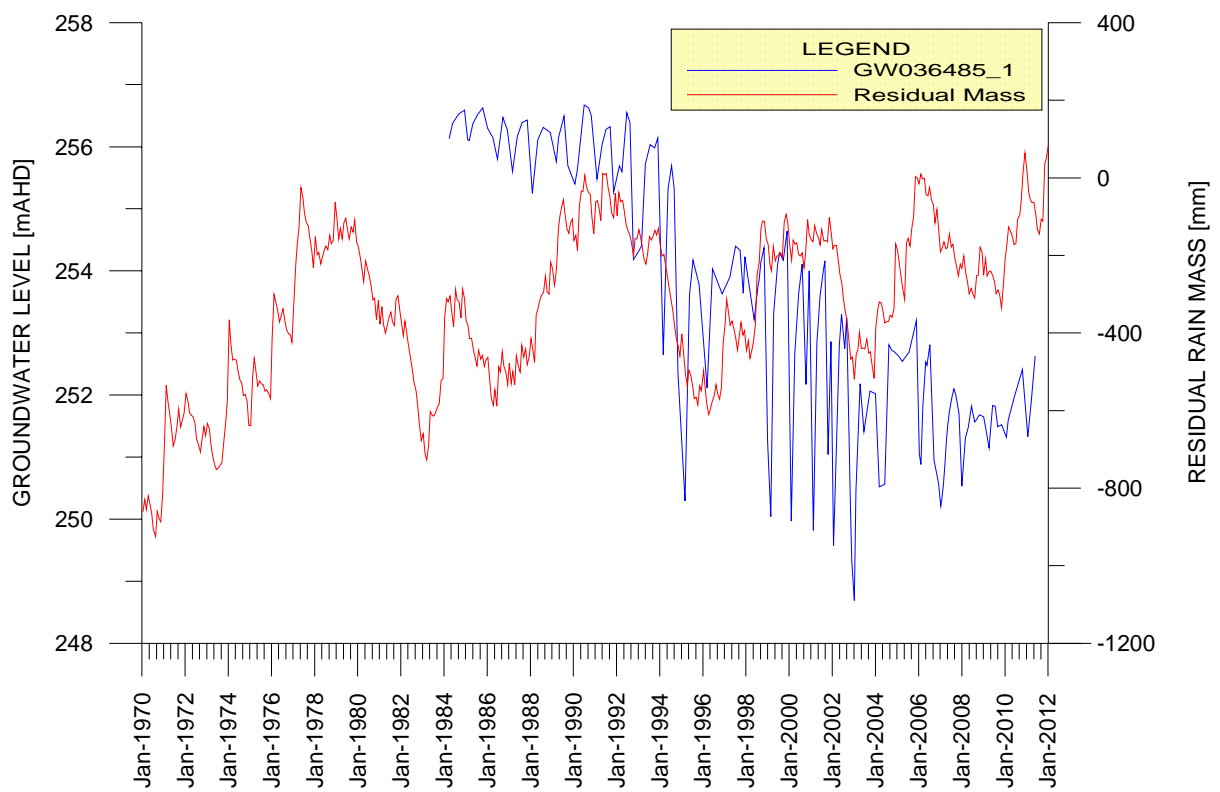


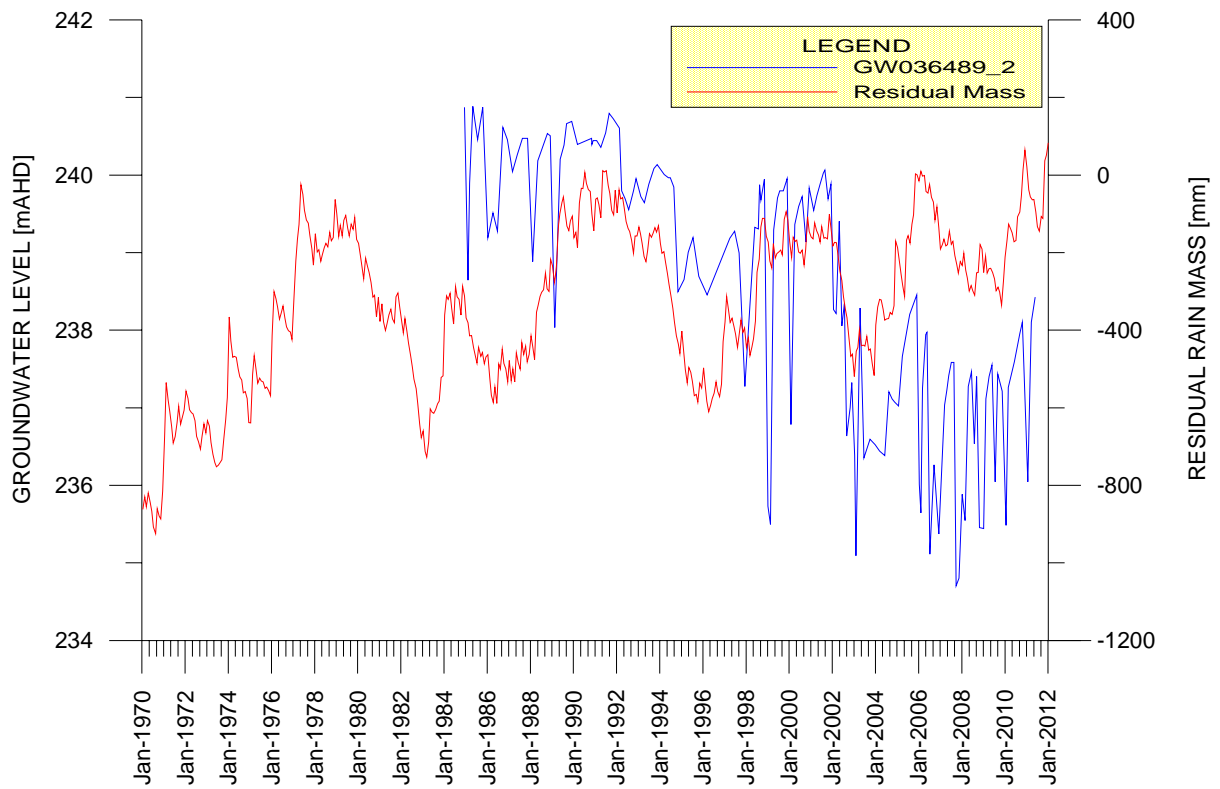
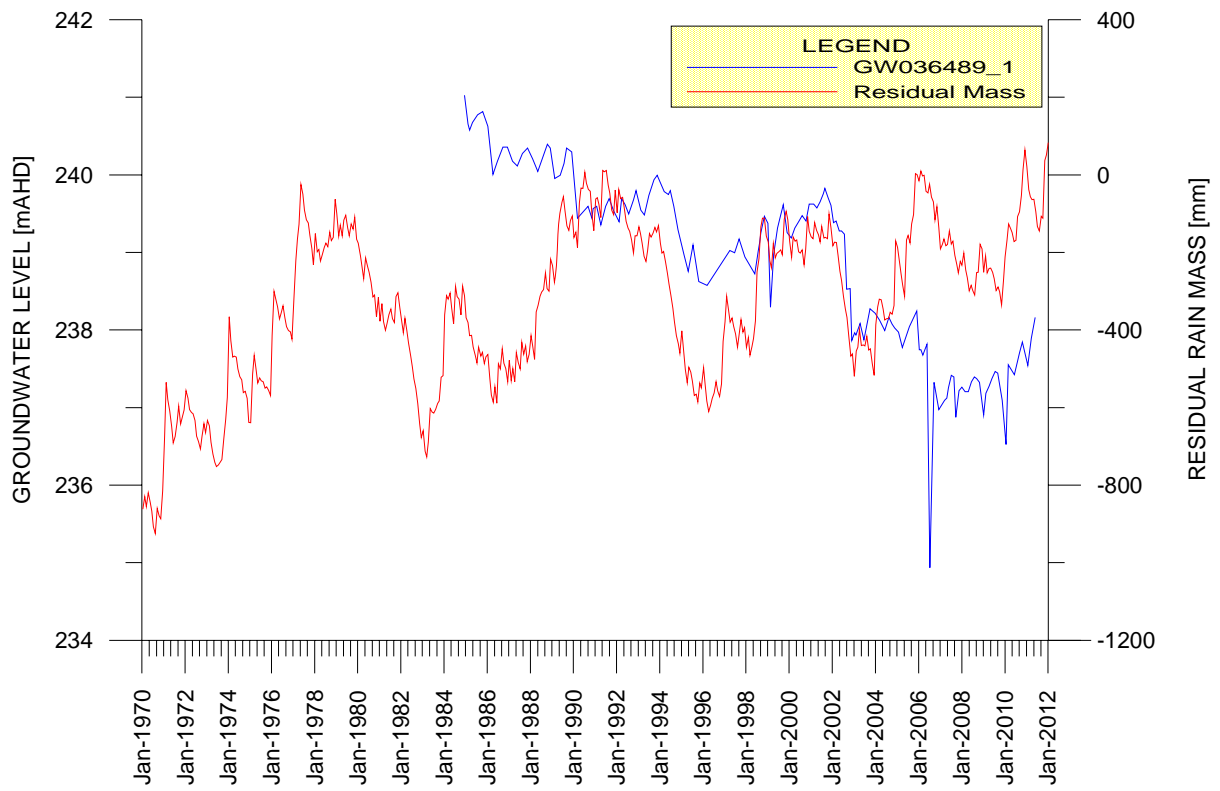


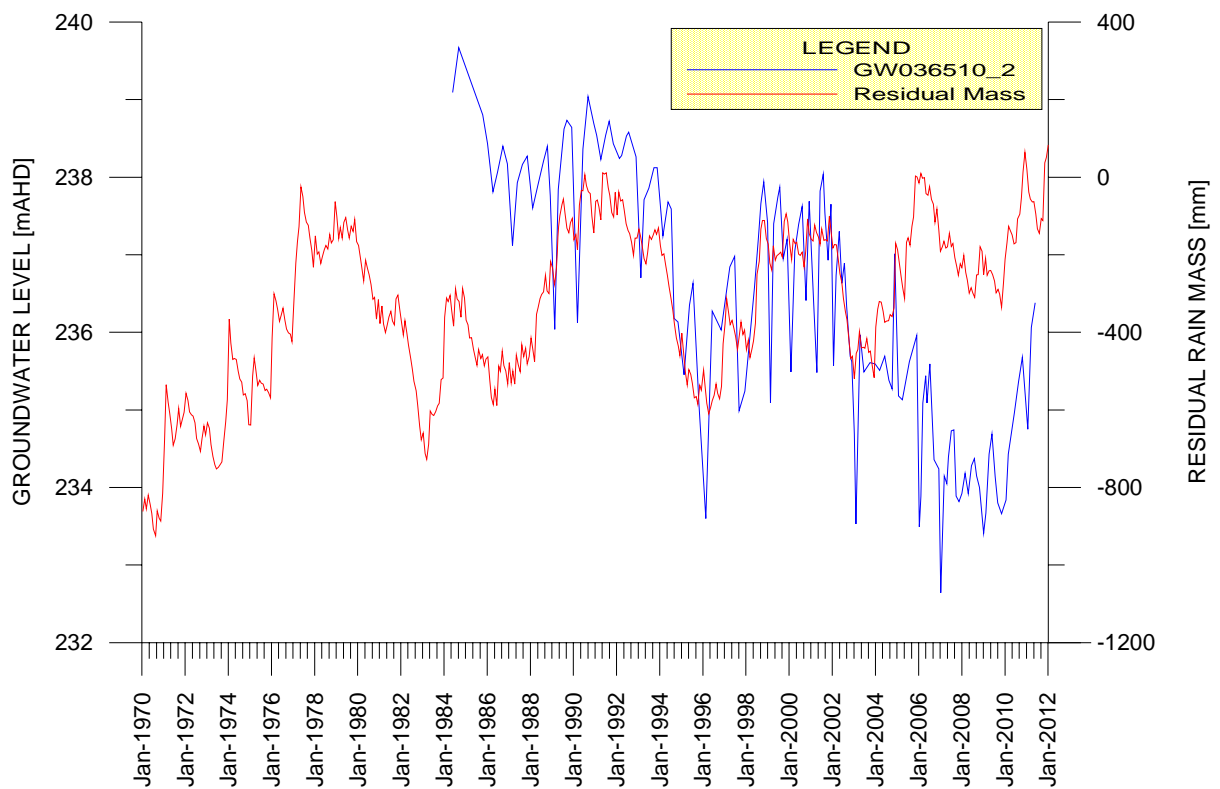
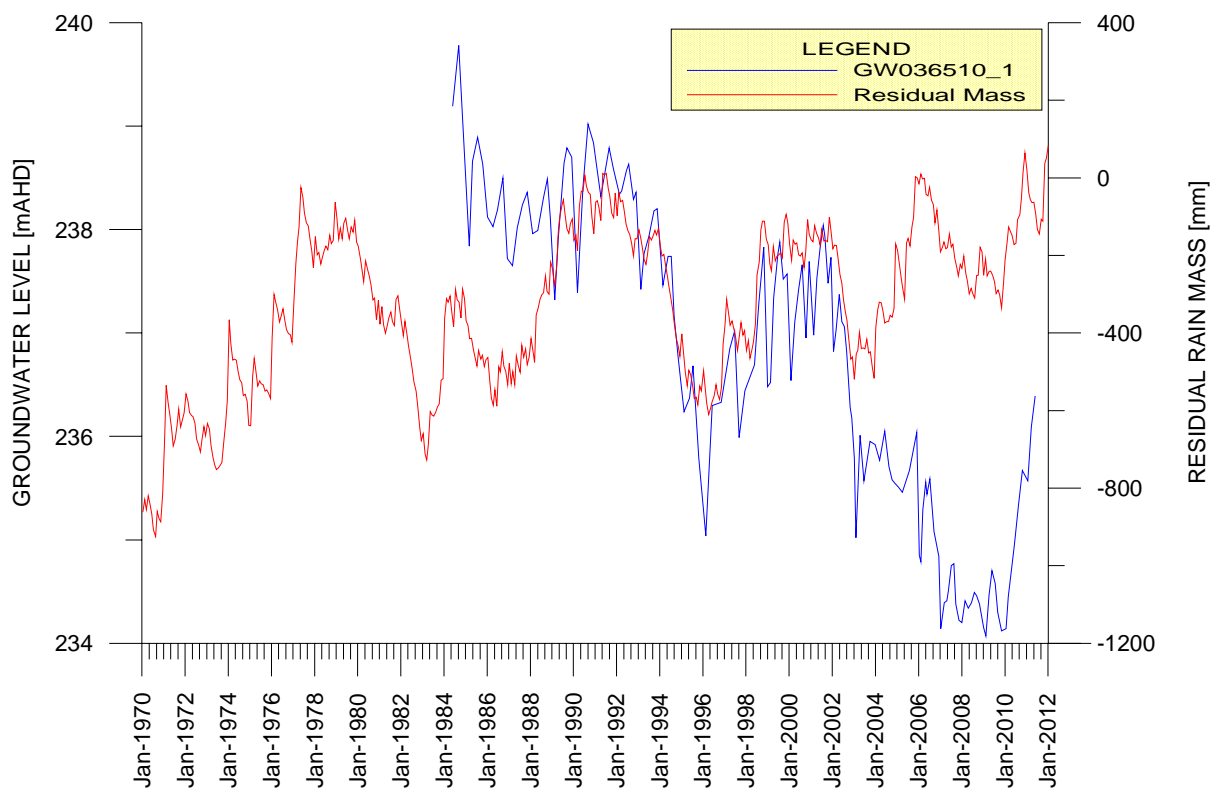


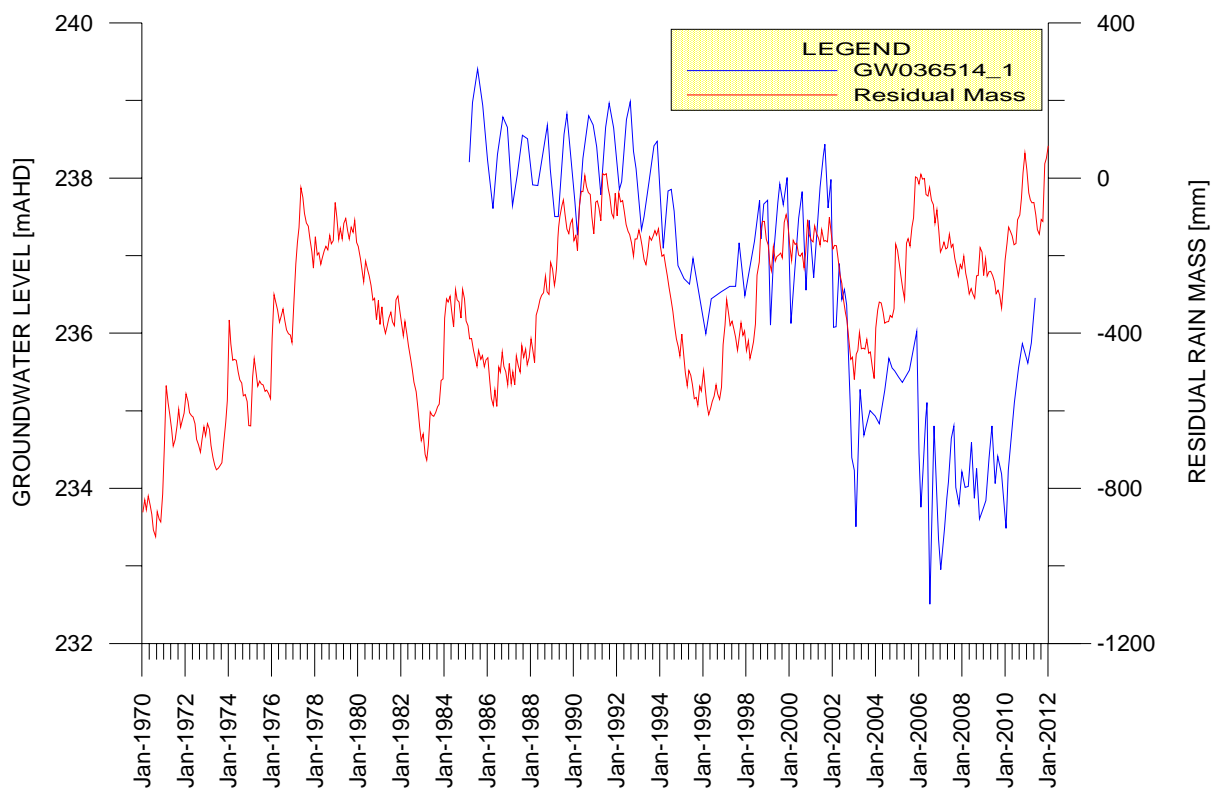
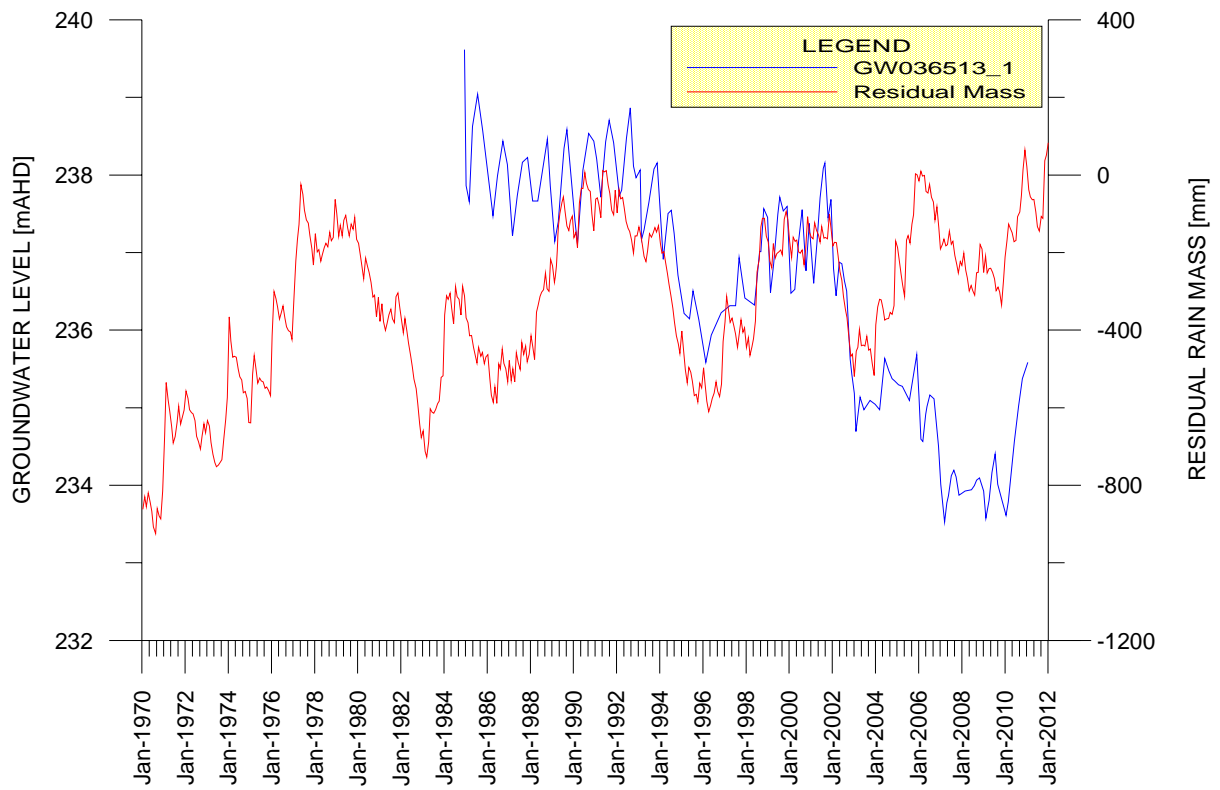


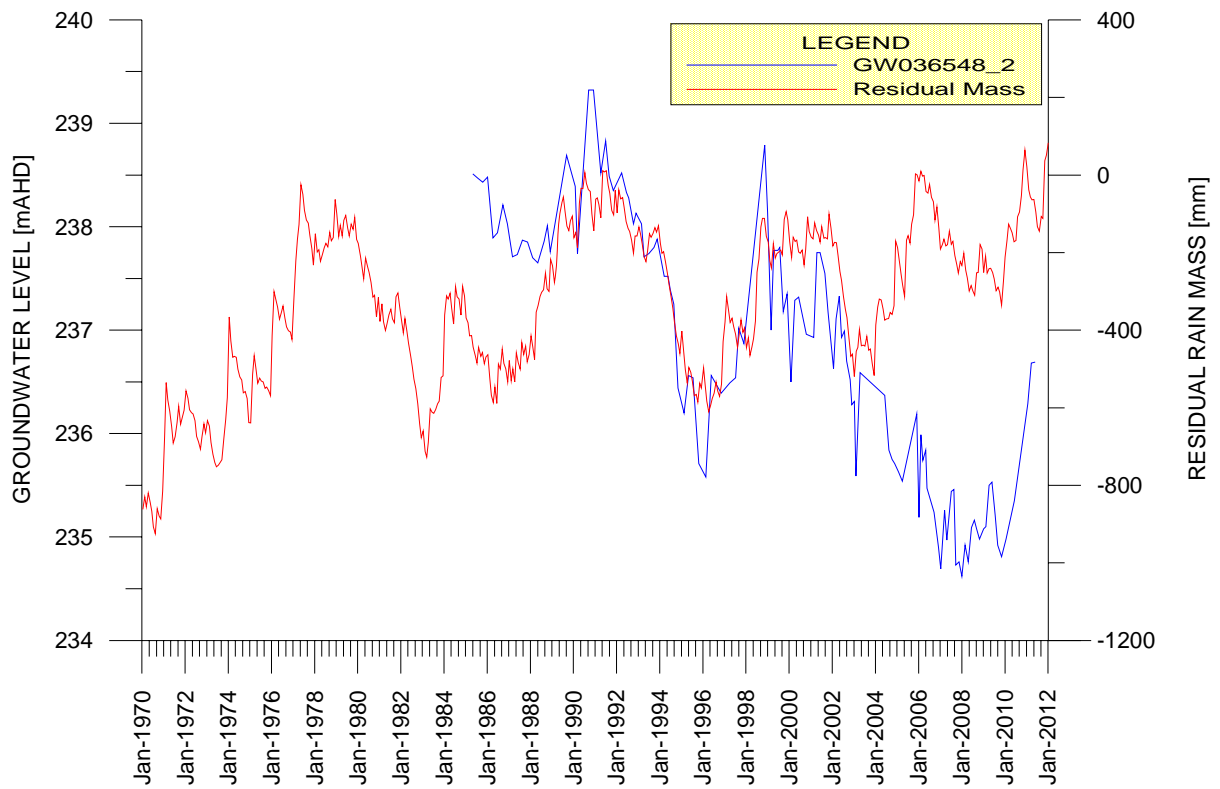
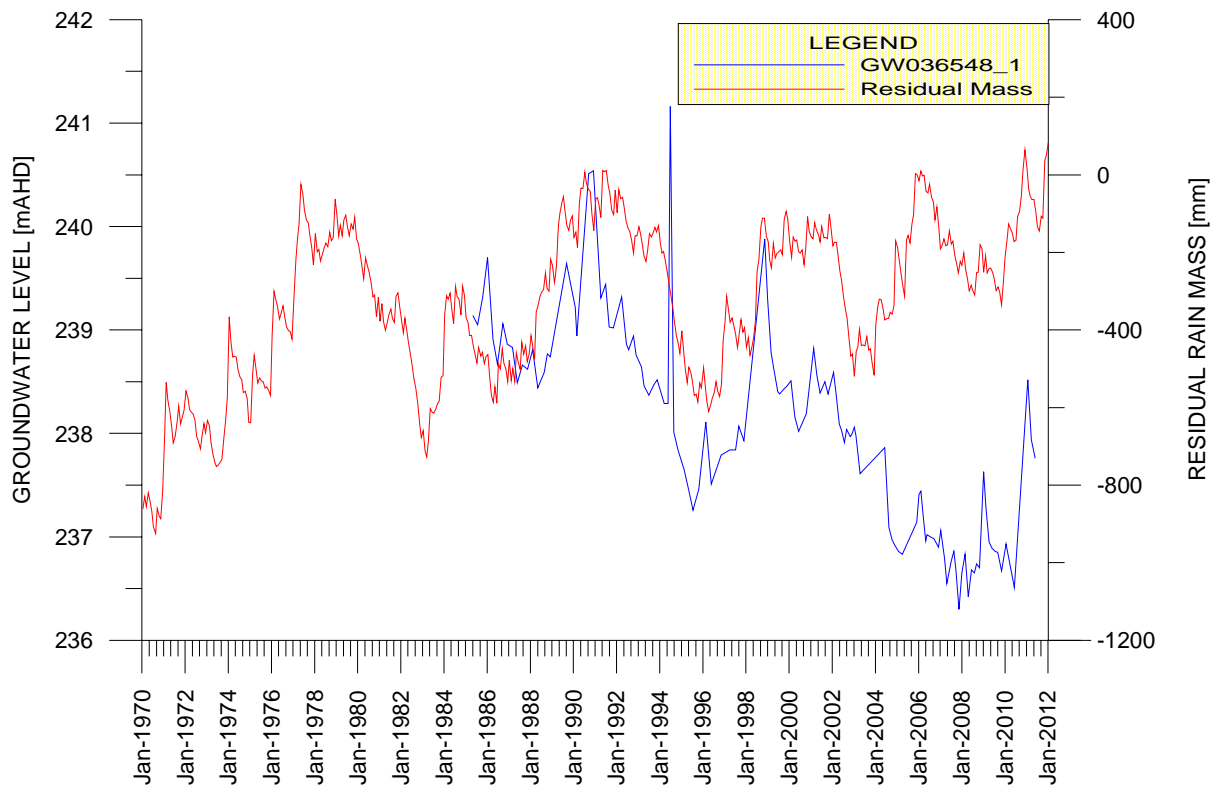


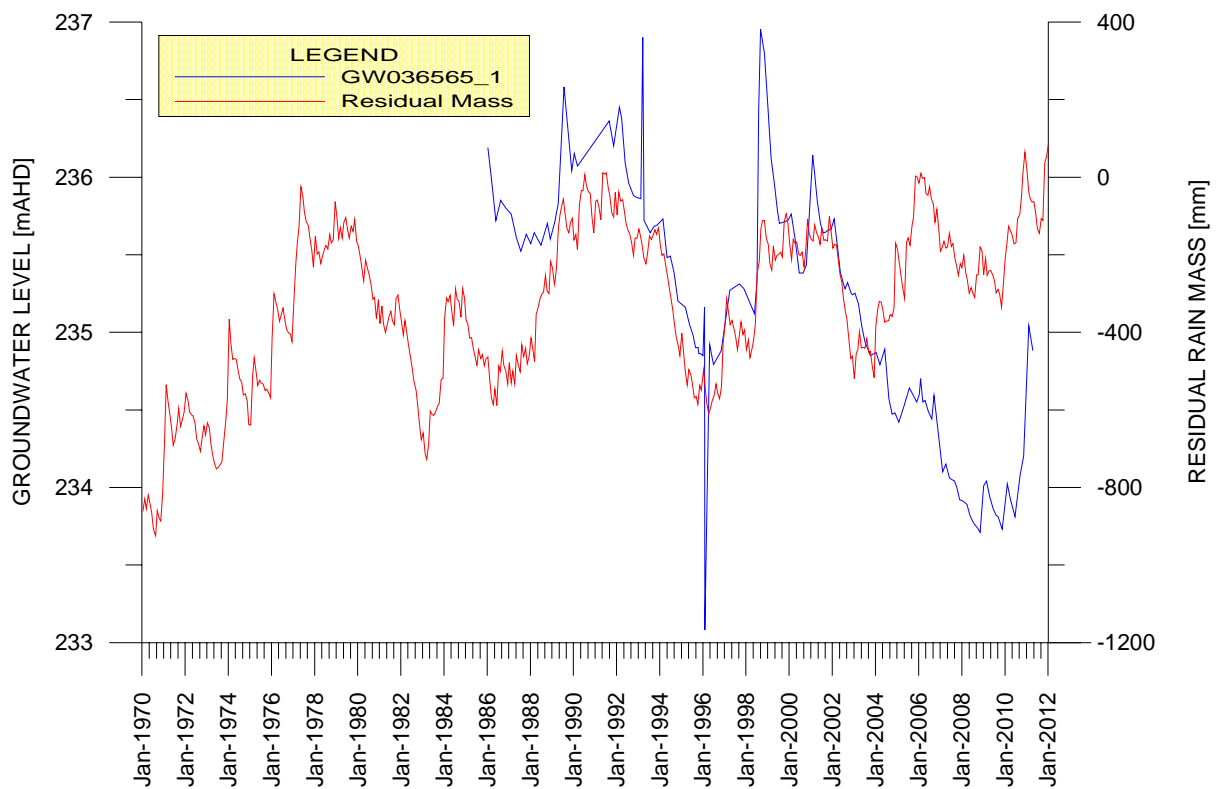
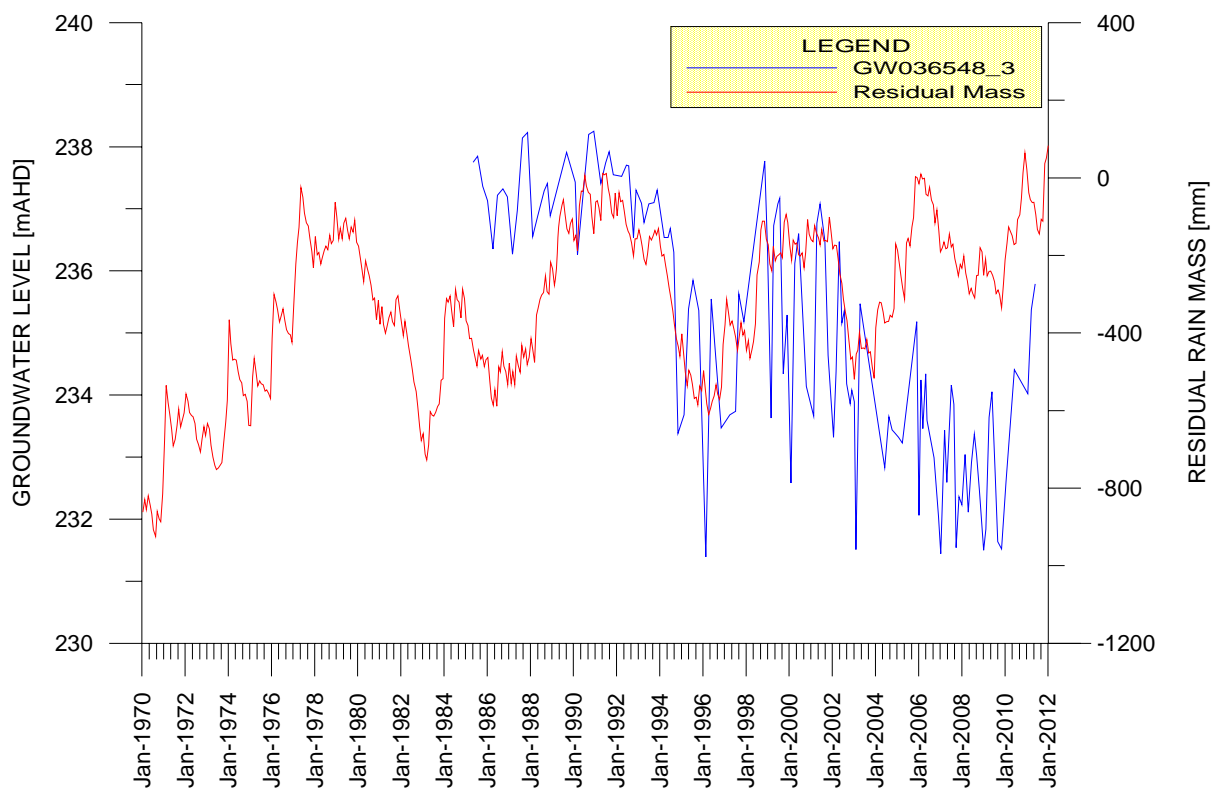


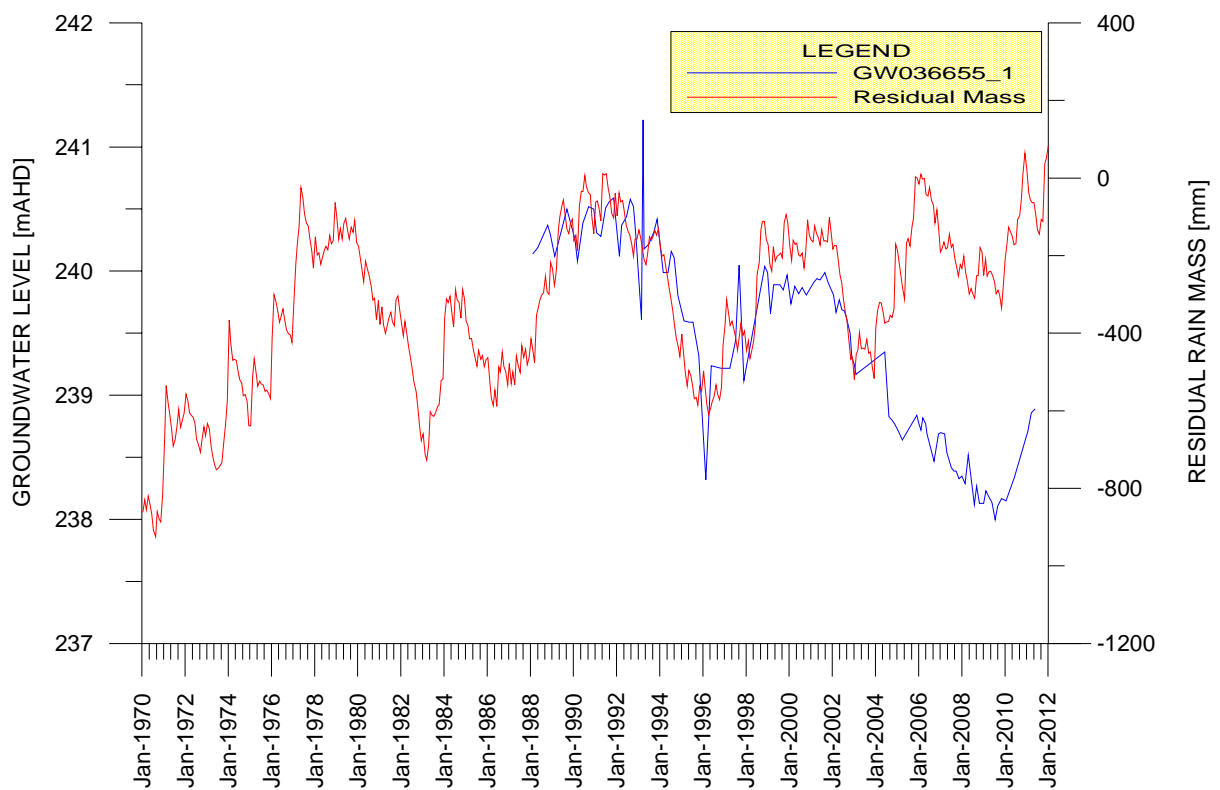
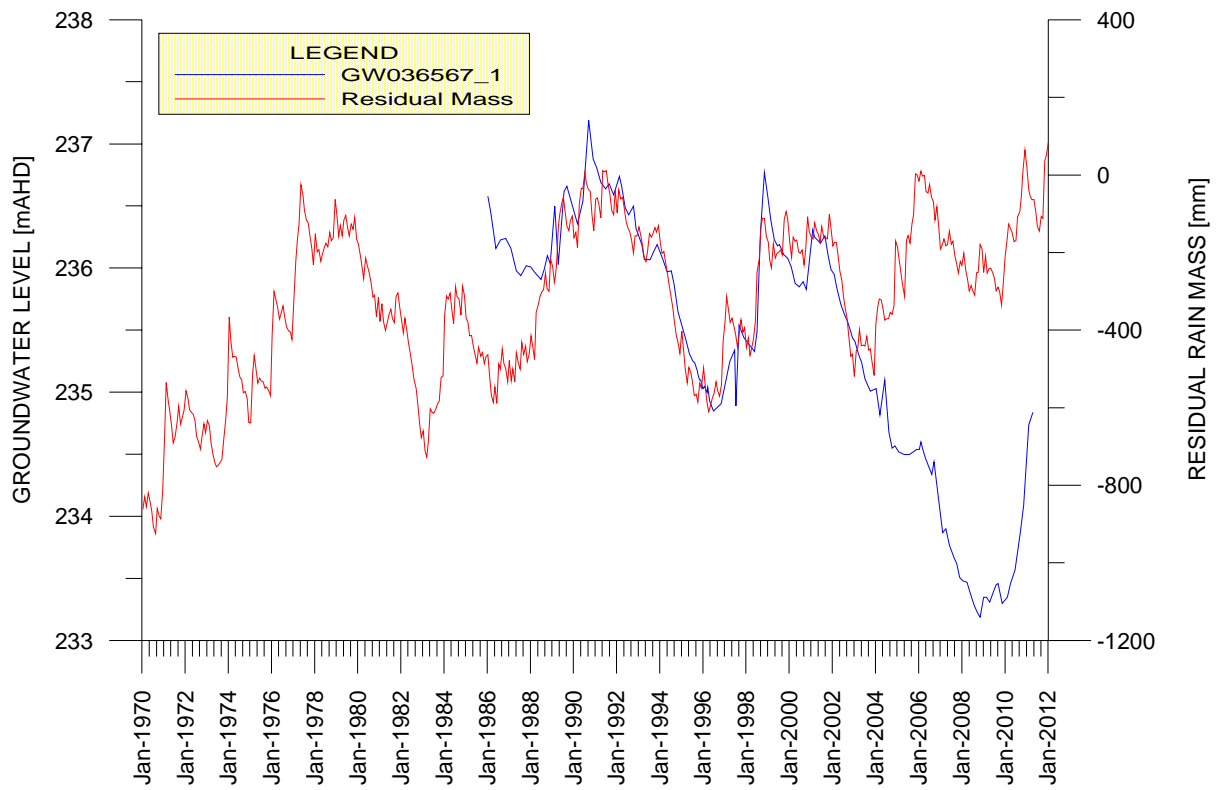


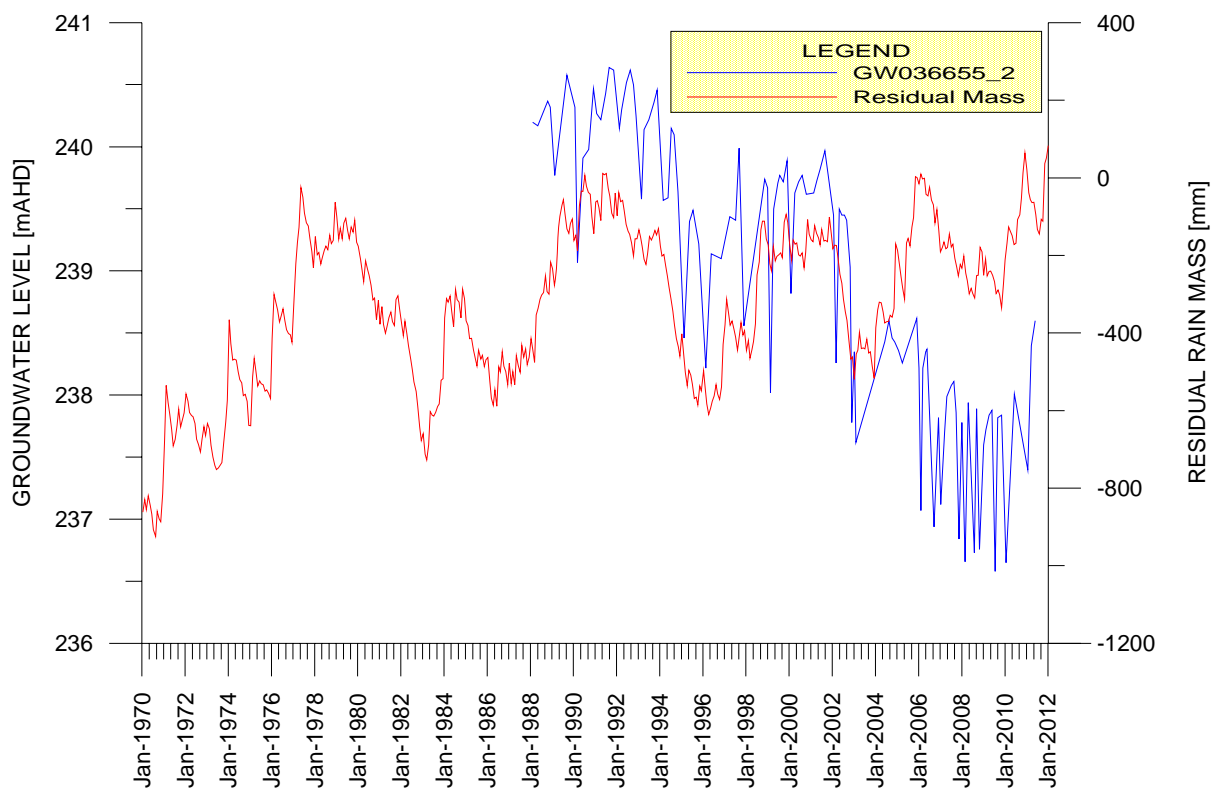








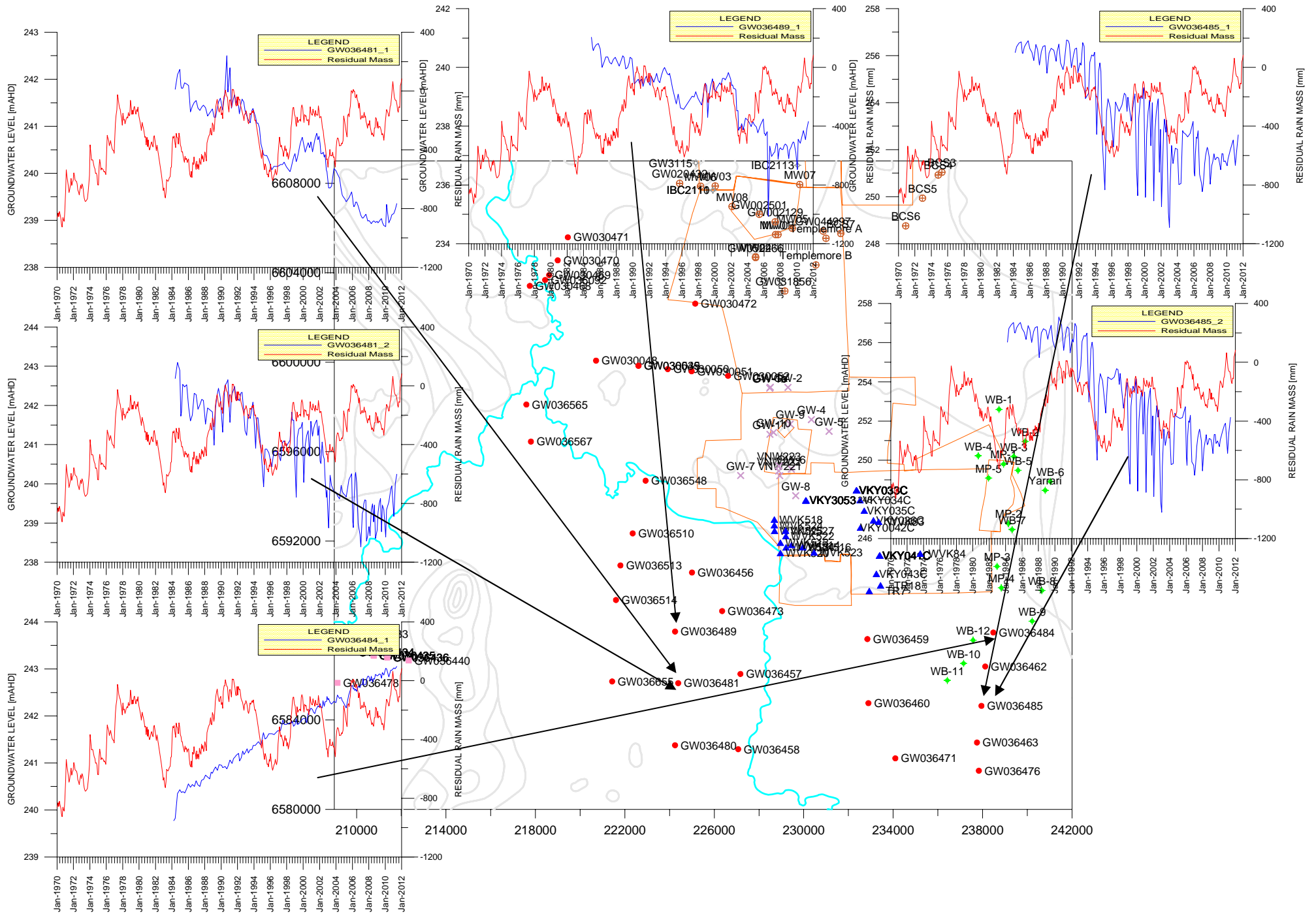


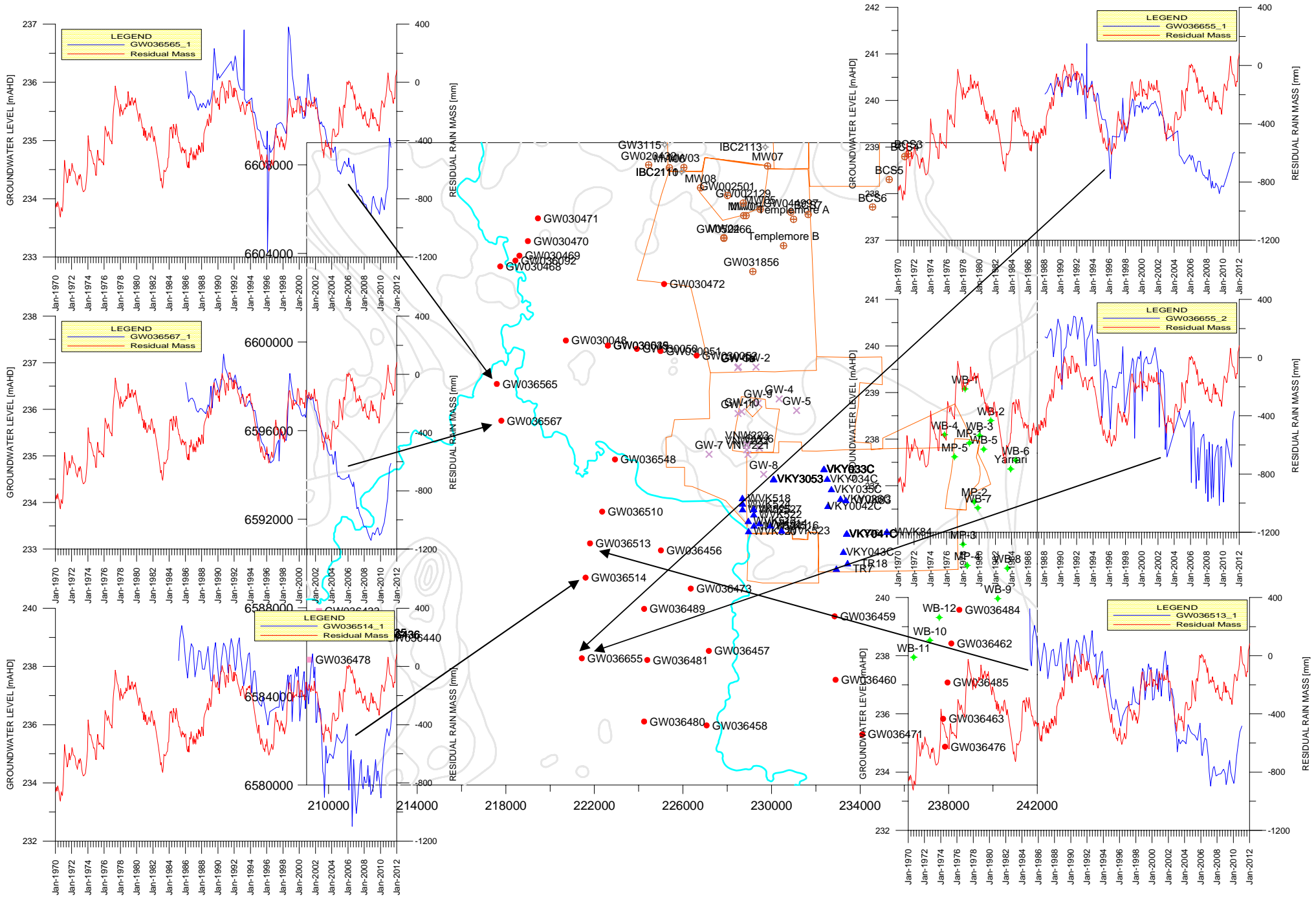


ATTACHMENT AC

Storyboard Figures for NOW Bores in Zone4



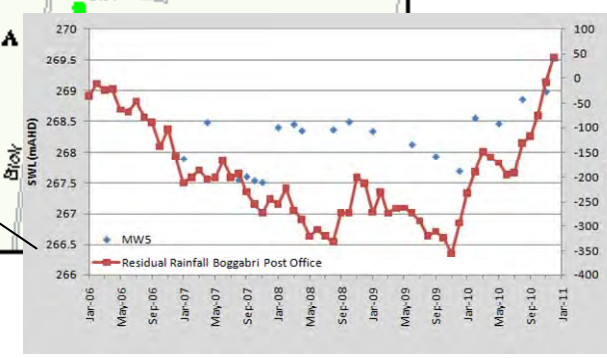
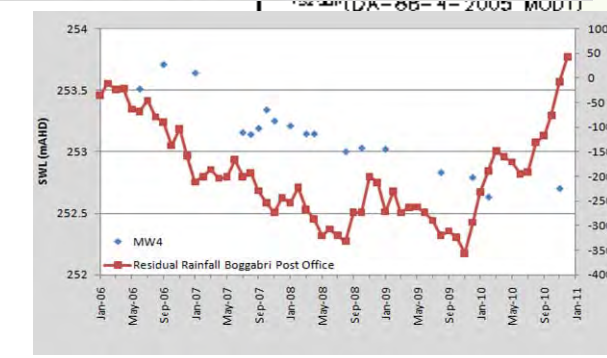
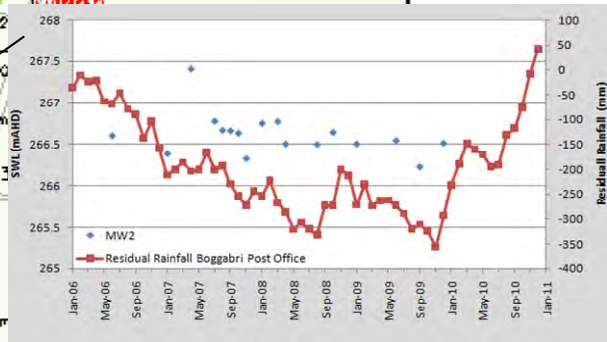
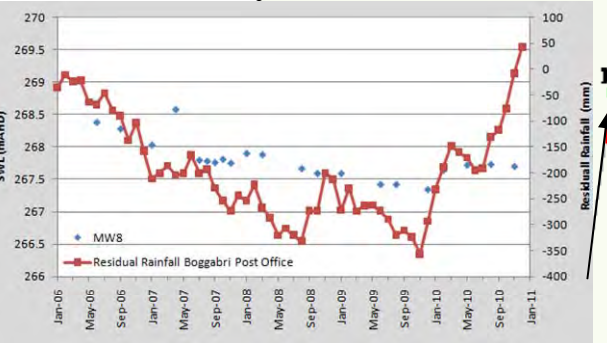
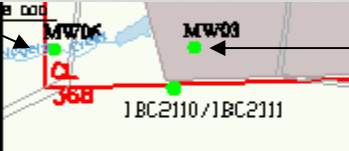
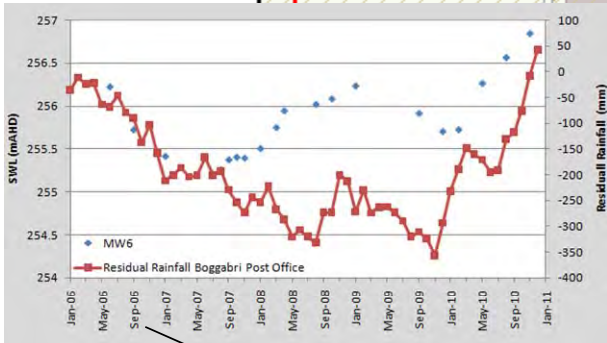
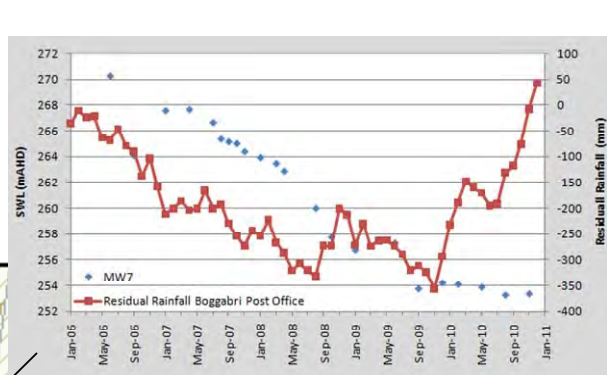
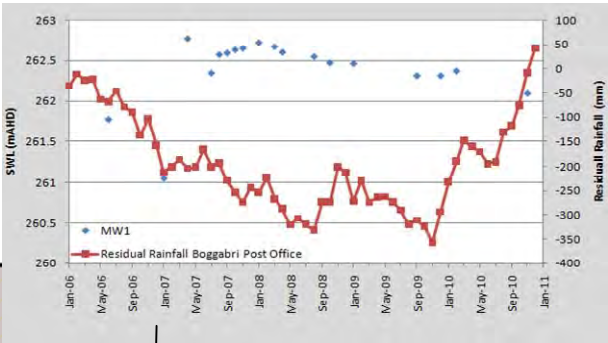
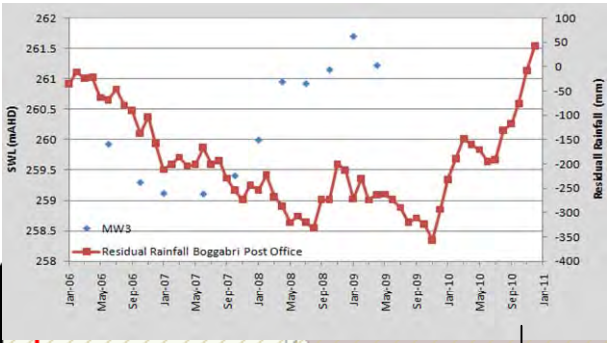




ATTACHMENT AD

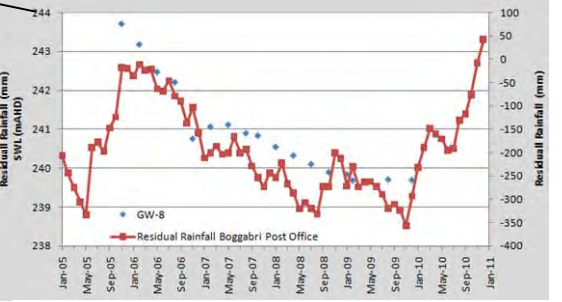
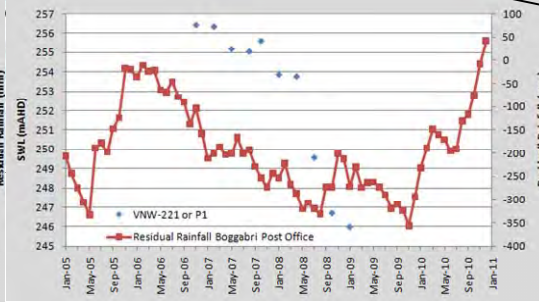
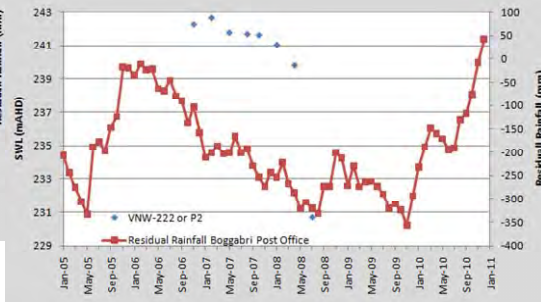
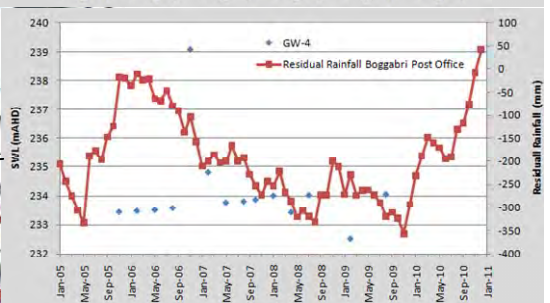
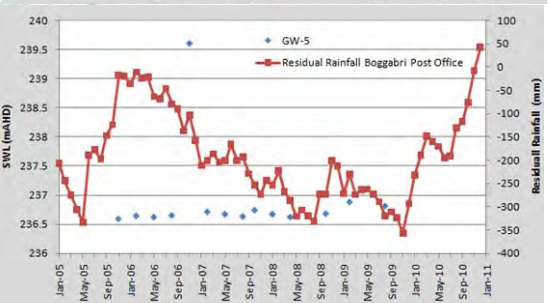
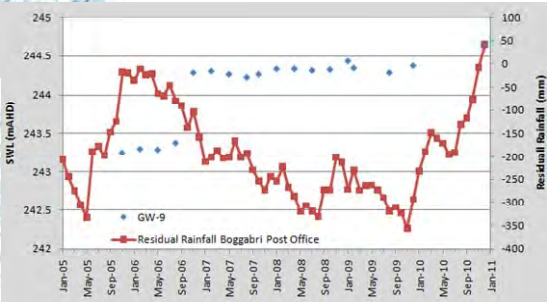
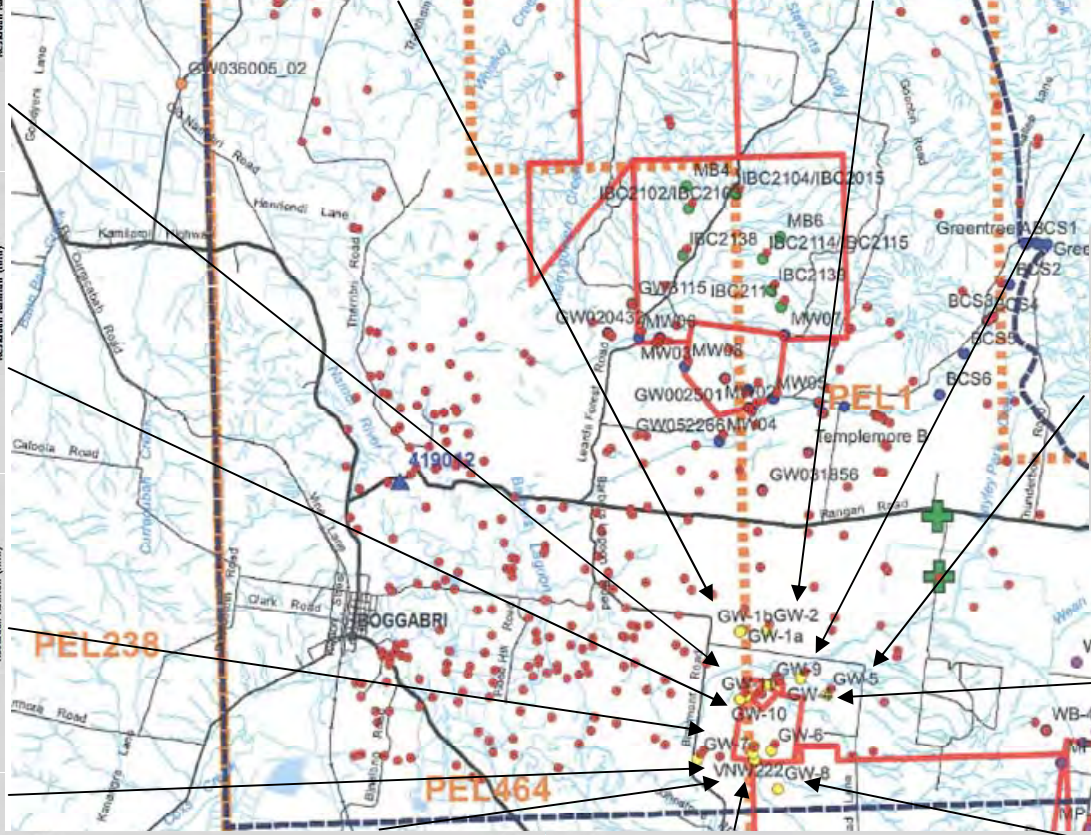
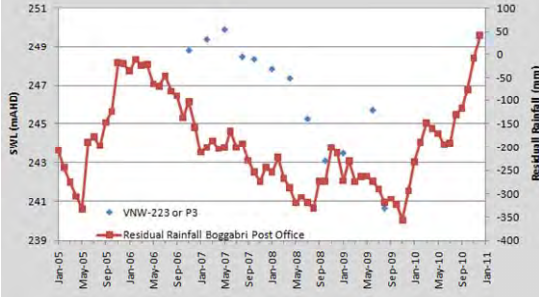
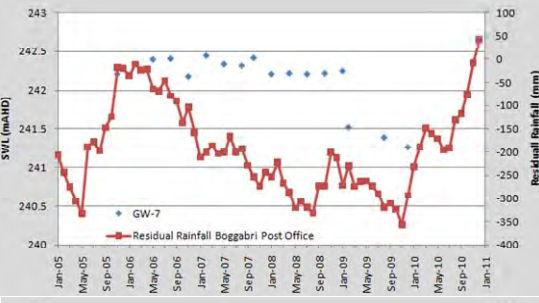
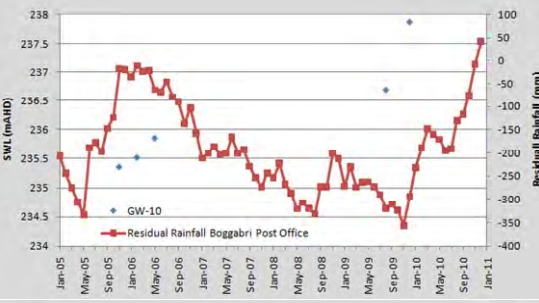
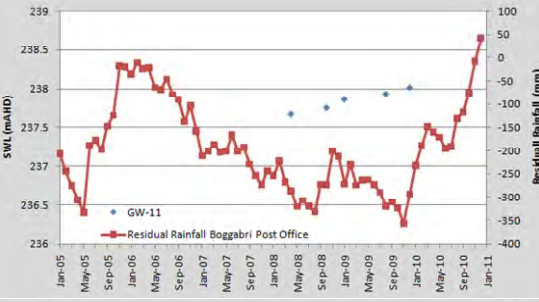
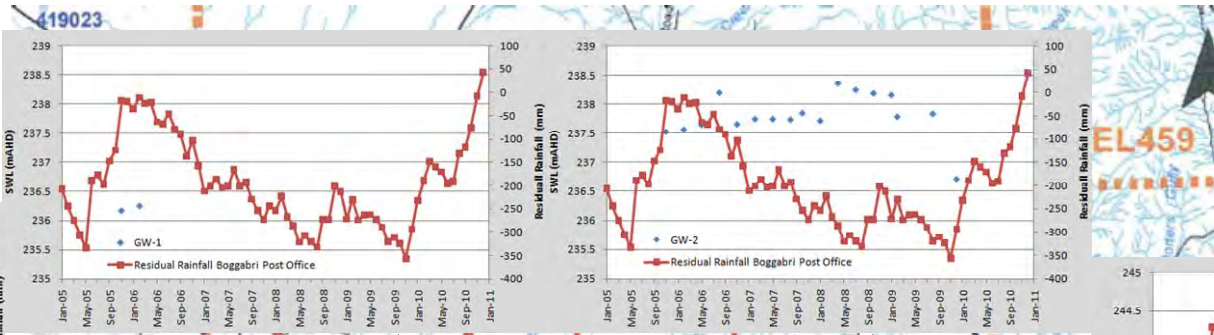
**Storyboard Figures
for Tarrawonga
Bores**





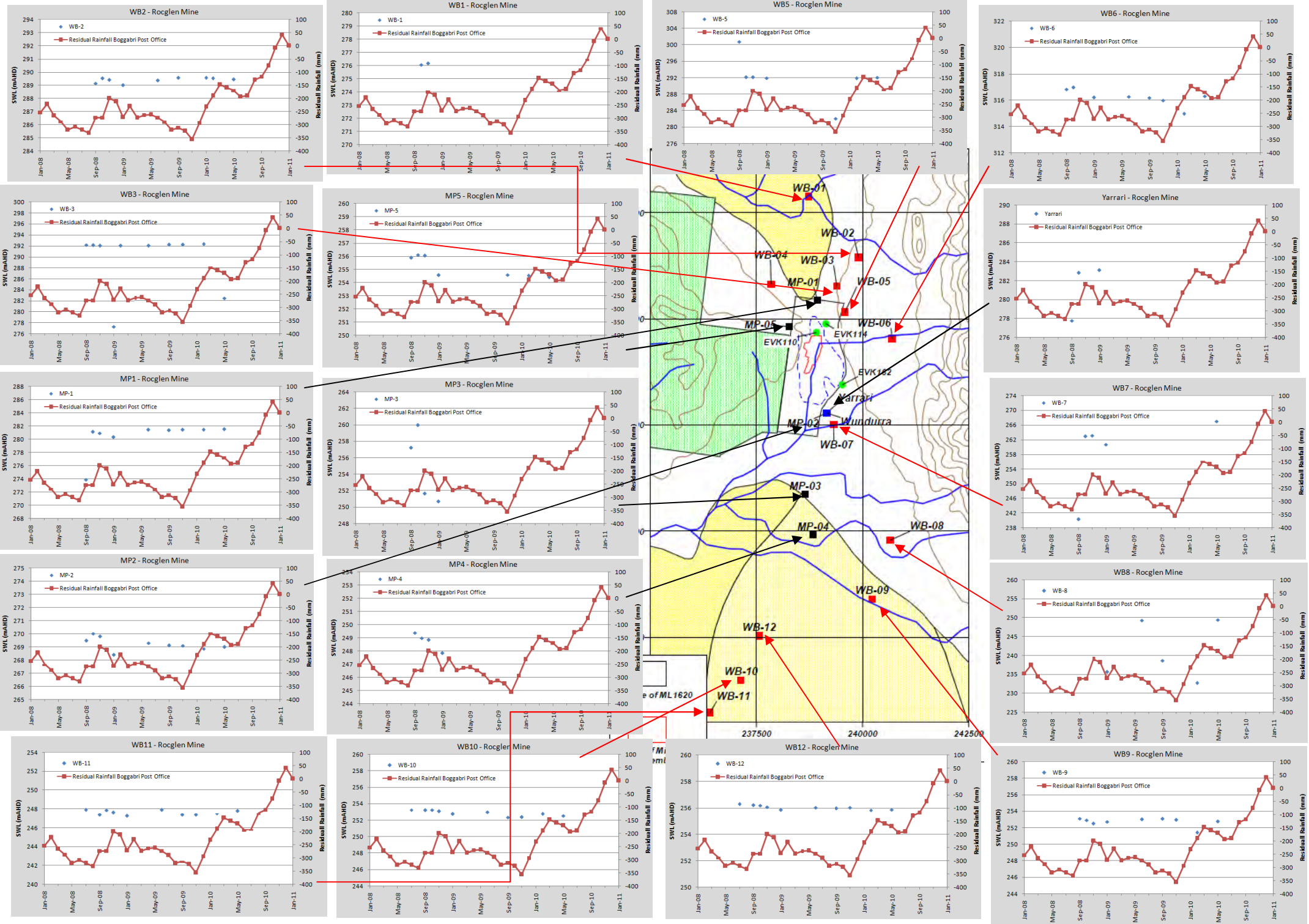
ATTACHMENT AE

Storyboard Figures for Canyon Bores



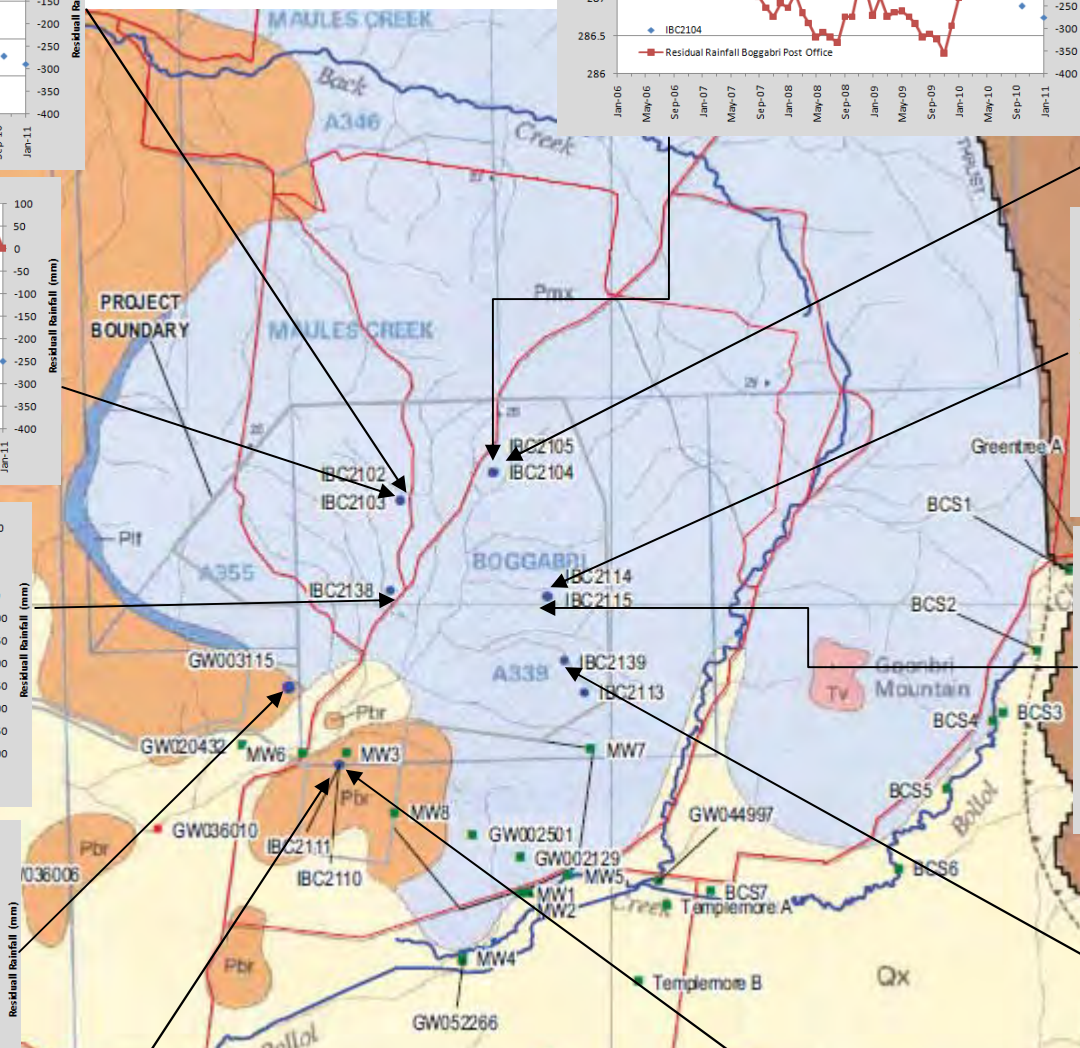
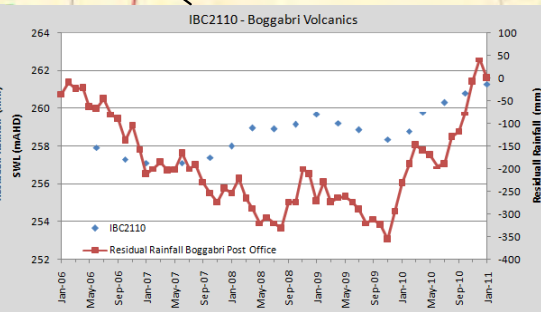
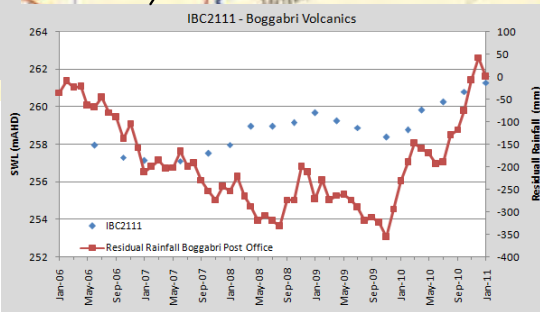
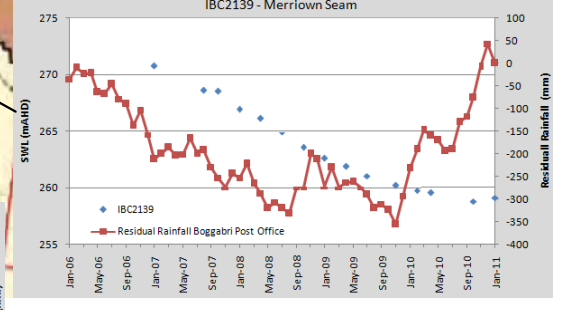
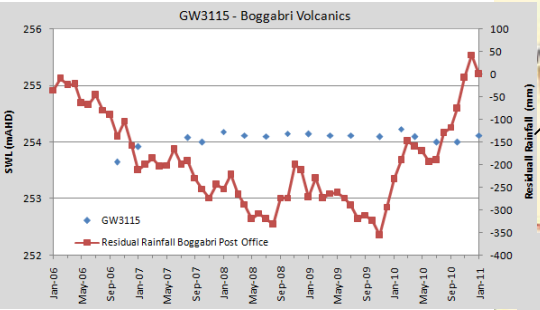
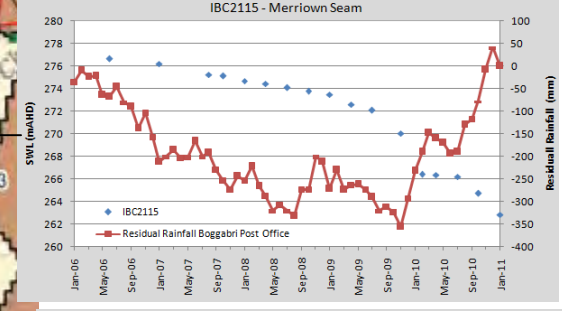
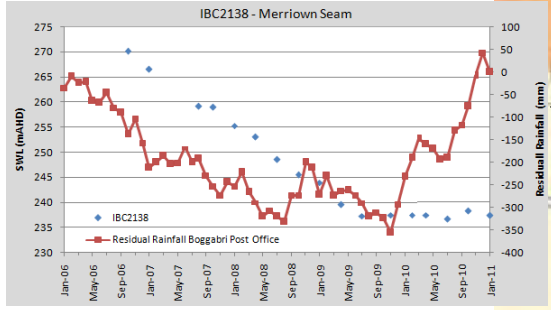
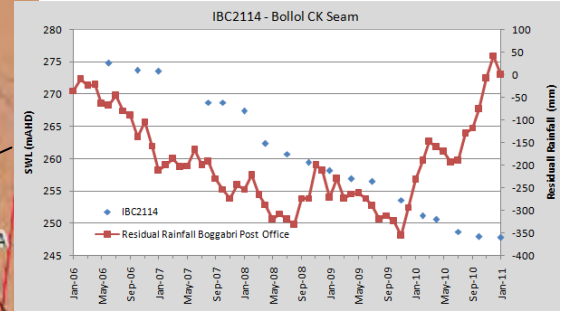
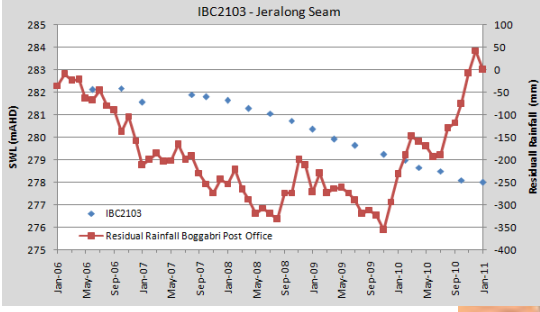
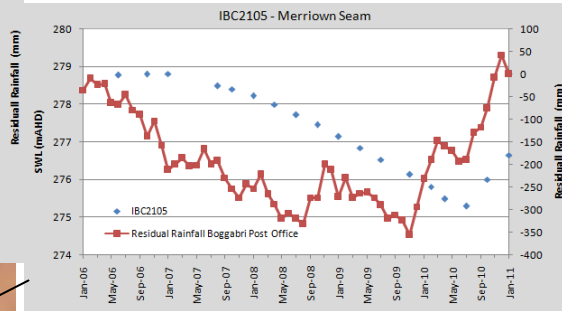
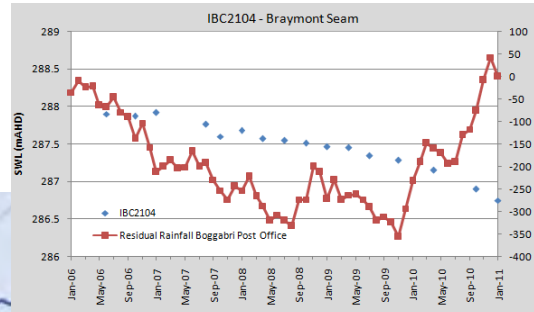
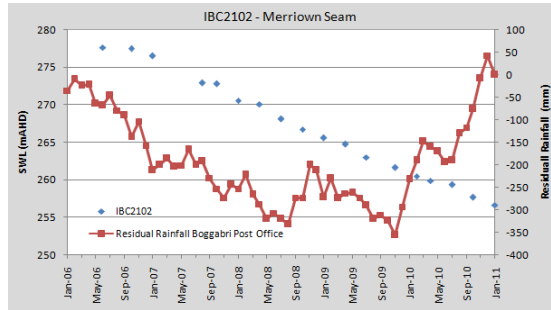
ATTACHMENT AF

Storyboard Figures for Rocglen Bores



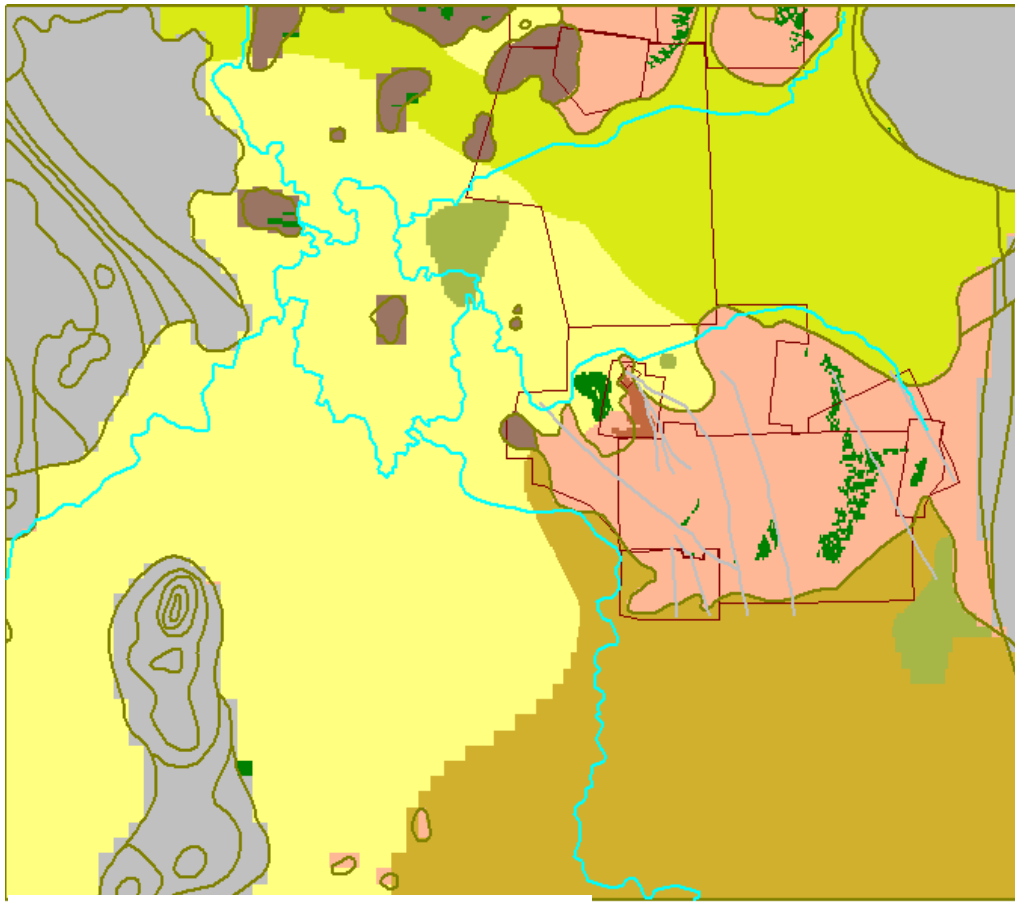
ATTACHMENT AG

Storyboard Figures for Boggabri Bores



ATTACHMENT AH

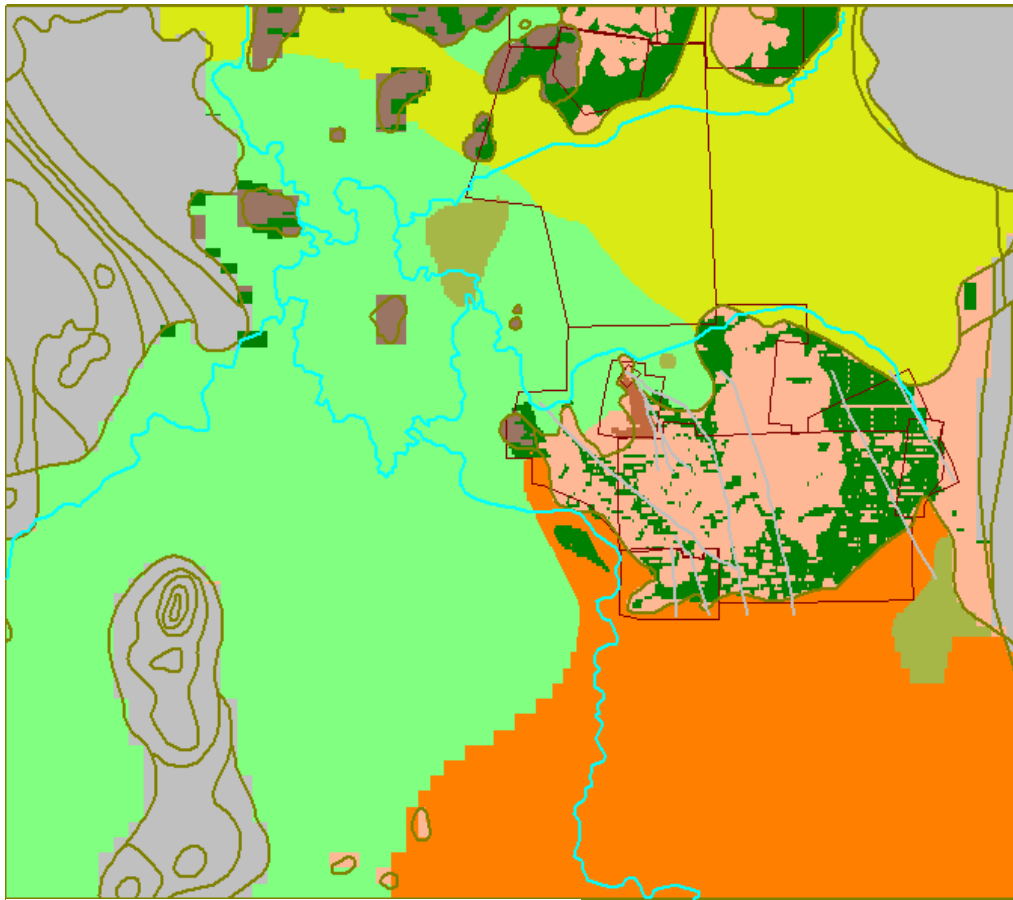
Calibrated Hydraulic and Storage Parameter Distributions



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
1	1.0e+001	5.0e-002	5.0e-002	1.0e-003
14	2.5e-003	5.0e-004	1.0e-002	1.0e-004
15	1.0e-002	1.0e-003	1.0e-002	1.0e-004
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004
21	1.0e+000	1.0e+000	1.0e-001	5.0e-003
24	3.5e-001	1.0e-002	5.0e-002	1.0e-003
25	4.0e+001	1.0e-001	5.0e-002	1.0e-003
26	5.0e+000	1.0e-001	2.0e-001	1.0e-003

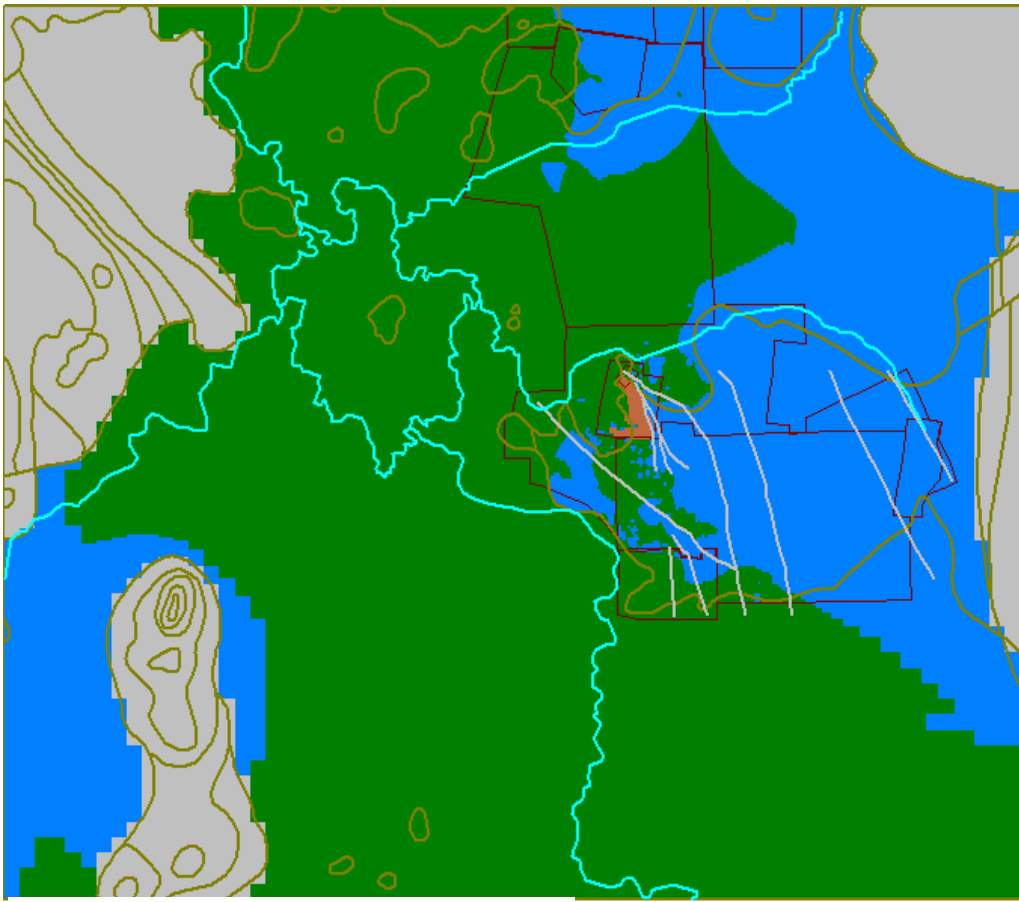
Figure AH-1. Hydraulic Property Zones for Layer 1



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
2	1.5e+001	1.0e-001	2.0e-001	5.0e-003
14	2.5e-003	5.0e-004	1.0e-002	1.0e-004
15	1.0e-002	1.0e-003	1.0e-002	1.0e-004
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004
21	1.0e+000	1.0e+000	1.0e-001	5.0e-003
24	3.5e-001	1.0e-002	2.0e-001	5.0e-003
25	4.0e+001	1.0e-001	2.0e-001	5.0e-003
27	8.0e+000	5.0e-002	2.0e-001	5.0e-003

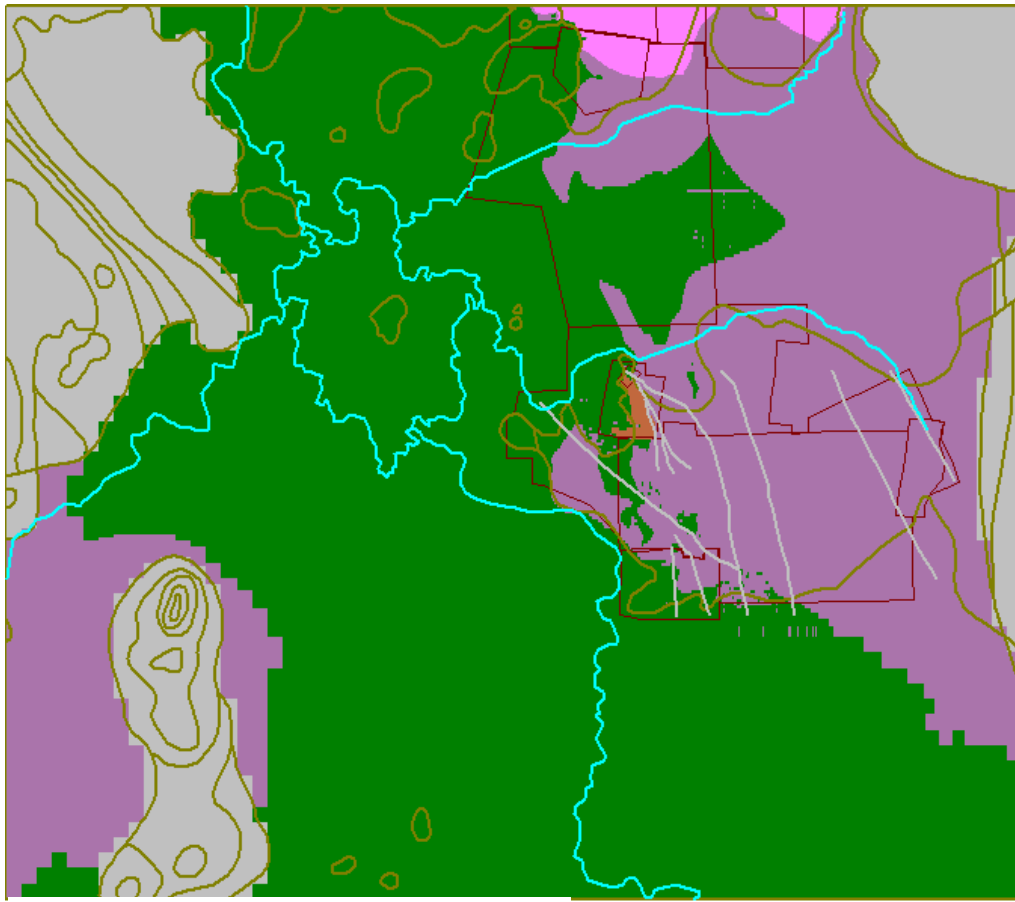
Figure AH-2. Hydraulic Property Zones for Layer 2



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
3	3.4e-004	1.2e-005	5.0e-003	5.0e-005
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004
21	1.0e+000	1.0e+000	1.0e-001	5.0e-003

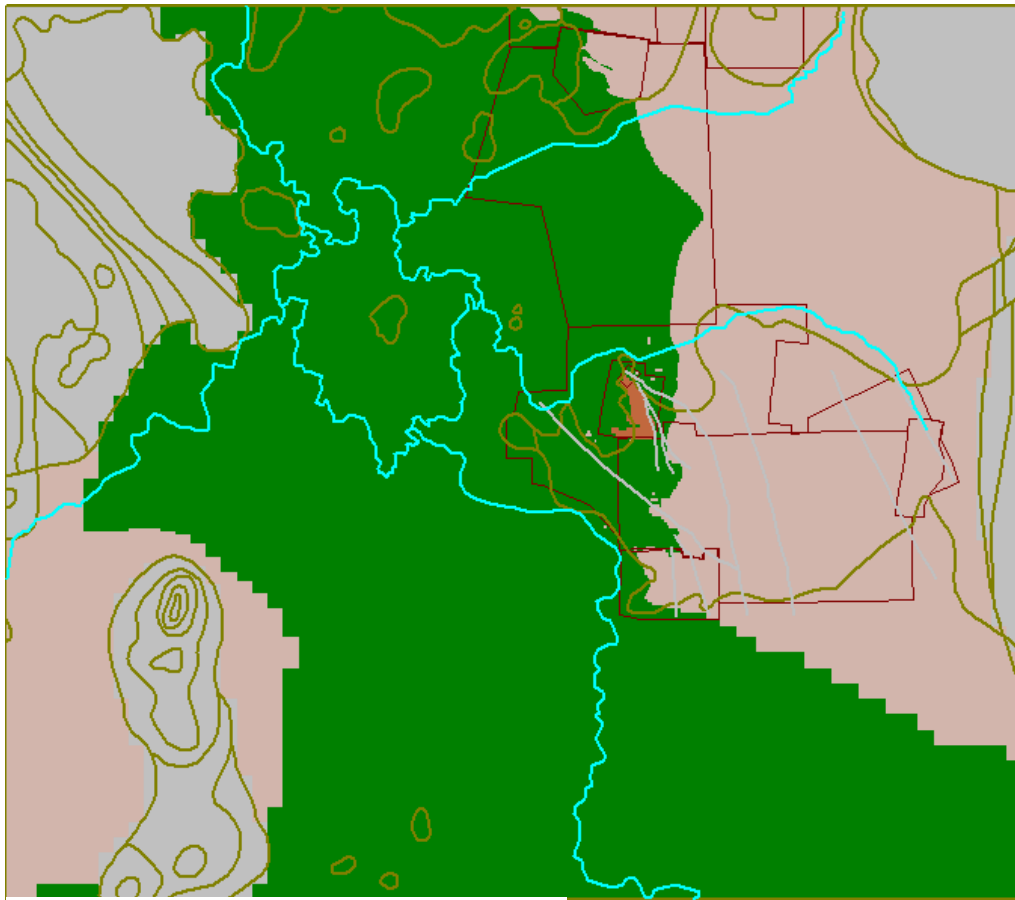
Figure AH-3. Hydraulic Property Zones for Layer 3



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
4	4.0e-001	1.0e-002	1.0e-002	1.0e-004
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004
18	2.5e-001	1.0e-002	1.0e-002	1.0e-004
21	1.0e+000	1.0e+000	1.0e-001	5.0e-003

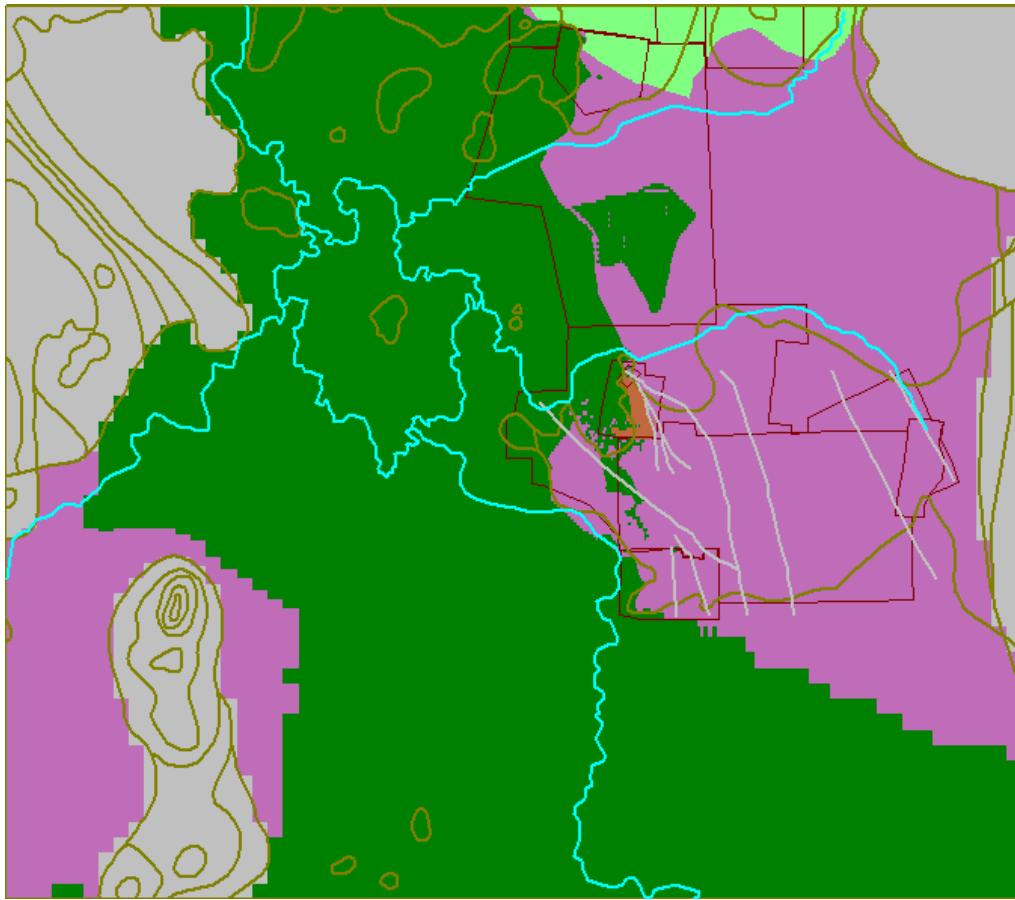
Figure AH-4. Hydraulic Property Zones for Layer 4



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
5	2.5e-004	1.3e-006	5.0e-003	5.0e-005
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004
21	1.0e+000	1.0e+000	1.0e-001	5.0e-003

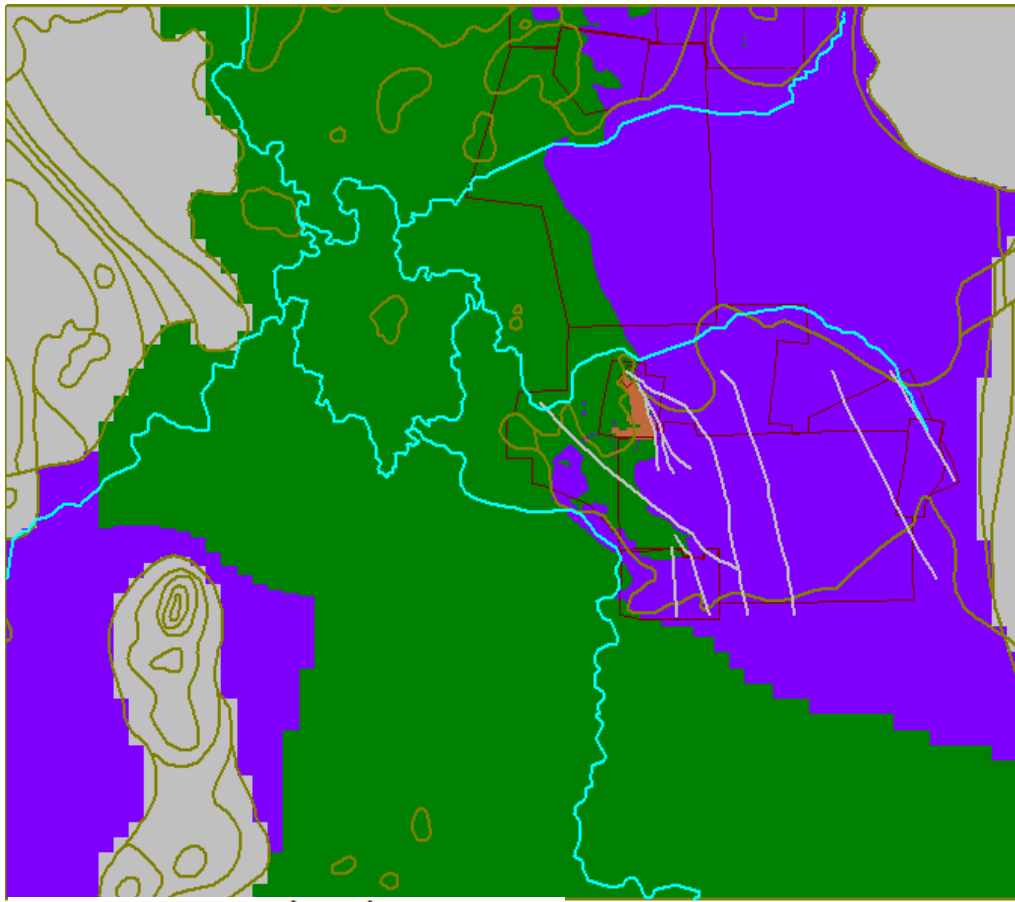
Figure AH-5. Hydraulic Property Zones for Layer 5



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
6	4.0e-001	1.0e-002	1.0e-002	1.0e-004
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004
19	2.5e-001	1.0e-002	1.0e-002	1.0e-004
21	1.0e+000	1.0e+000	1.0e-001	5.0e-003

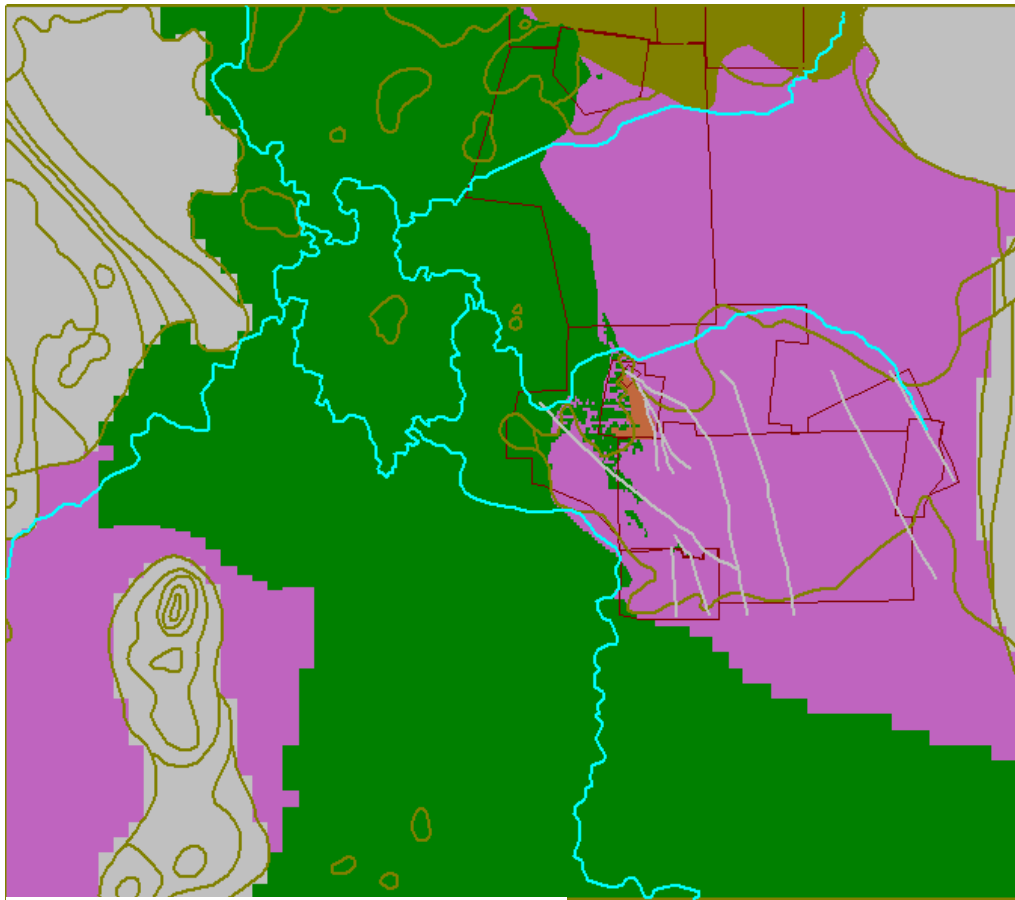
Figure AH-6. Hydraulic Property Zones for Layer 6



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
7	4.0e-005	1.1e-006	5.0e-003	5.0e-005
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004
21	1.0e+000	1.0e+000	1.0e-001	5.0e-003

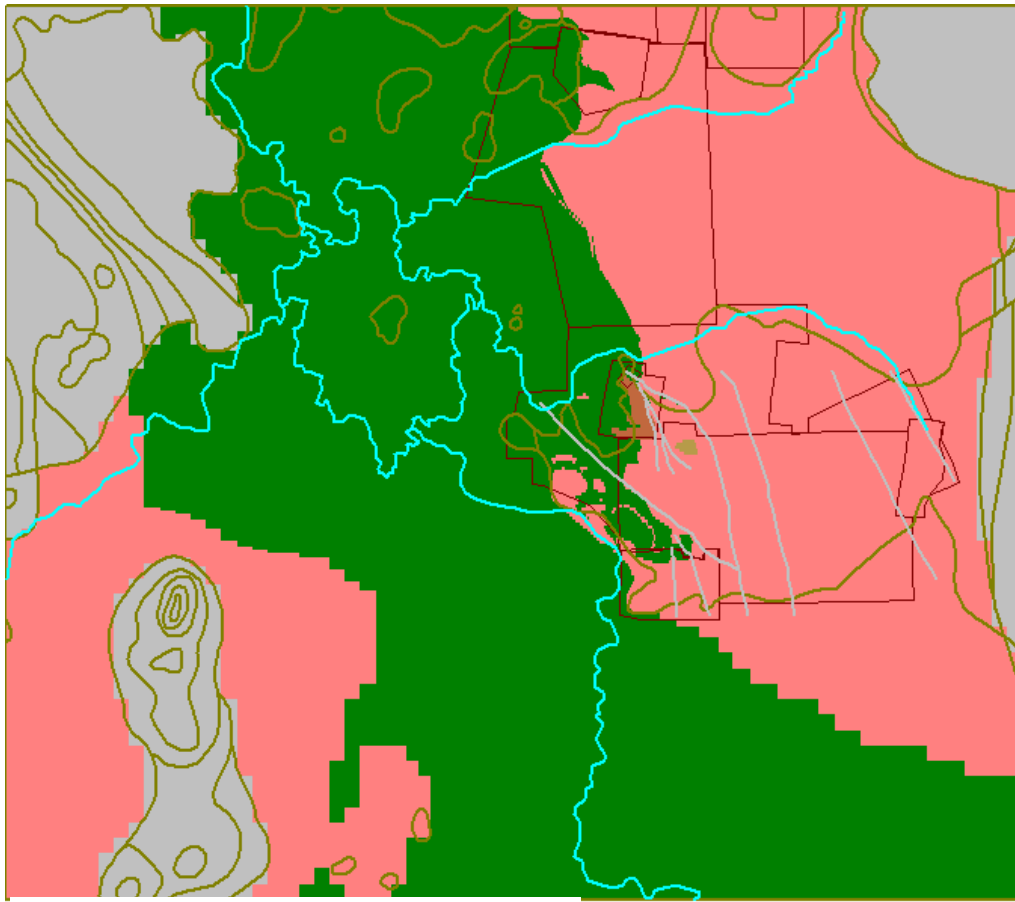
Figure AH-7. Hydraulic Property Zones for Layer 7



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
8	3.0e-001	1.0e-002	1.0e-002	1.0e-004
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004
20	2.5e-001	1.0e-002	1.0e-002	1.0e-004
21	1.0e+000	1.0e+000	1.0e-001	5.0e-003

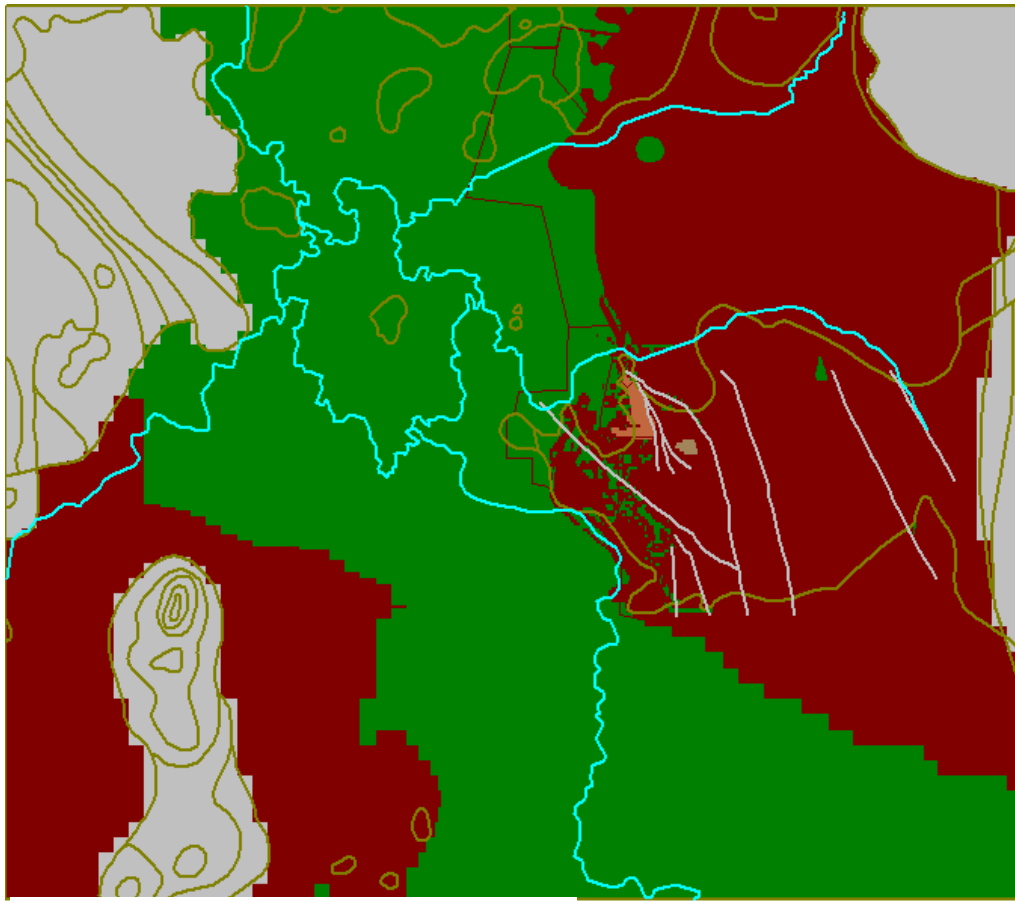
Figure AH-8. Hydraulic Property Zones for Layer 8



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
9	3.0e-005	8.3e-007	5.0e-003	5.0e-005
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004
21	1.0e+000	1.0e+000	1.0e-001	5.0e-003
23	6.1e-005	8.3e-006	4.0e-002	1.0e-004

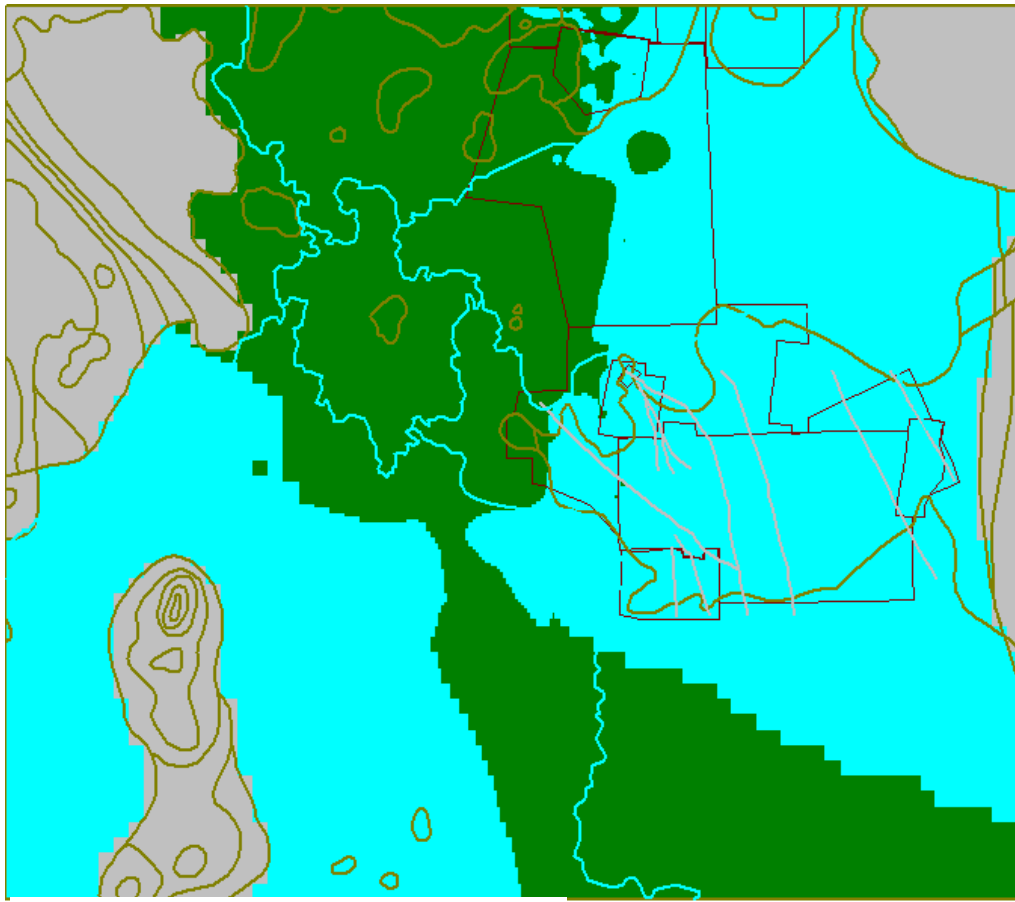
Figure AH-9. Hydraulic Property Zones for Layer 9



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
10	5.0e-002	1.0e-002	1.0e-002	1.0e-004
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004
21	1.0e+000	1.0e+000	1.0e-001	5.0e-003
22	1.0e+001	1.0e+001	2.5e-001	5.0e-004

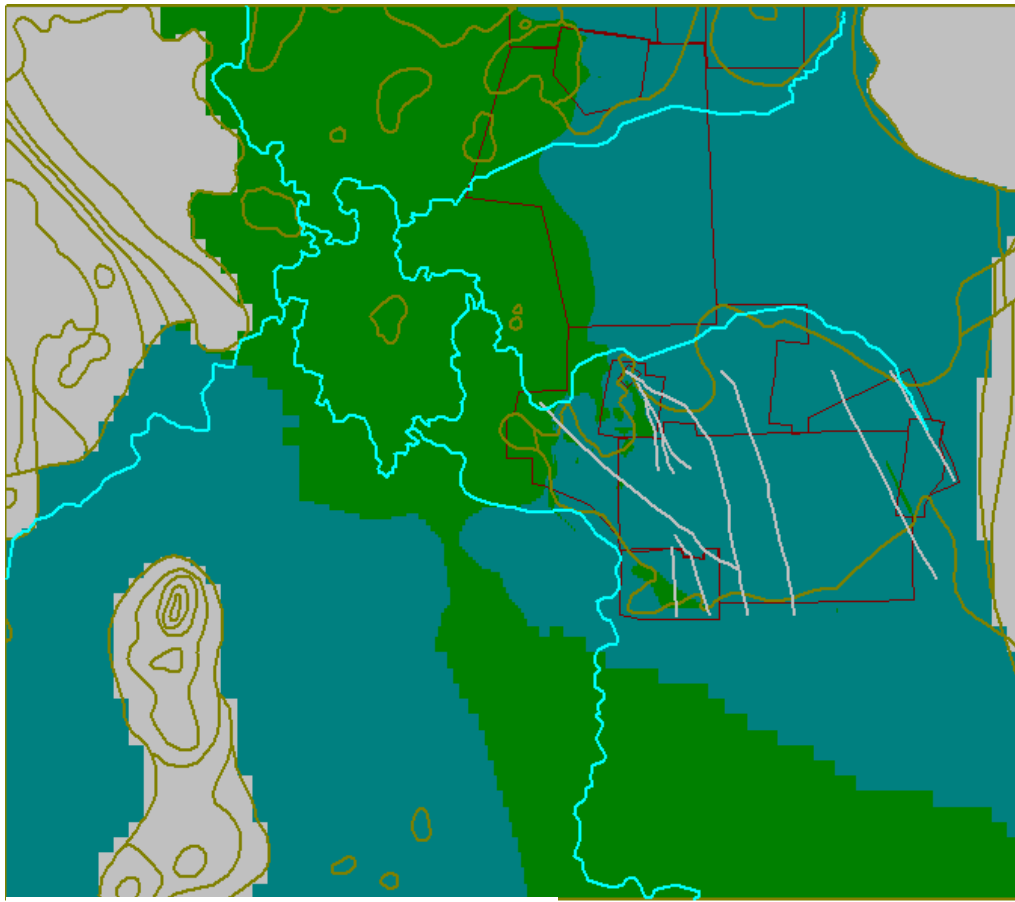
Figure AH-10. Hydraulic Property Zones for Layer 10



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
11	3.0e-005	3.6e-006	5.0e-003	5.0e-005
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004

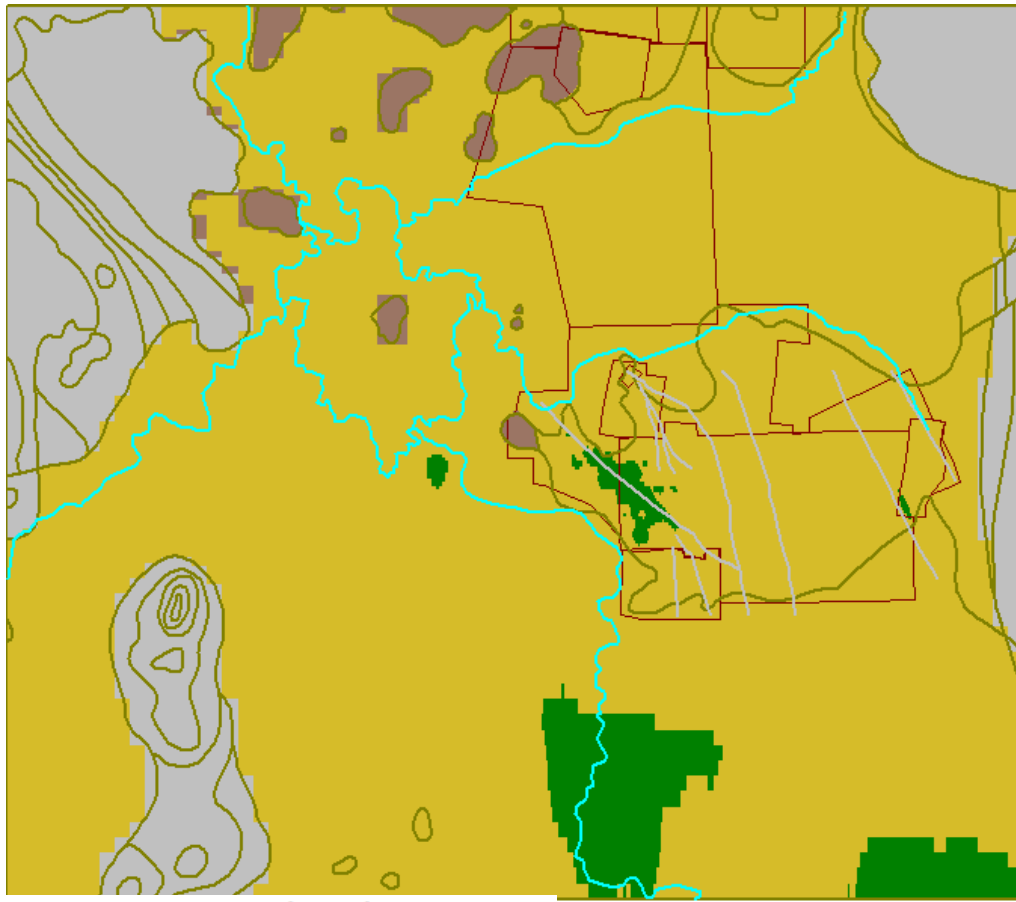
Figure AH-11. Hydraulic Property Zones for Layer 11



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
12	5.0e-002	1.0e-002	1.0e-002	1.0e-004
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004

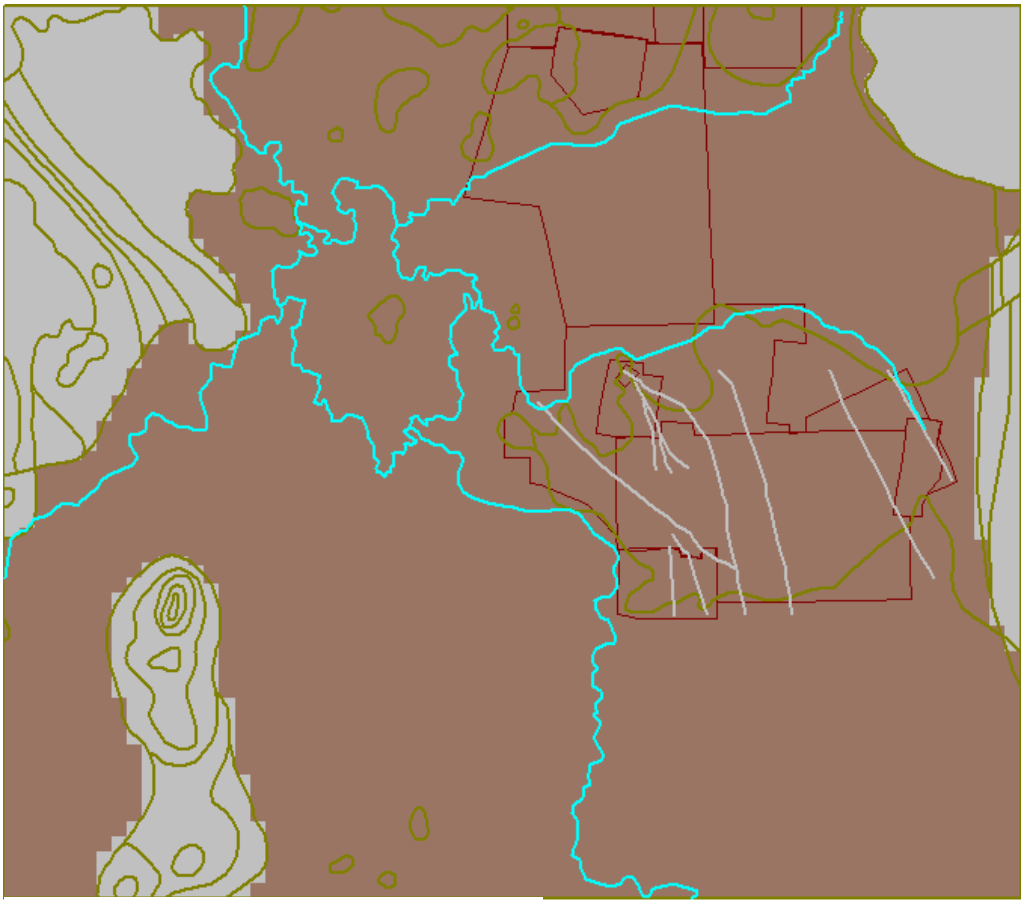
Figure AH-12. Hydraulic Property Zones for Layer 12



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
13	3.0e-005	2.0e-006	5.0e-003	5.0e-005
14	2.5e-003	5.0e-004	1.0e-002	1.0e-004
17	5.0e-003	5.0e-004	1.0e-002	1.0e-004

Figure AH-13. Hydraulic Property Zones for Layer 13



Legend

Zone	Kh (m/d)	Kv (m/d)	Sy	S
14	2.5e-003	5.0e-004	1.0e-002	1.0e-004

Figure AH-14. Hydraulic Property Zone for Layer 14

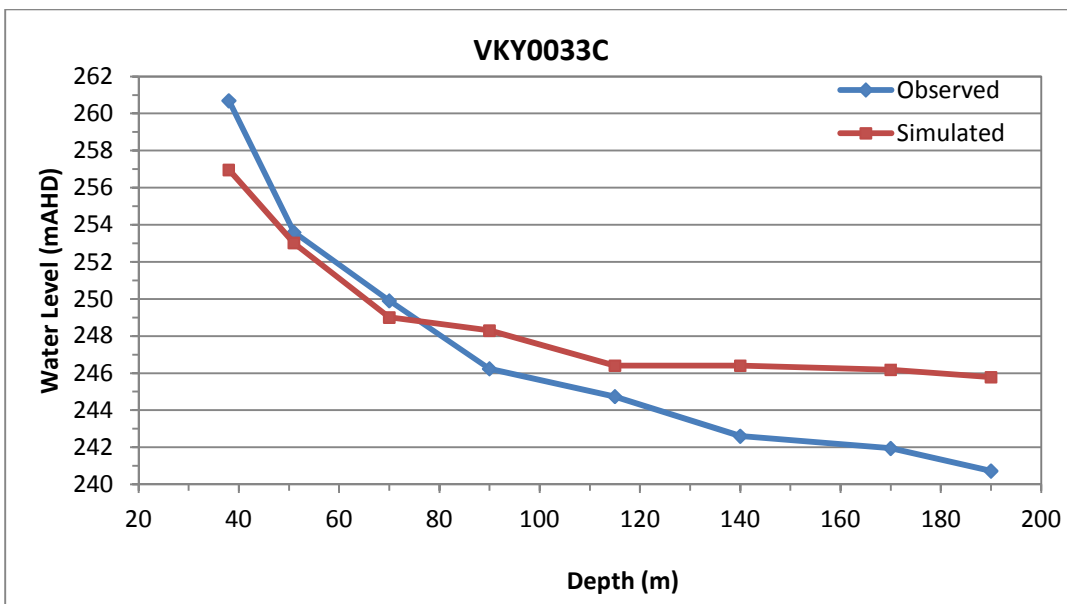
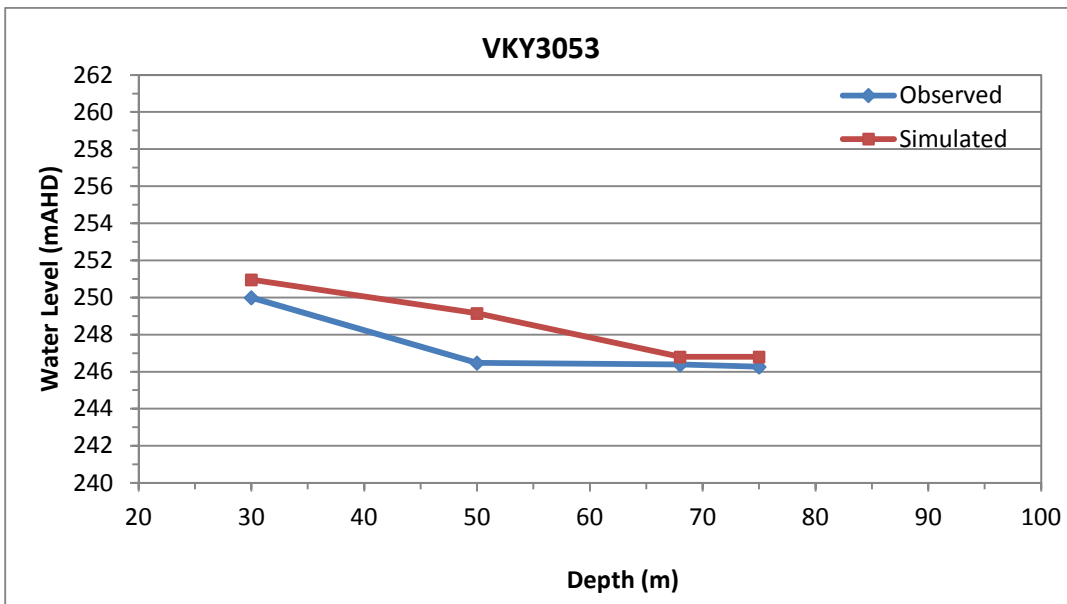
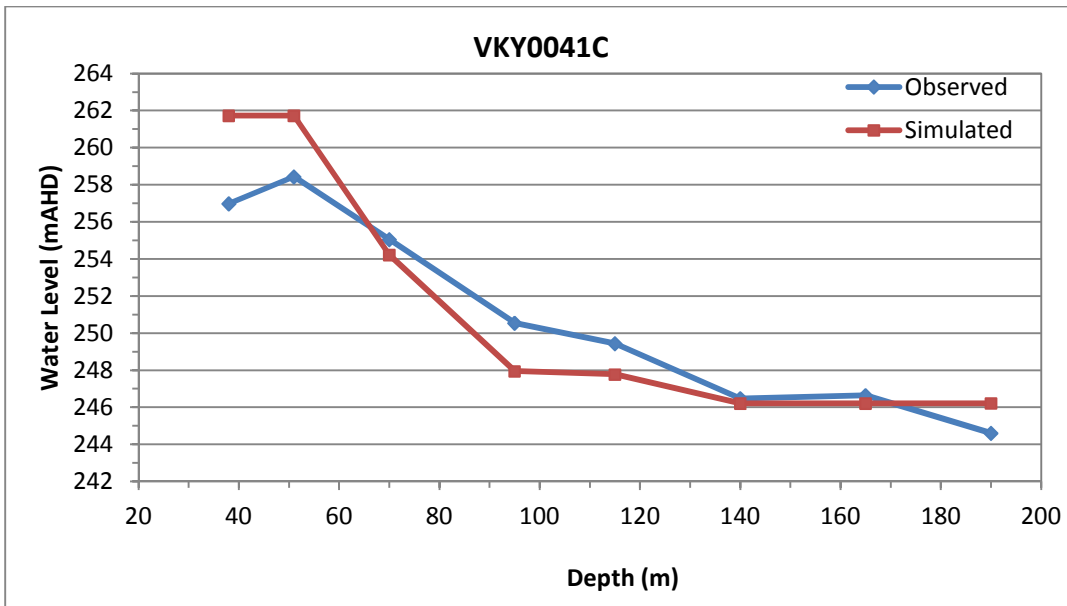
ATTACHMENT AI

Calibration Hydrographs

Vickery Coal Mine Monitoring Network

Vibrating Wire Piezometers

VKY0041C, VKY3053 and VKY0033C



Vickery Coal Mine Monitoring Network

Standpipes

TR7, TR18, VKY0034, VKY0035, VKY0036, VKY0042 and VKY0043

Stand Pipe	Formation	Measured Water Level (mAHD)	Simulated Water Level (mAHD)	Residual
TR7	Alluvium	245.5	246.1	-0.6
TR18	Alluvium	246.0	249.4	-3.4
VKY0034	Shannon Harbour Seam	244.8	246.5	-1.7
VKY0035	Kurrumbede - Shannon Harbour interburden	243.7	246.3	-2.6
VKY0036	Shannon Harbour - Stratford Interburden	250.8	246.2	4.5
VKY0042	Stratford Seam	249.5	246.7	2.8
VKY0043	Bluevale - Cranleigh	247.7	245.7	2.0

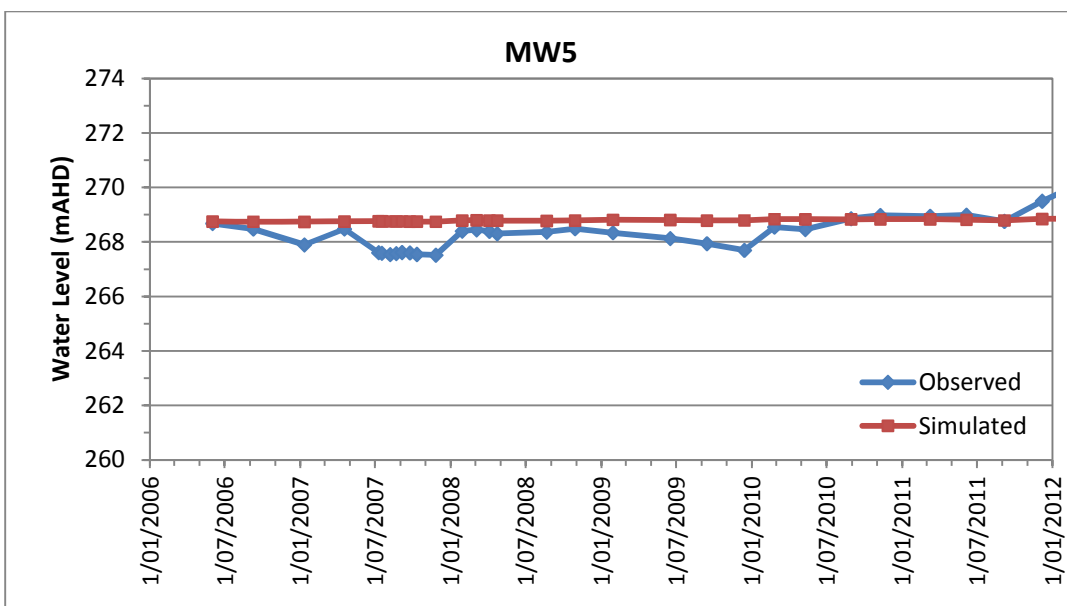
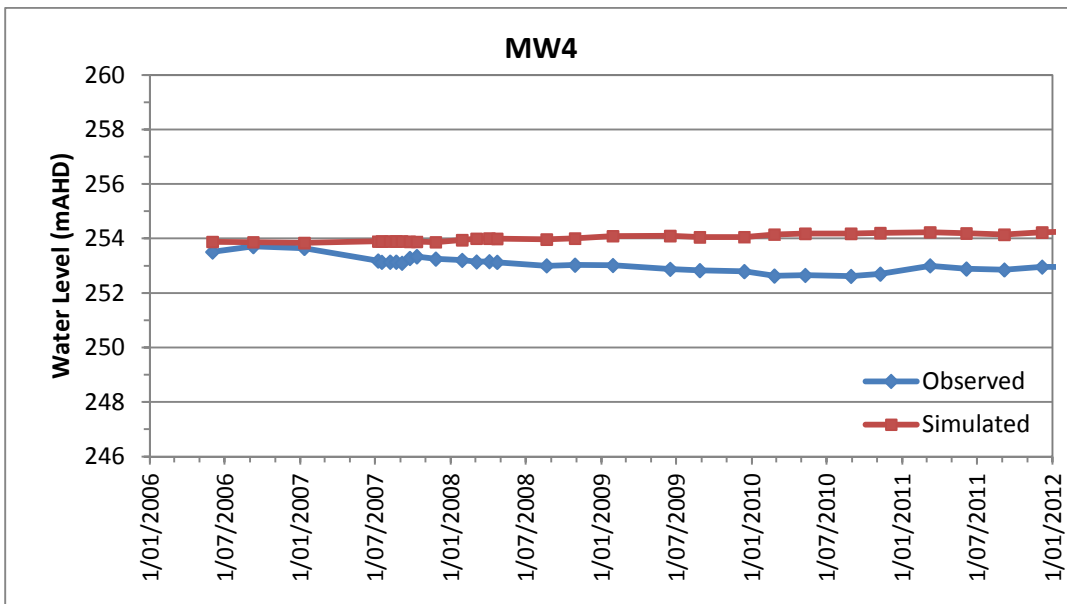
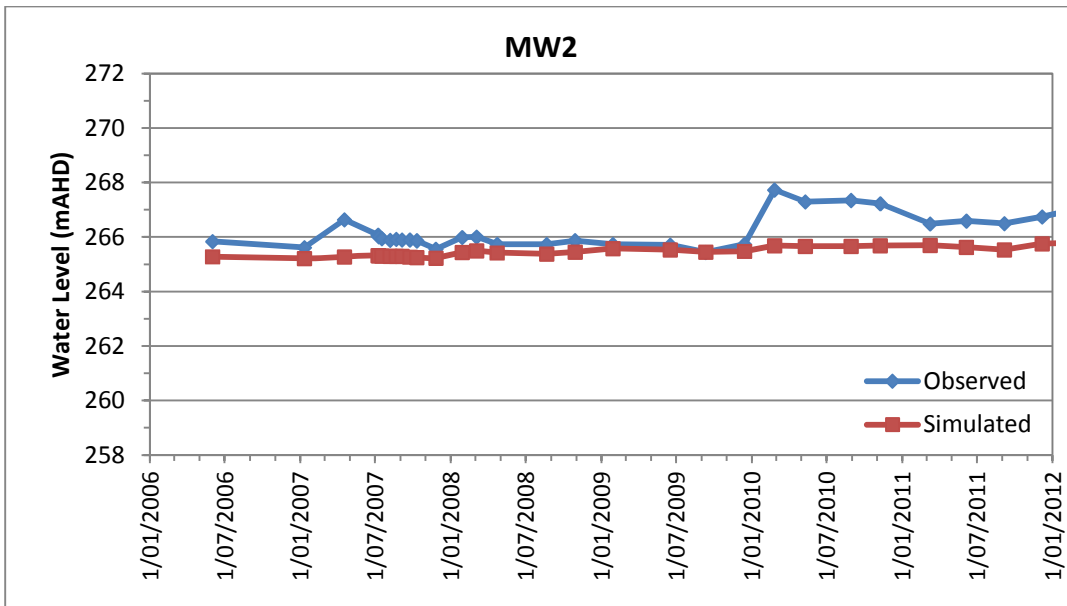
Tarrawonga Coal Mine Monitoring Network – Alluvium

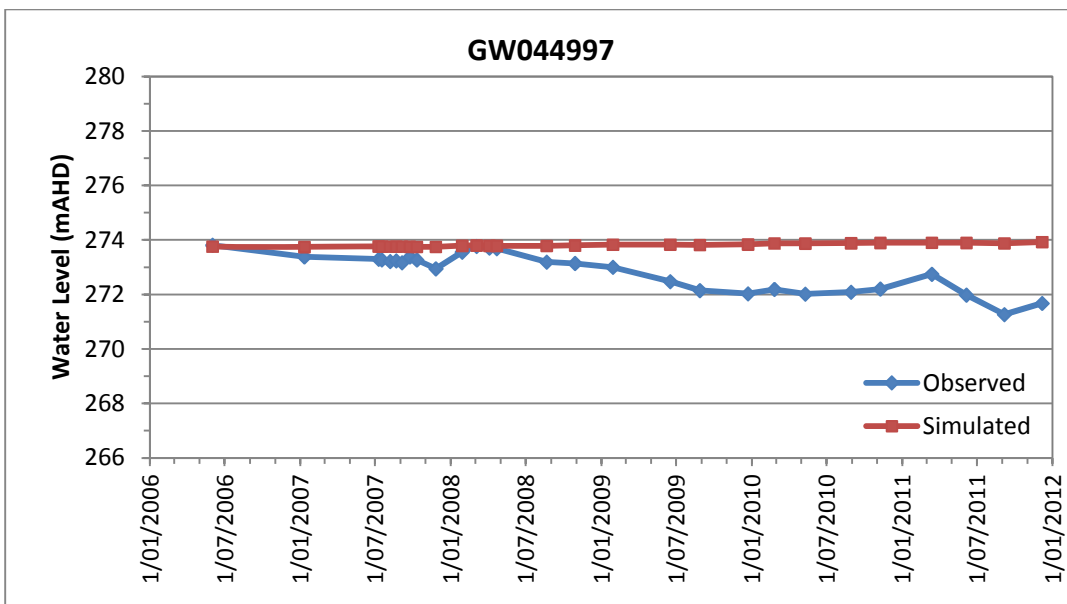
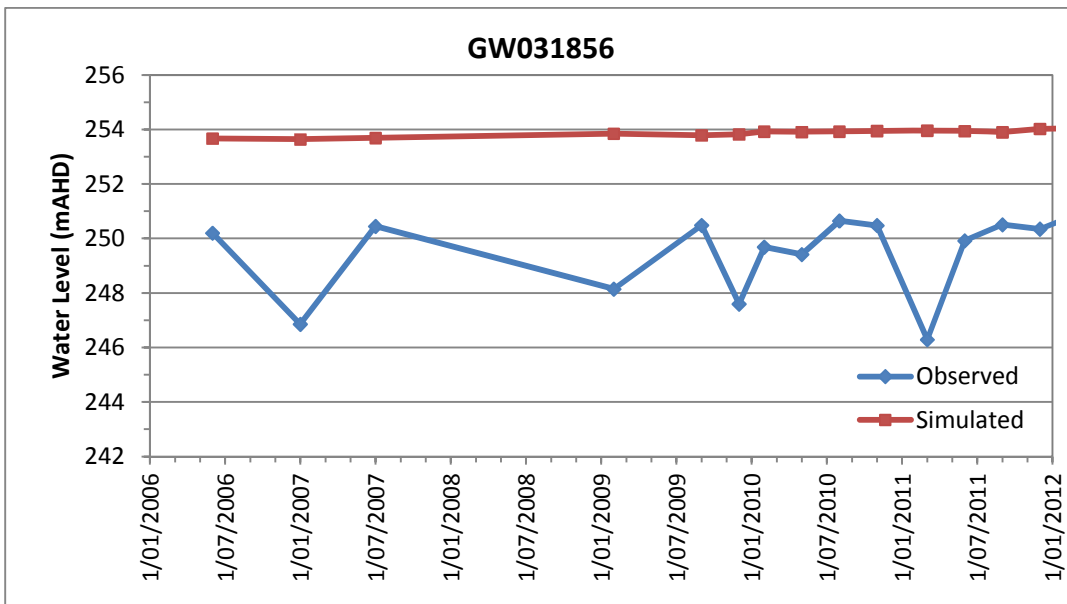
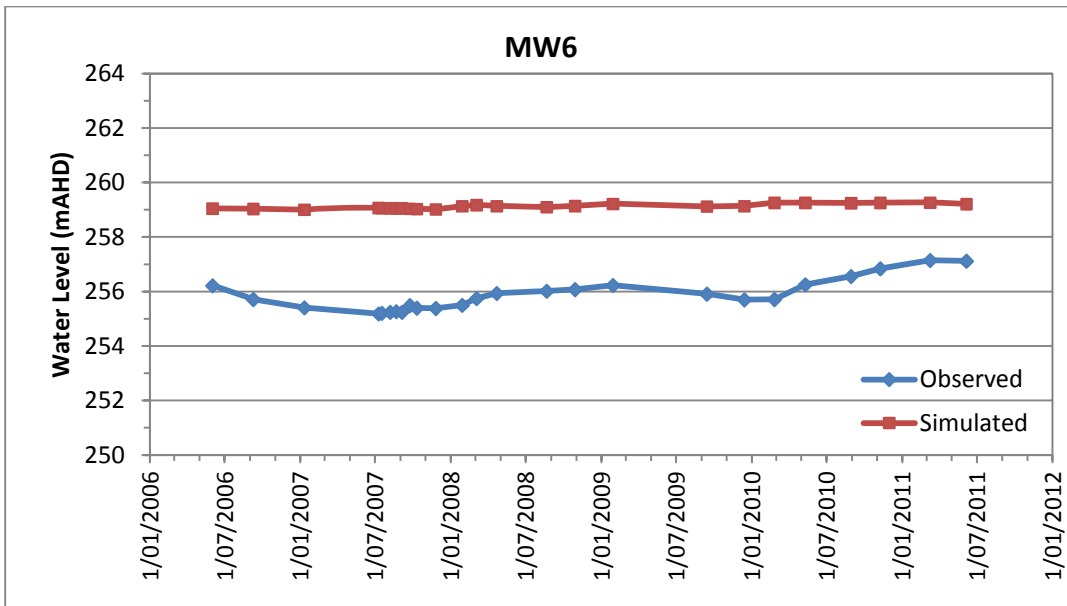
MW2, MW4, MW5, MW6

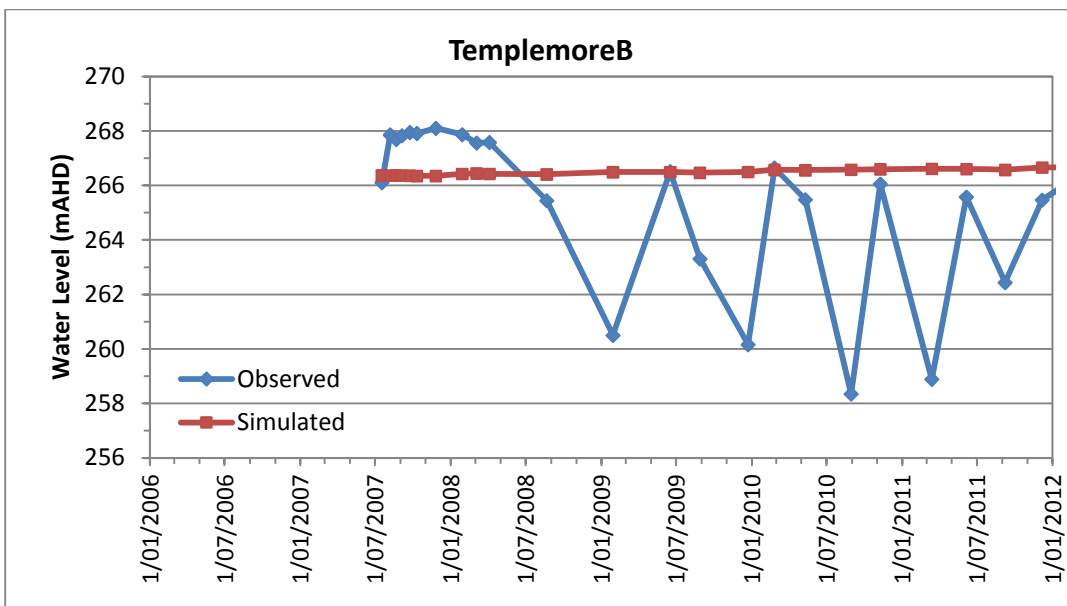
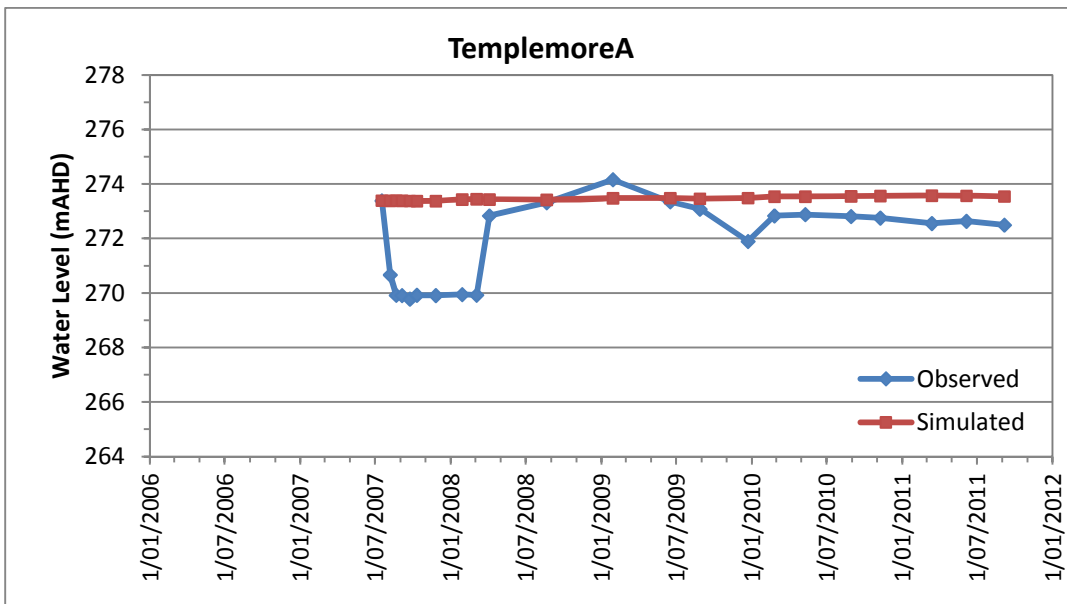
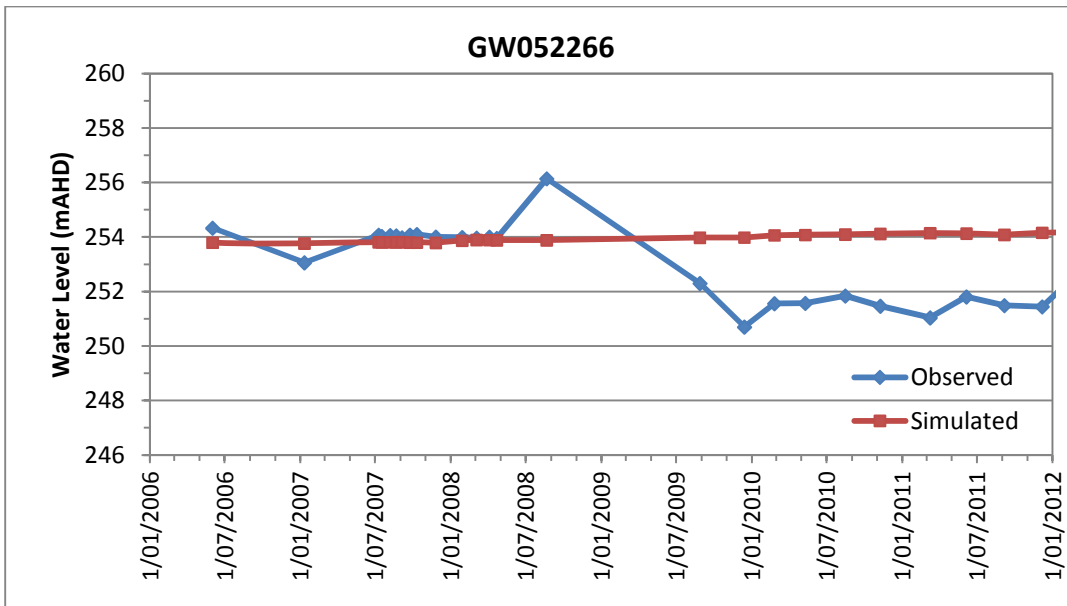
GW031856, GW044997, GW052266

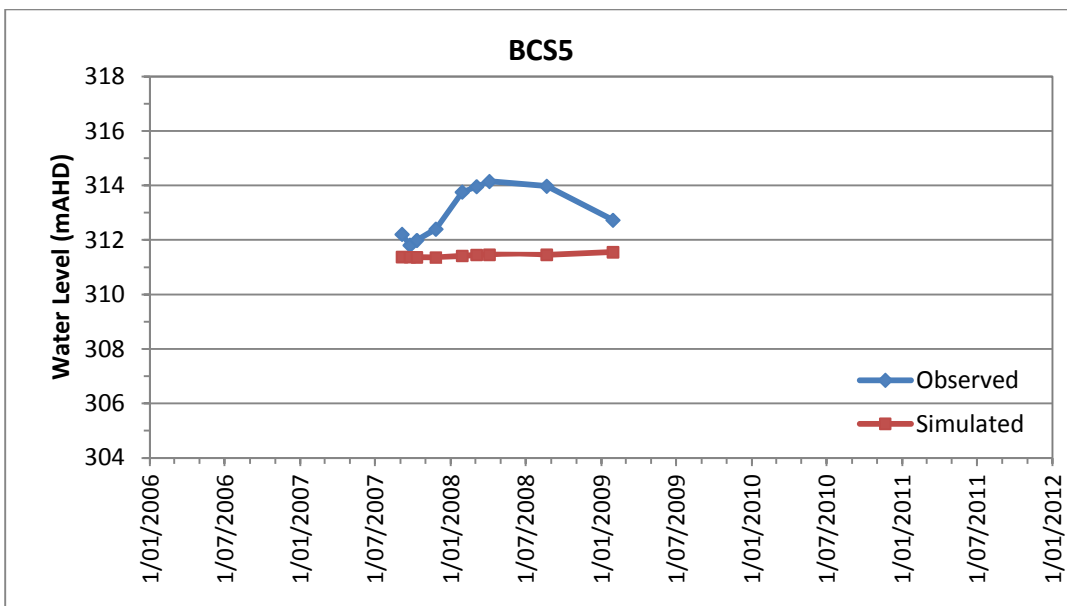
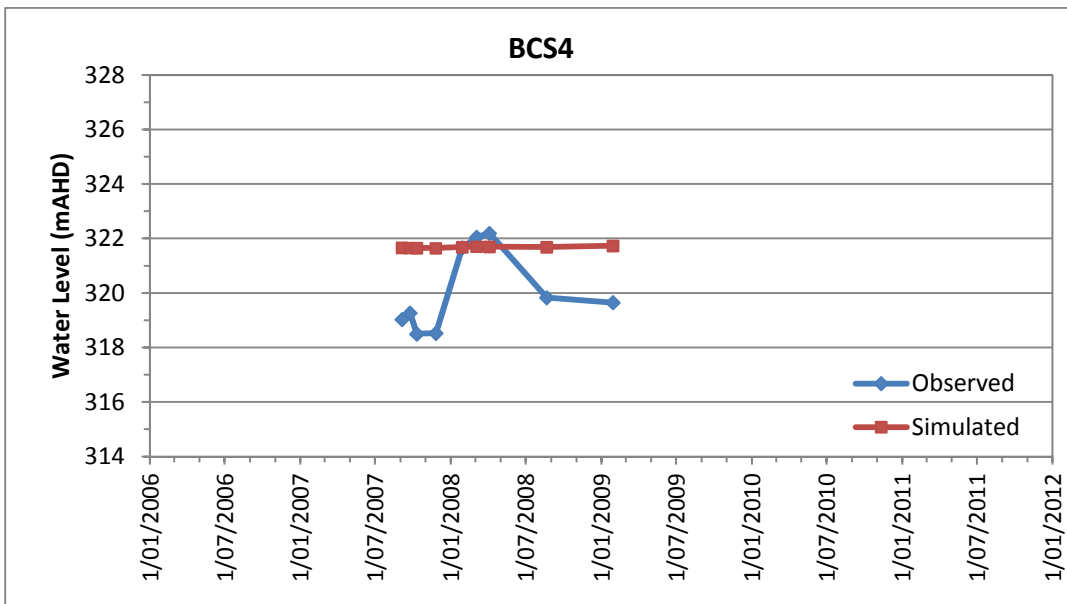
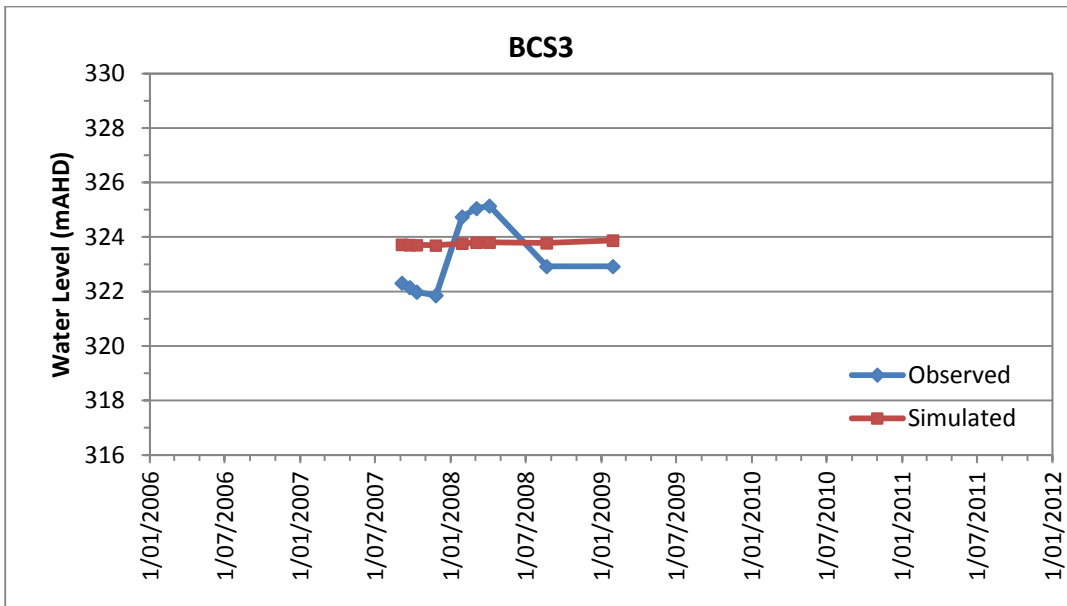
Templemore A and B

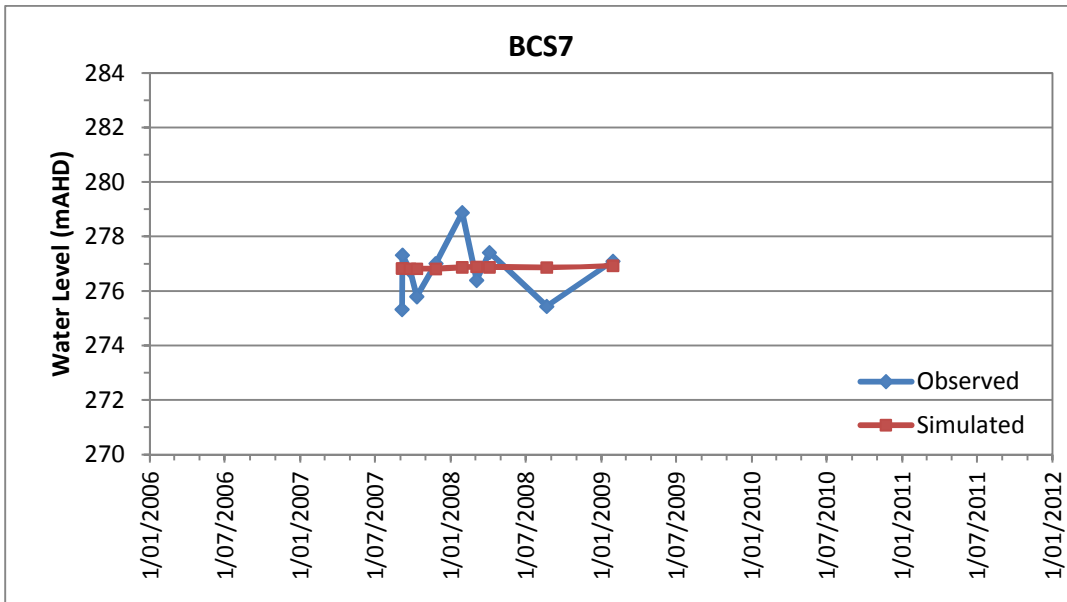
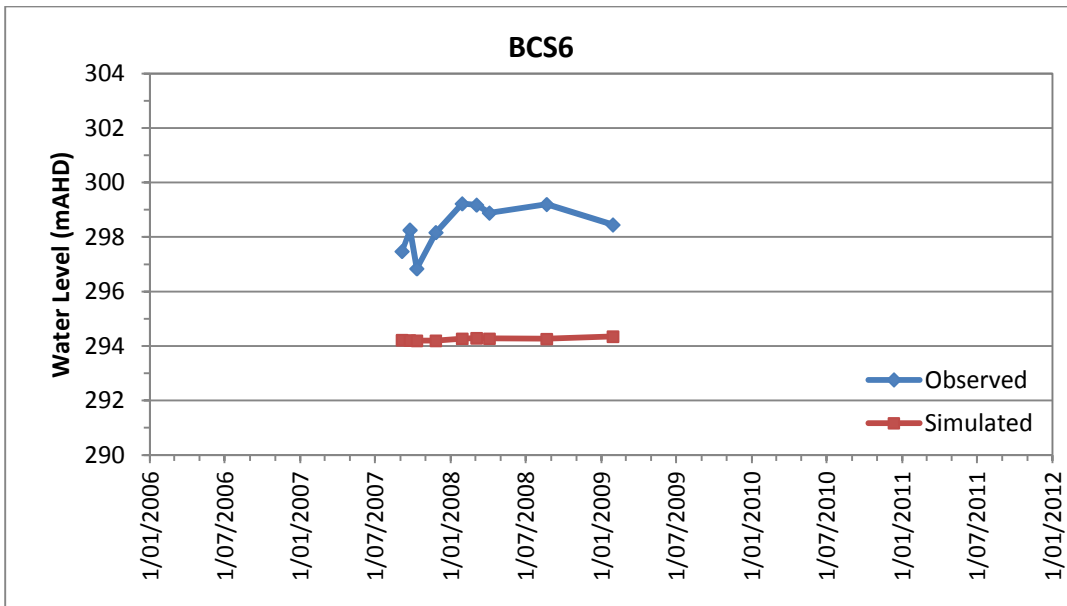
BCS3 to BCS7







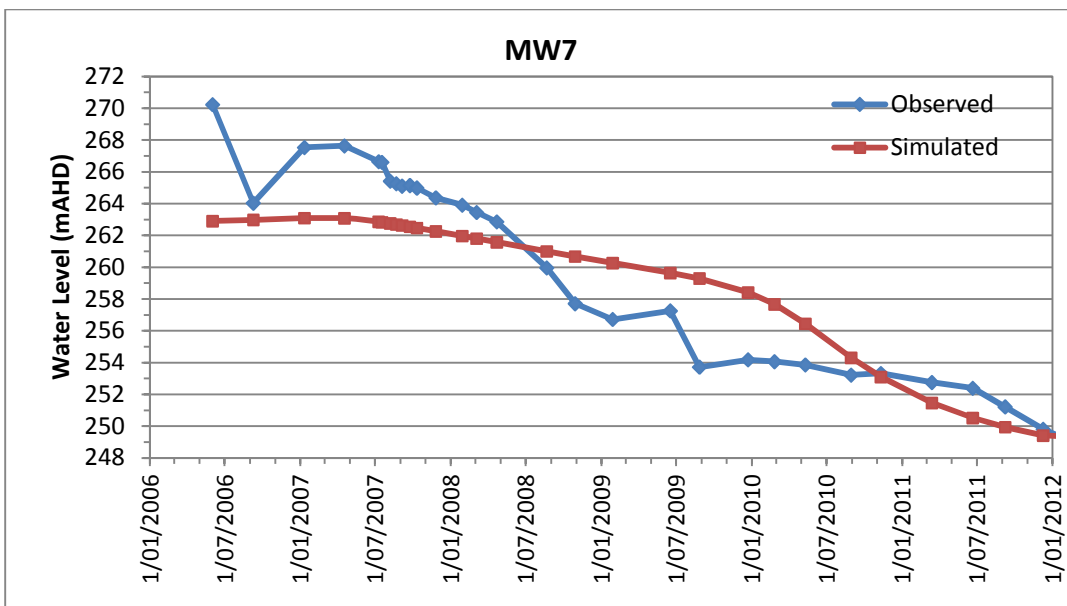
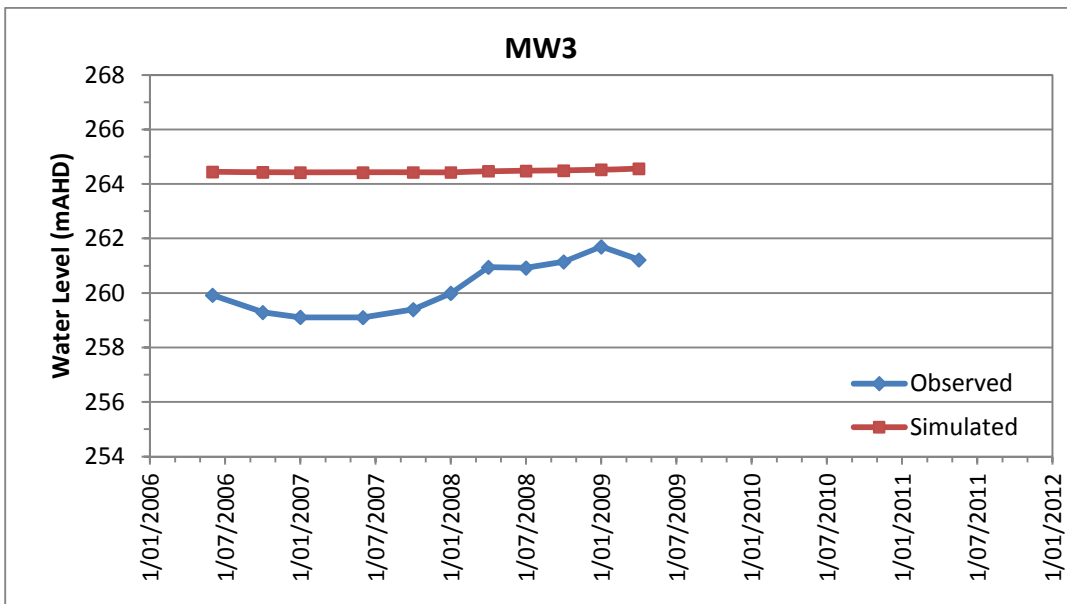
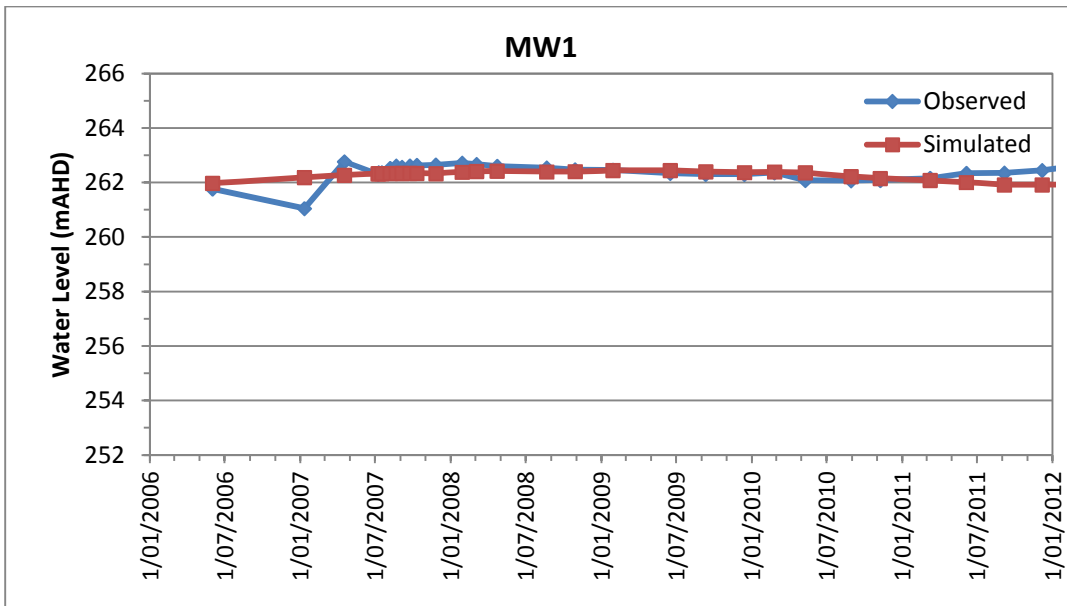


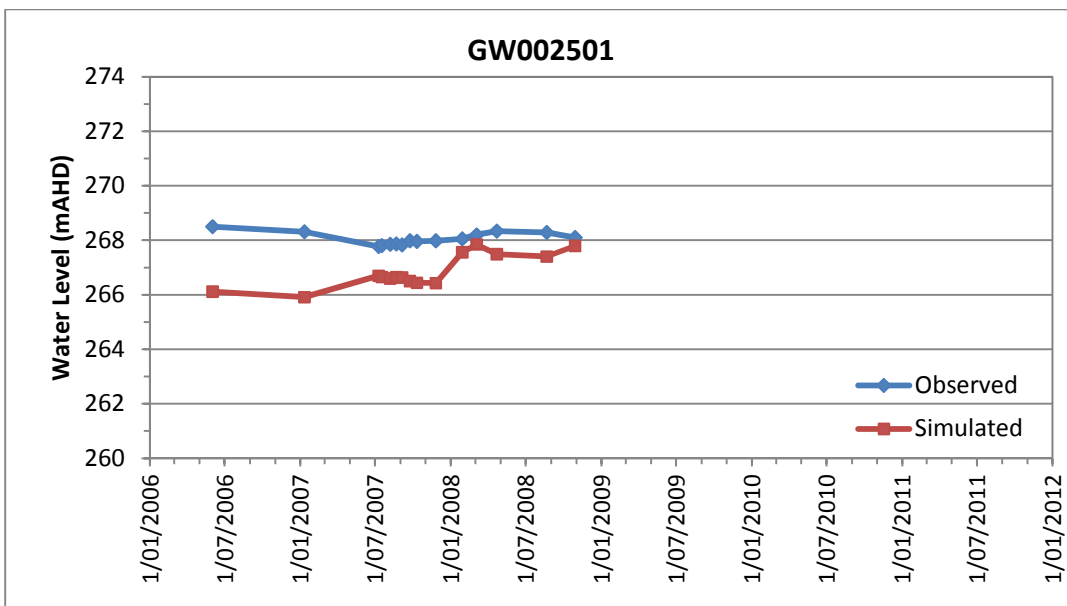
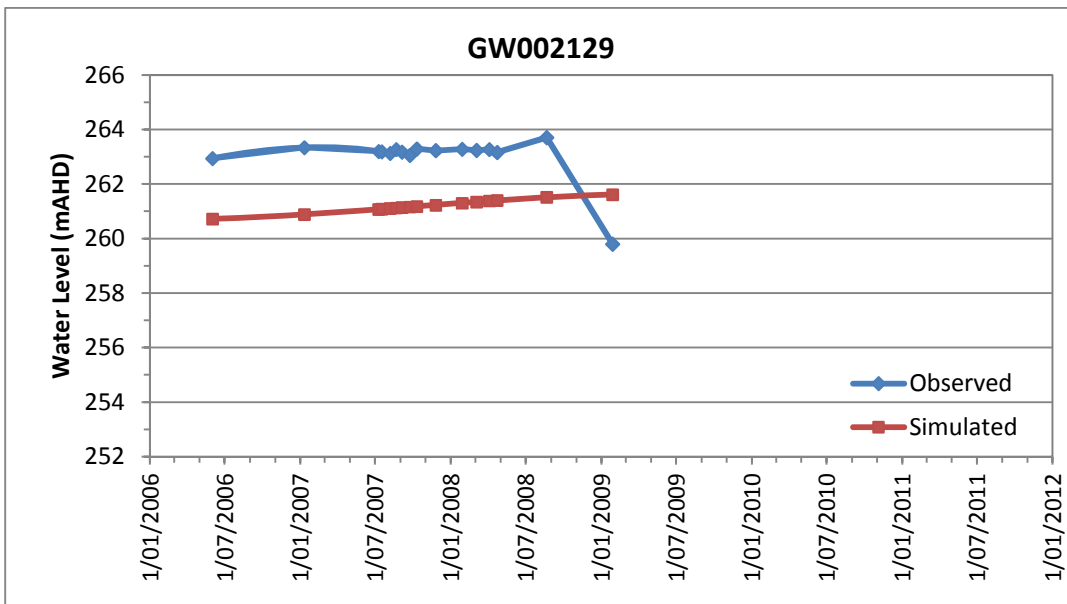
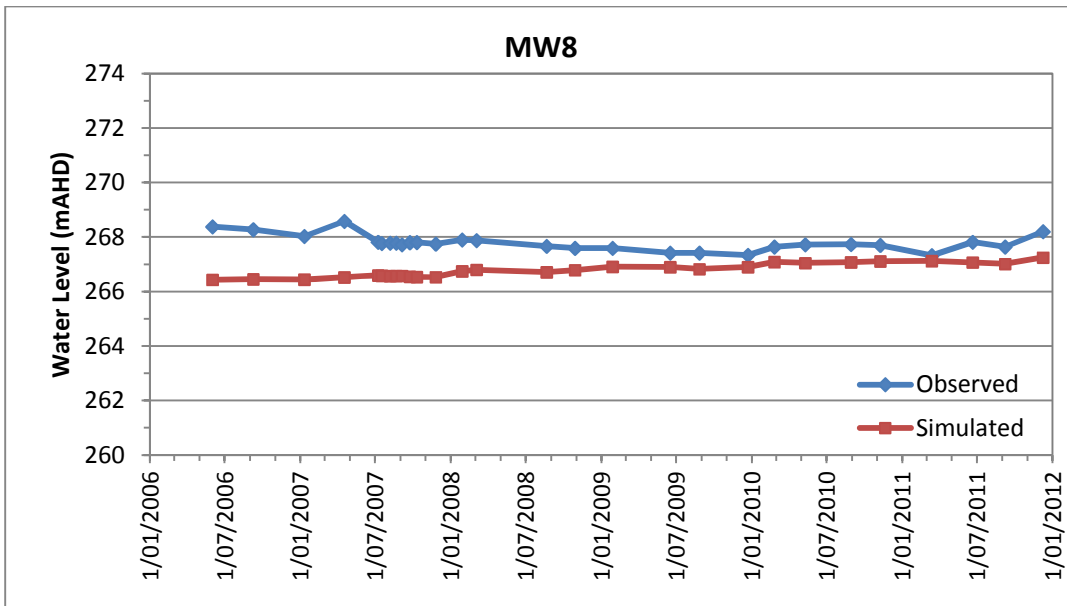


Tarrawonga Mine Coal, Coal Measures and Volcanics

MW1, MW3, MW7, MW8

GW002129, GW002501

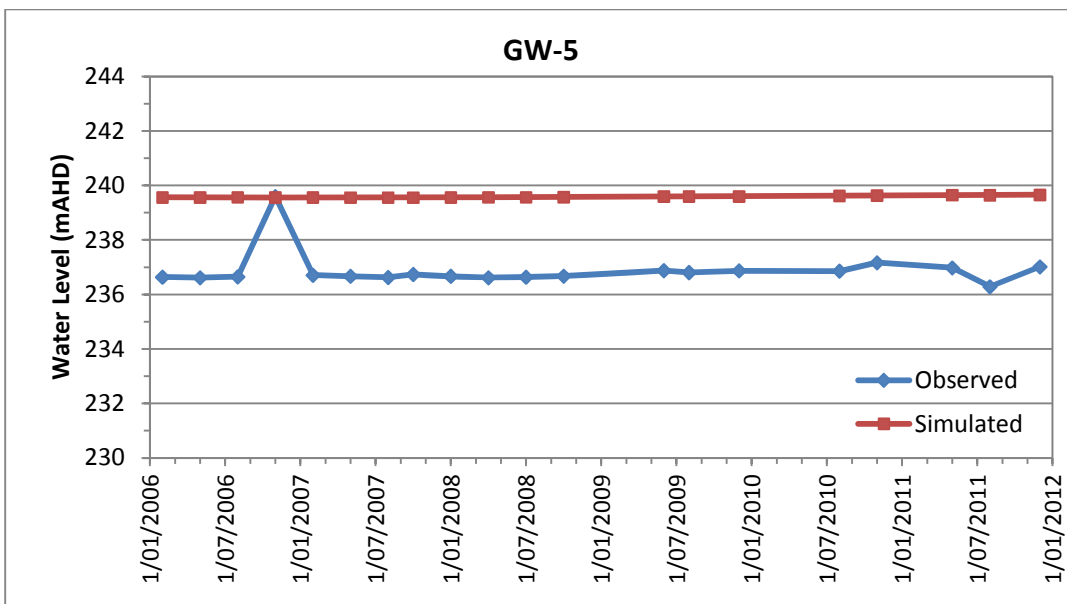
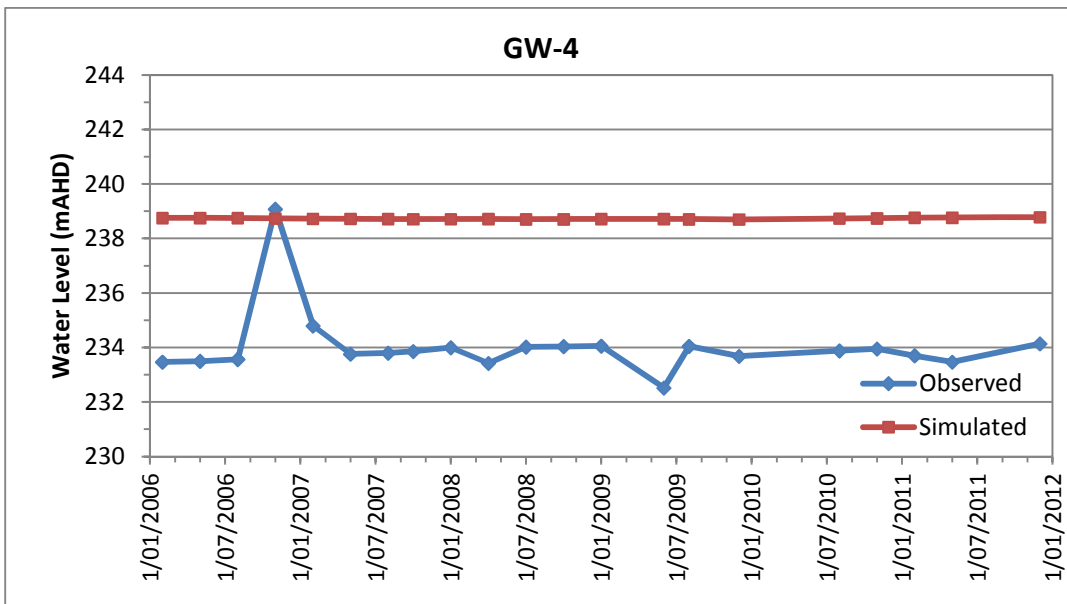
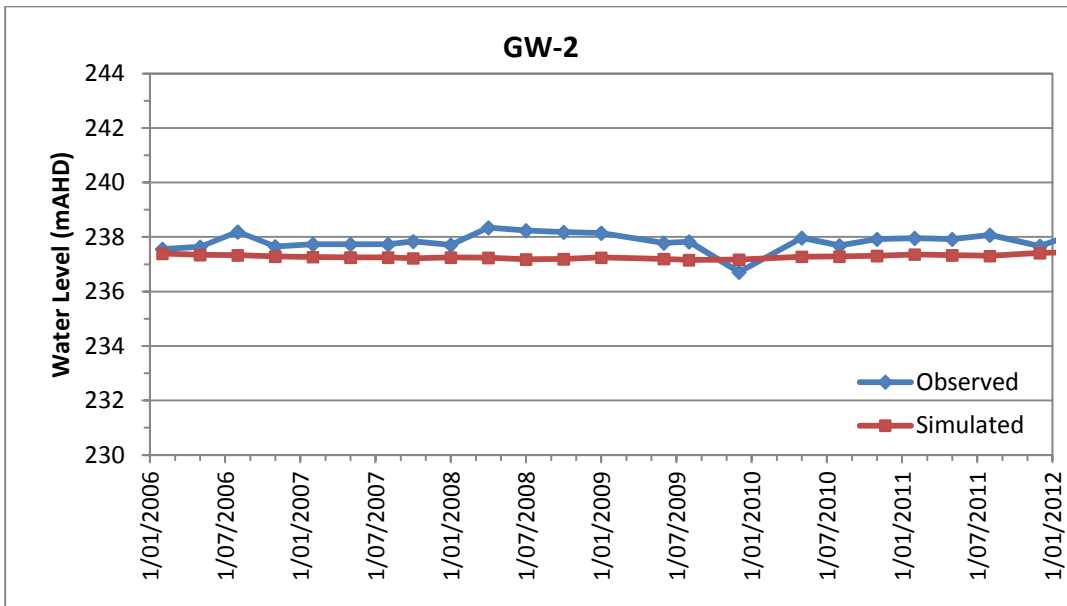


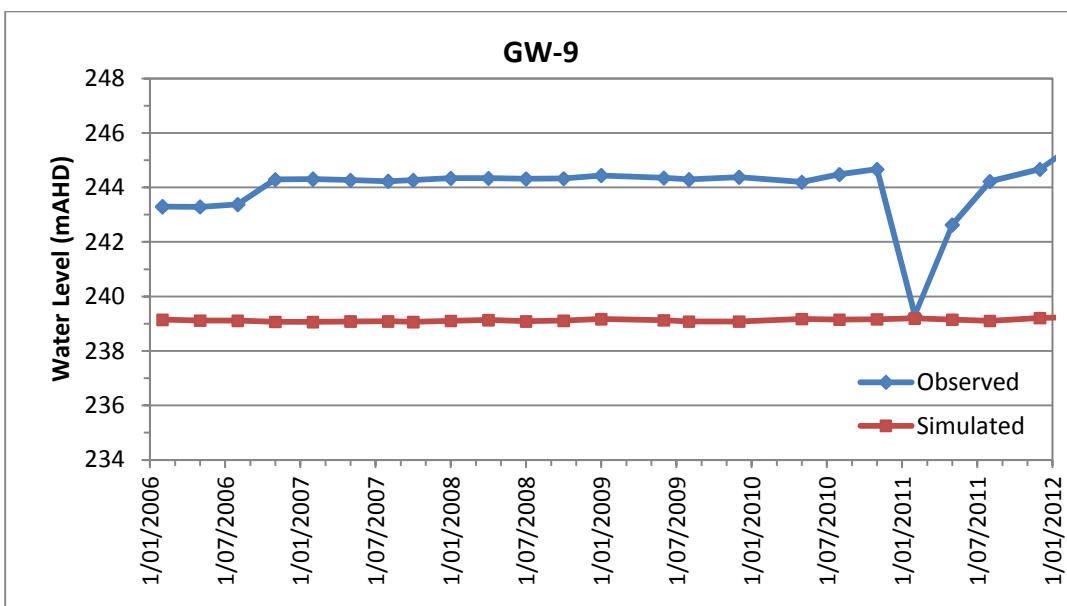
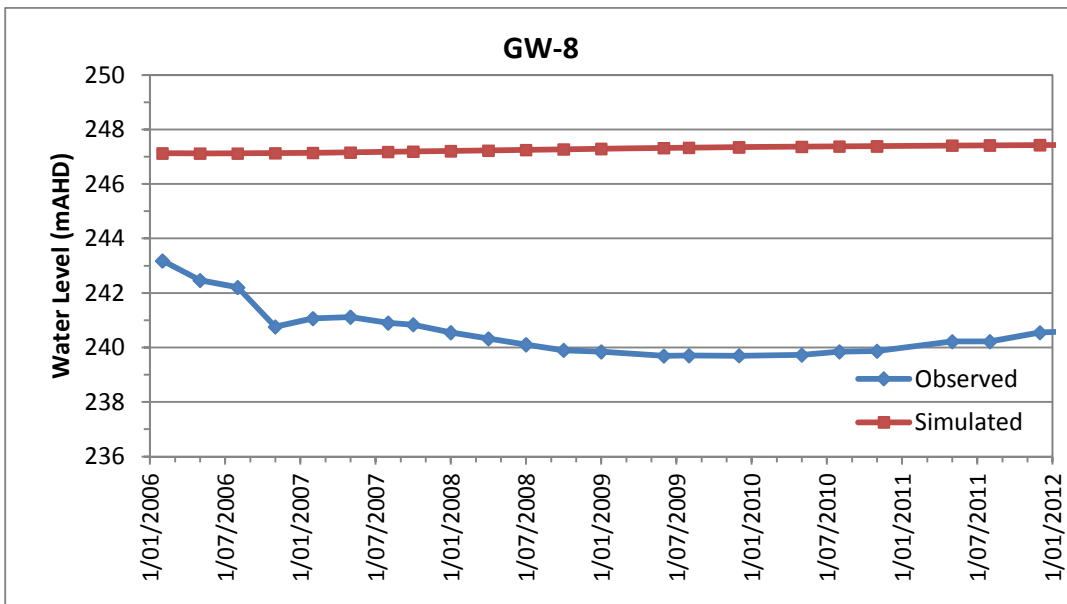
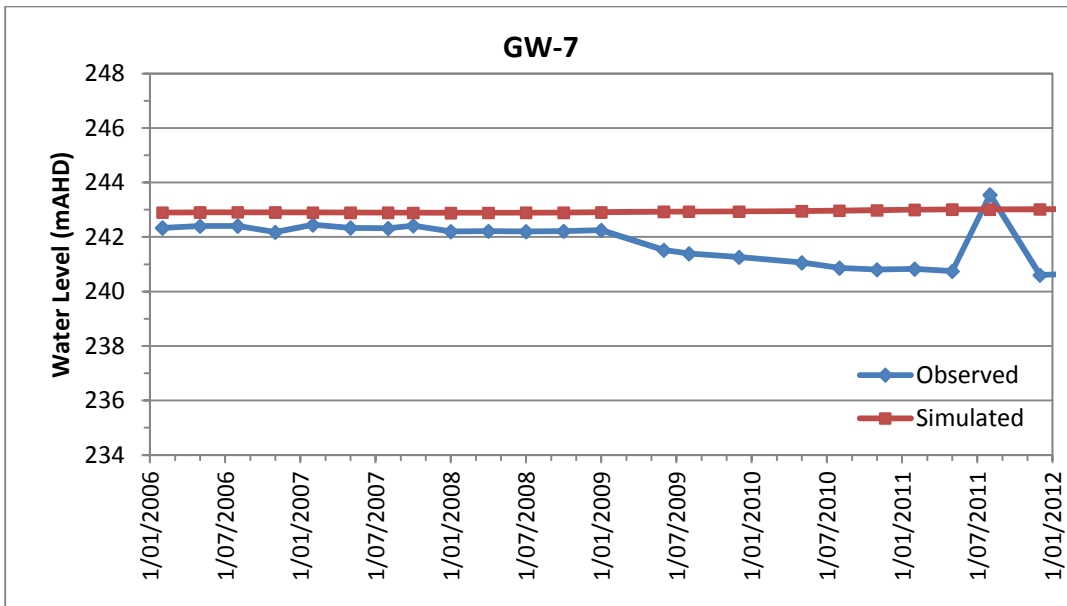


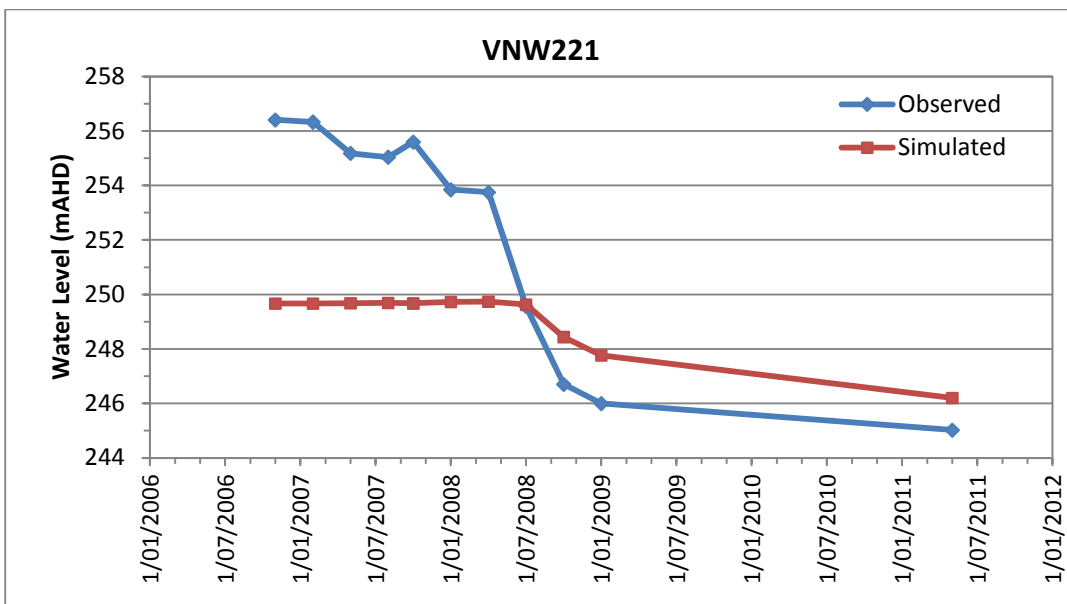
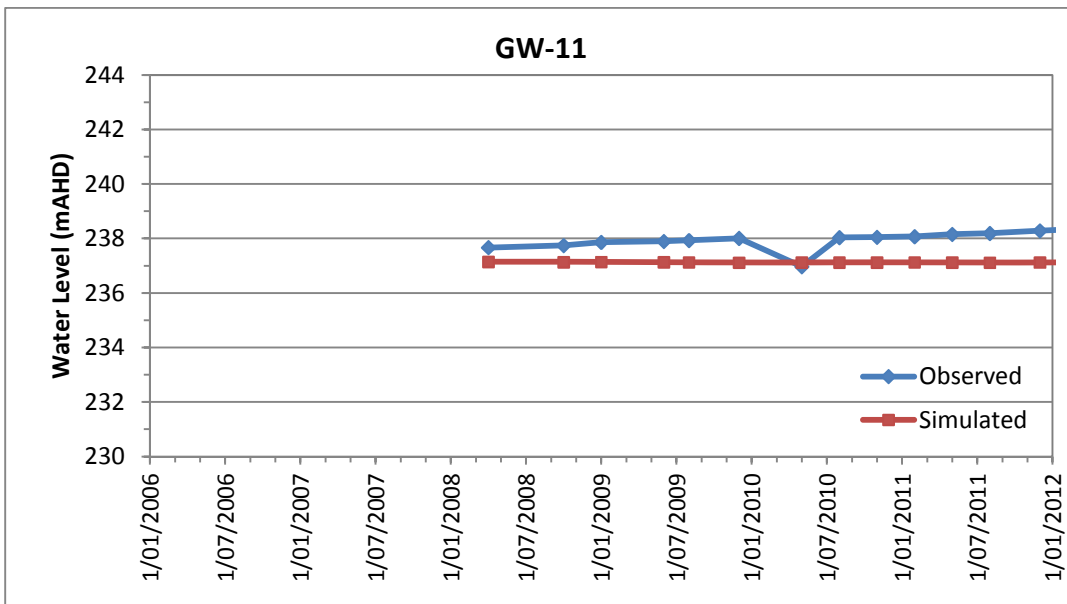
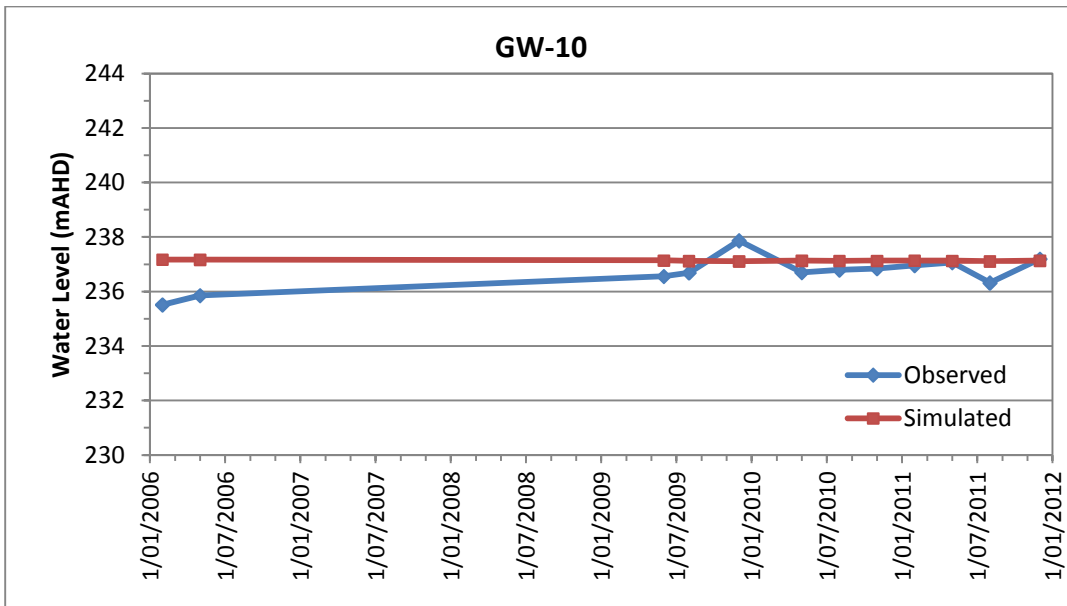
Canyon Coal Mine Monitoring Network

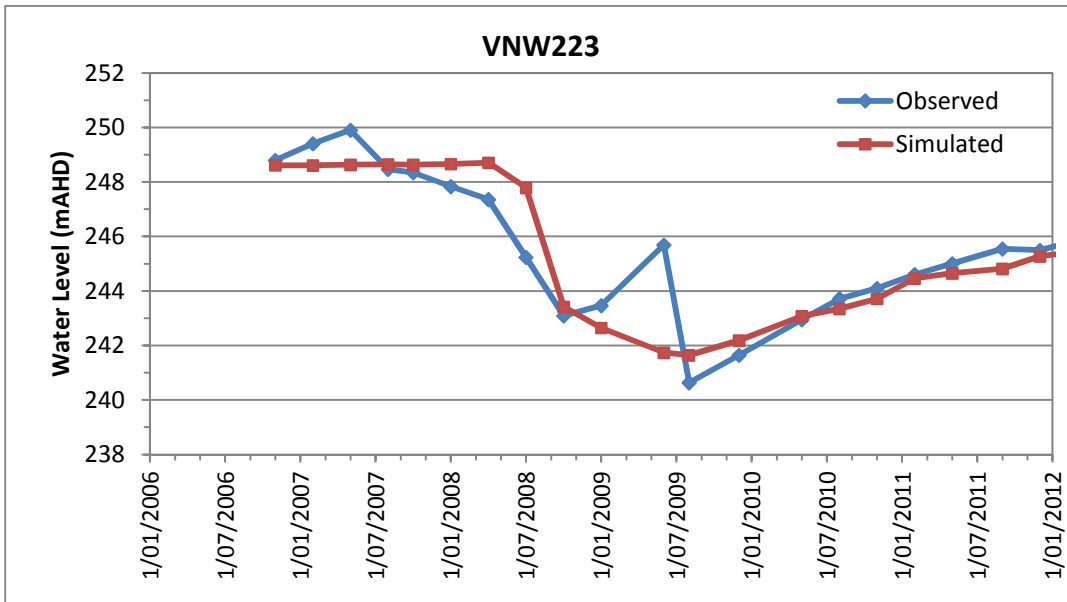
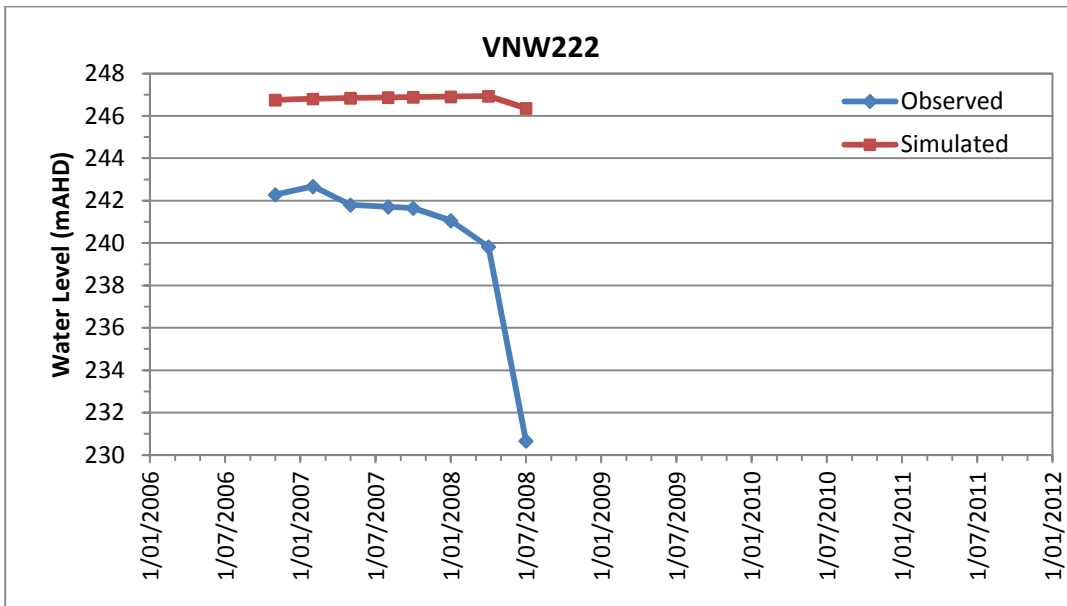
GW2, GW4, GW5, GW7 to GW11

VNW221, VNW222, VNW223





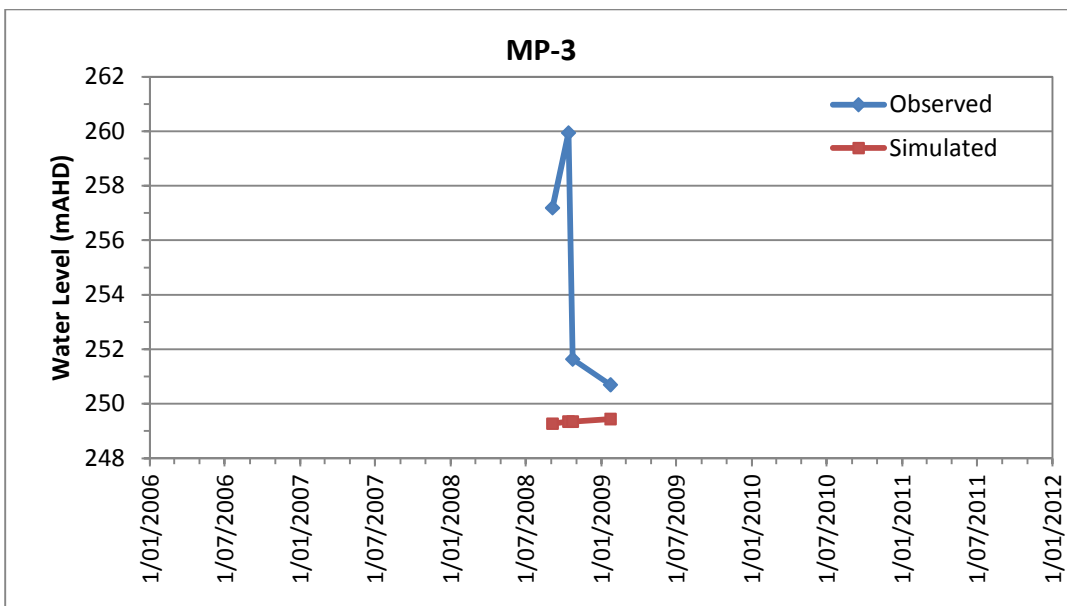
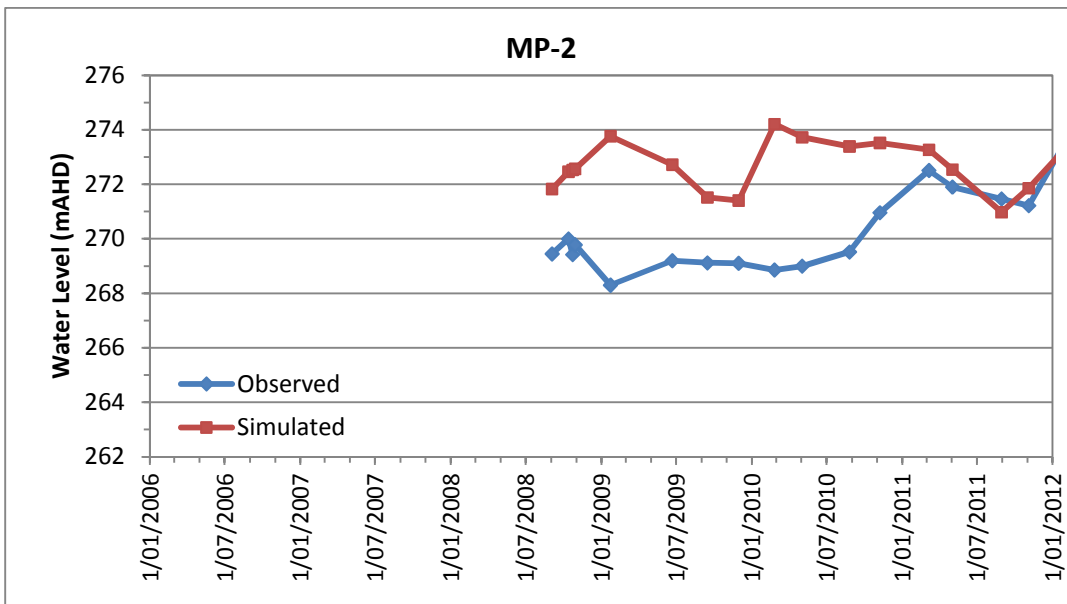
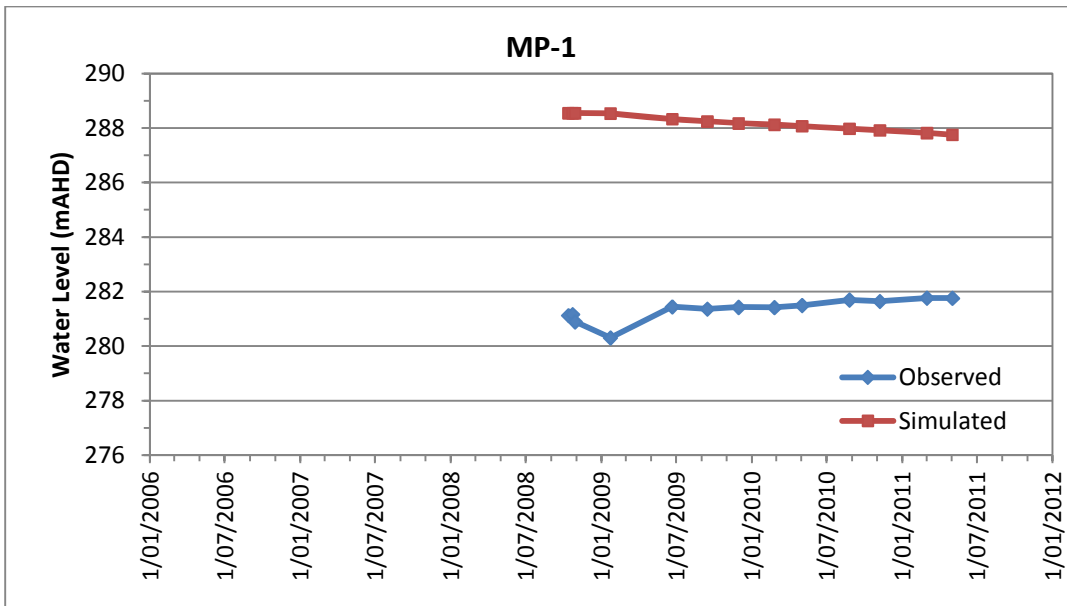


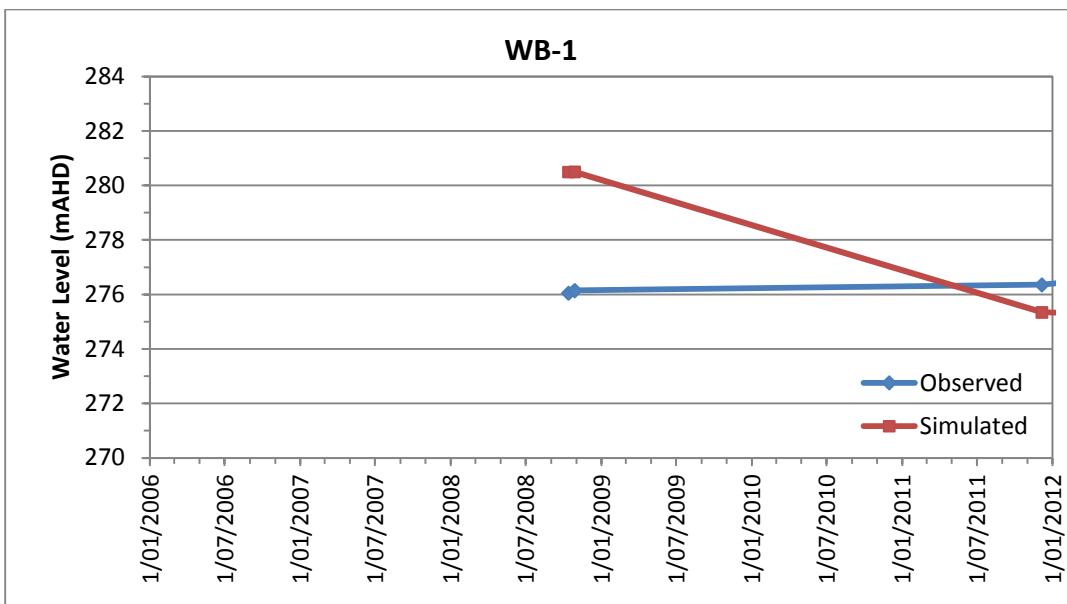
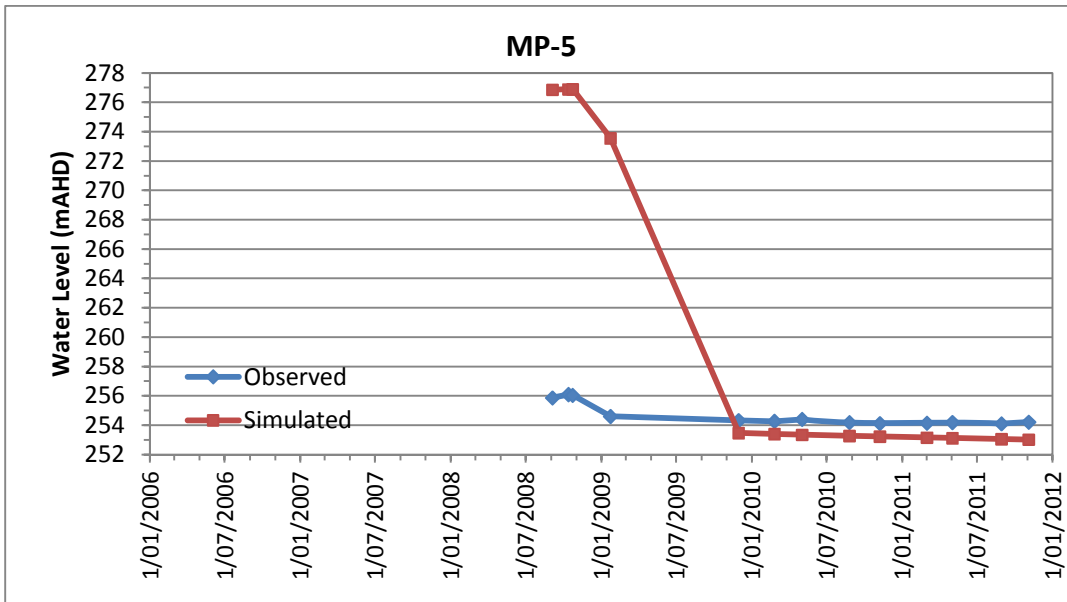
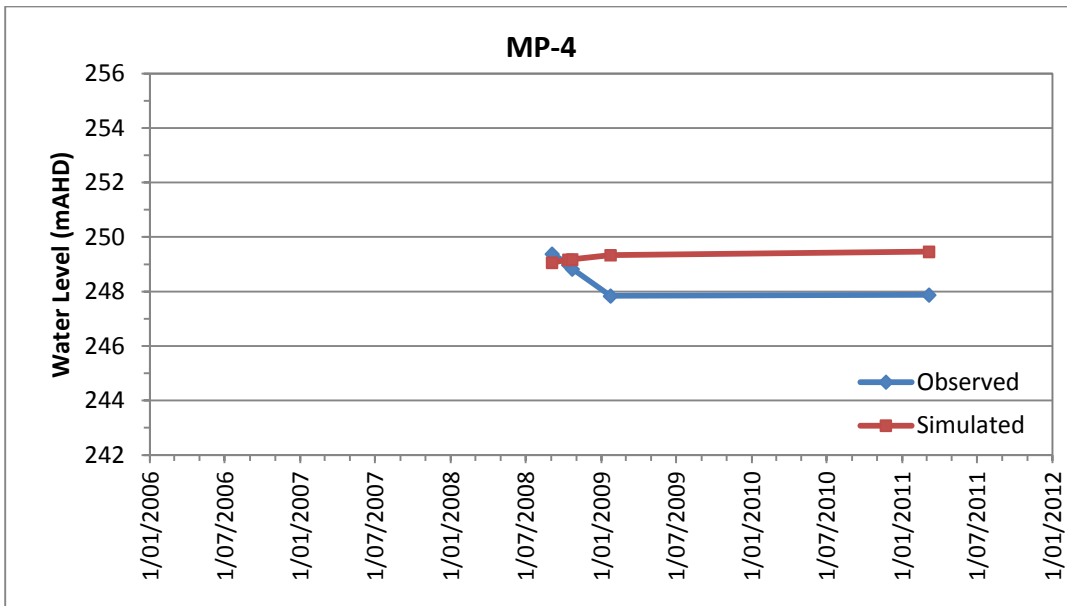


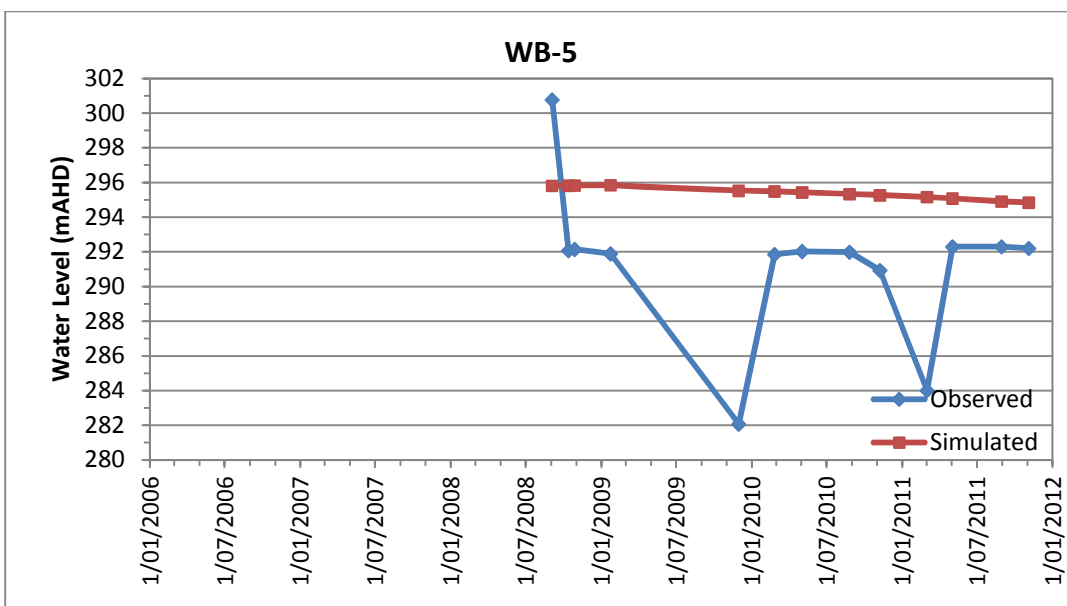
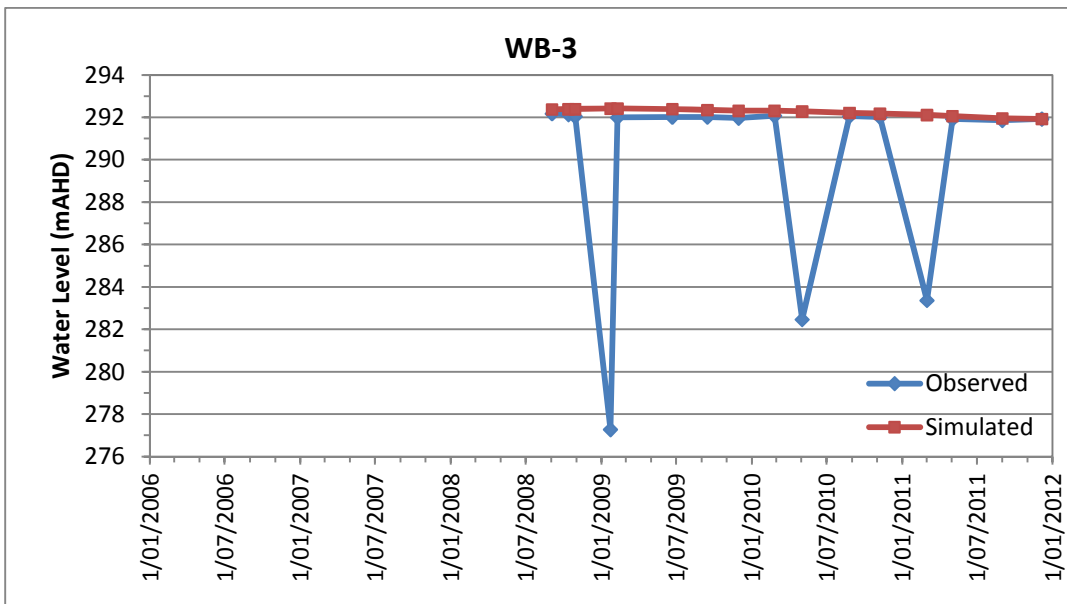
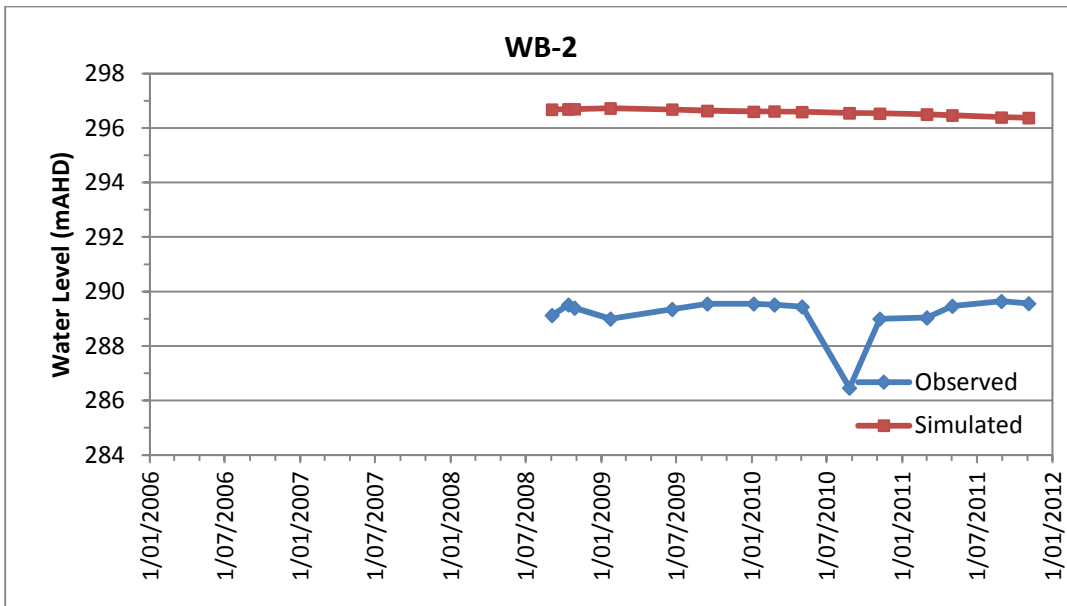
Rocglen Coal Mine Monitoring Network

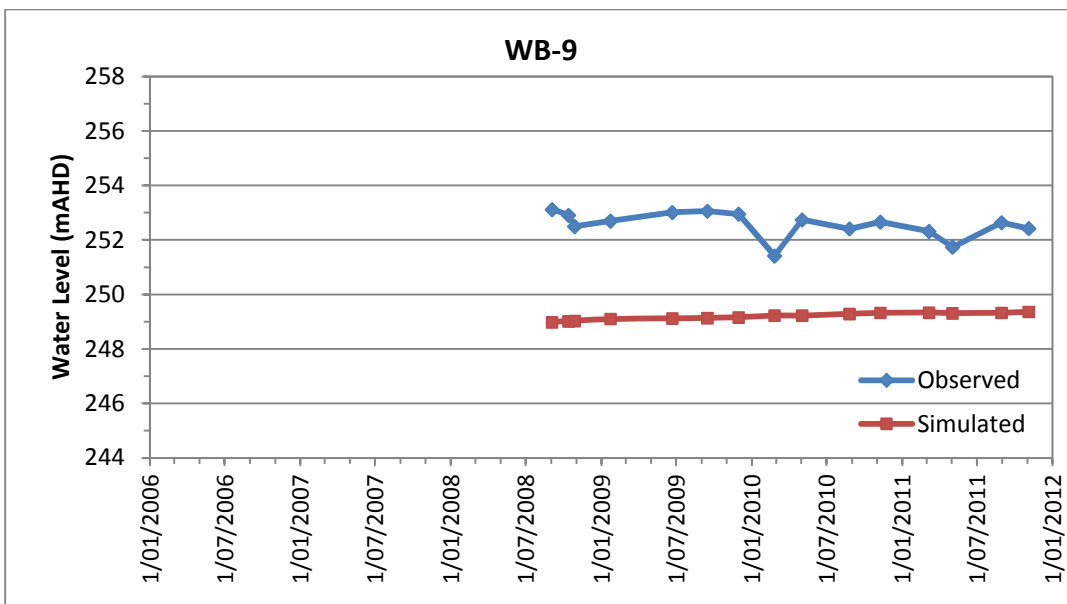
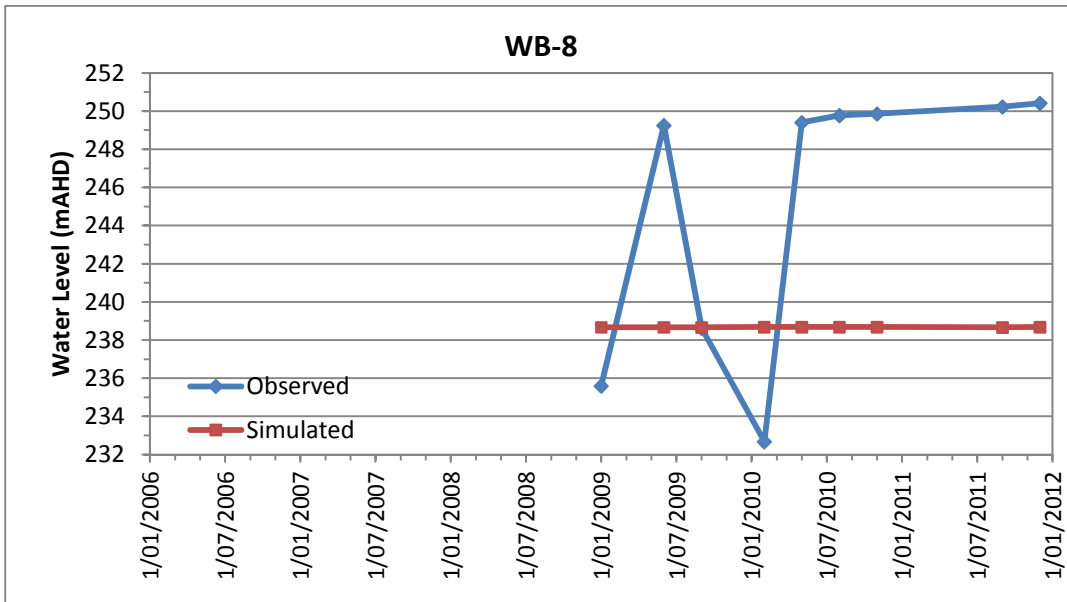
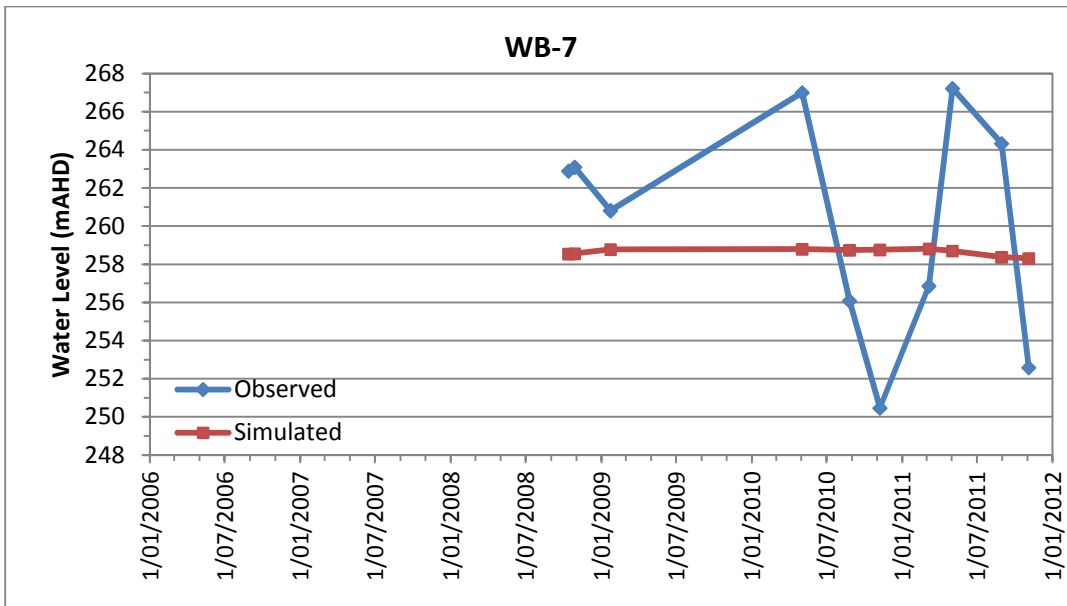
MP Series 1-5

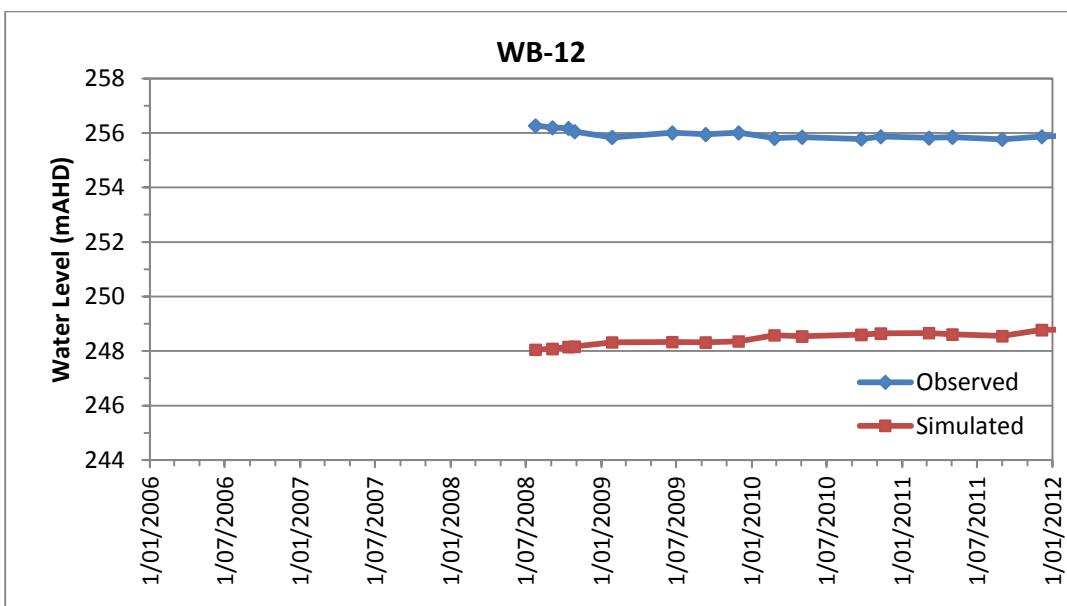
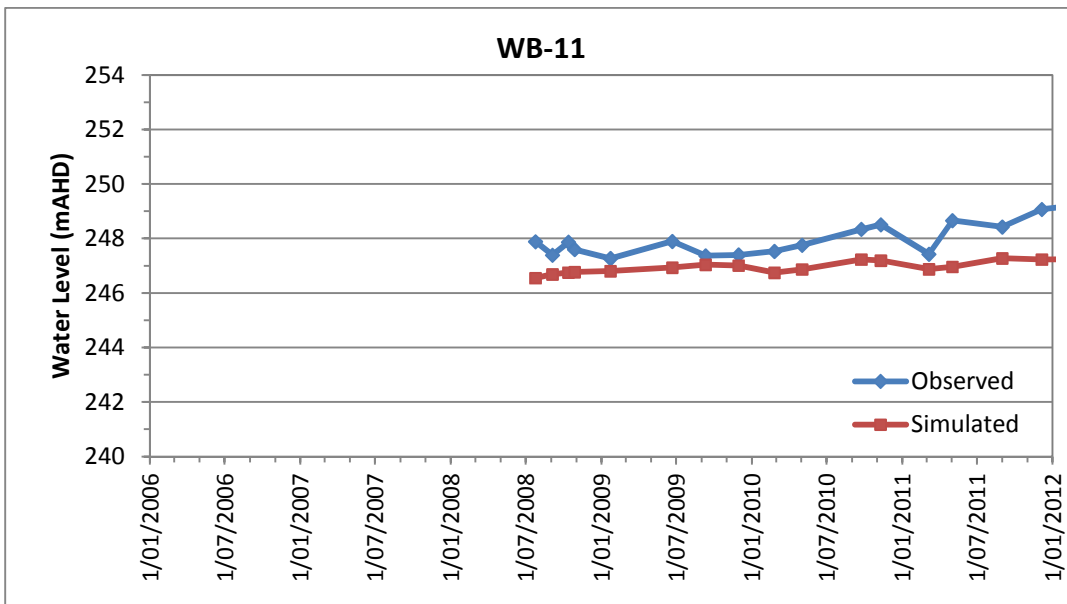
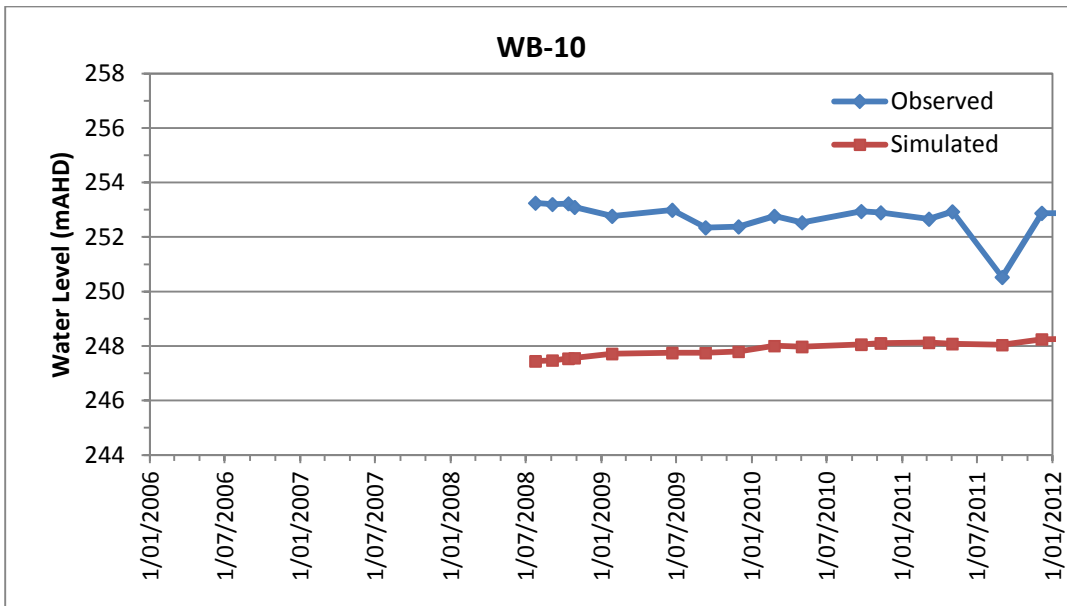
WB Series 1, 2, 3, 5, 7, 8, 9, 10, 12





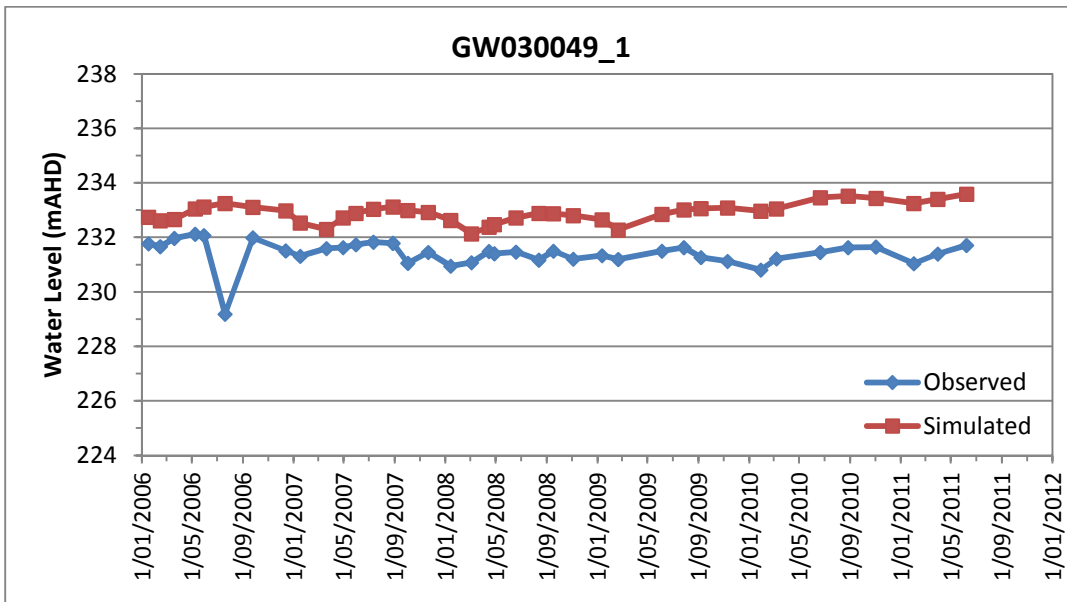
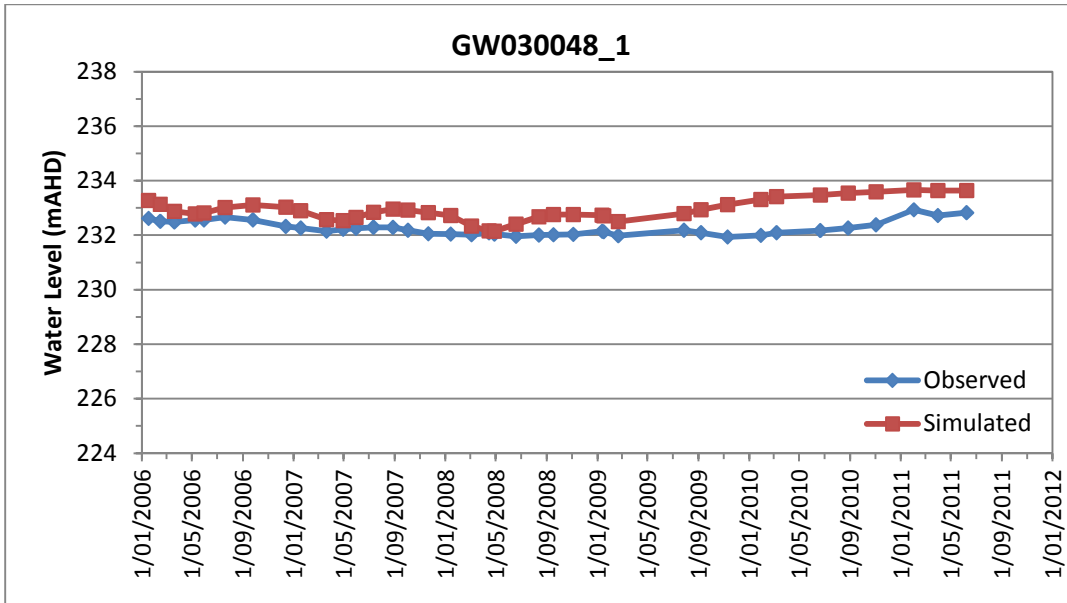


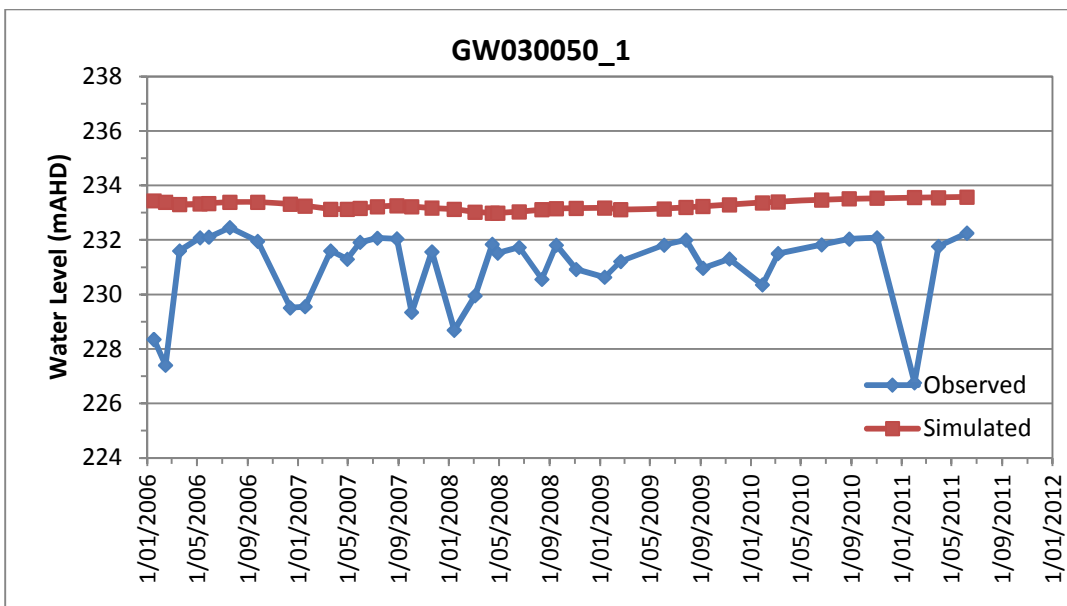
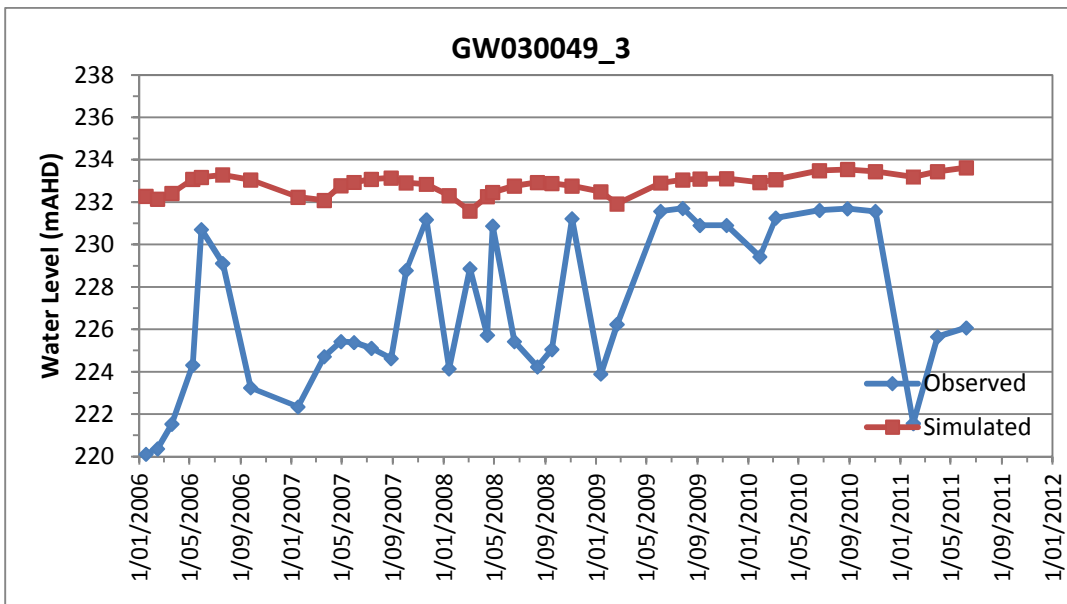
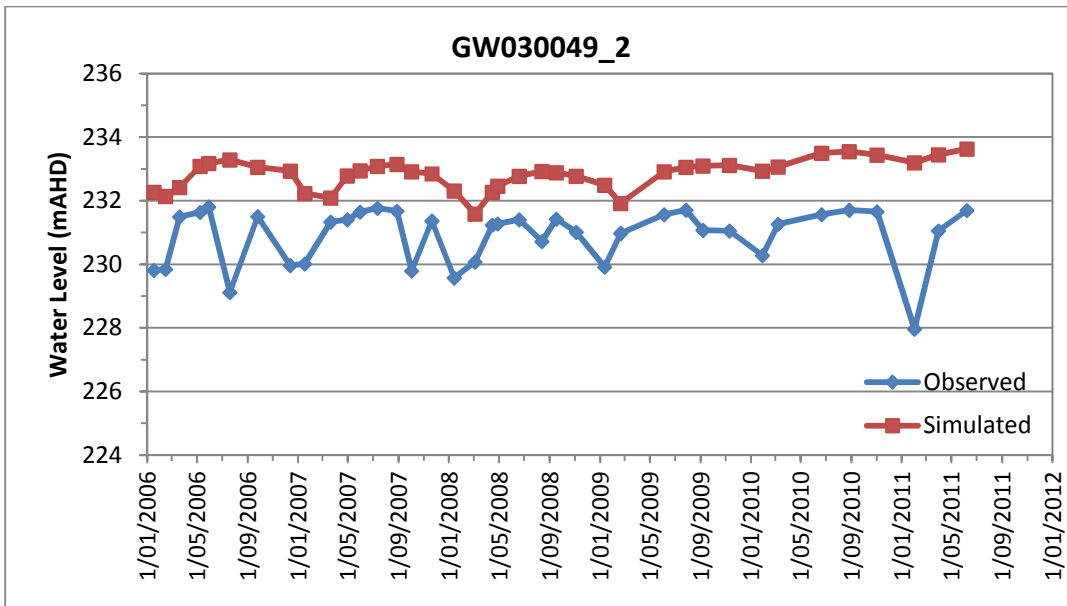


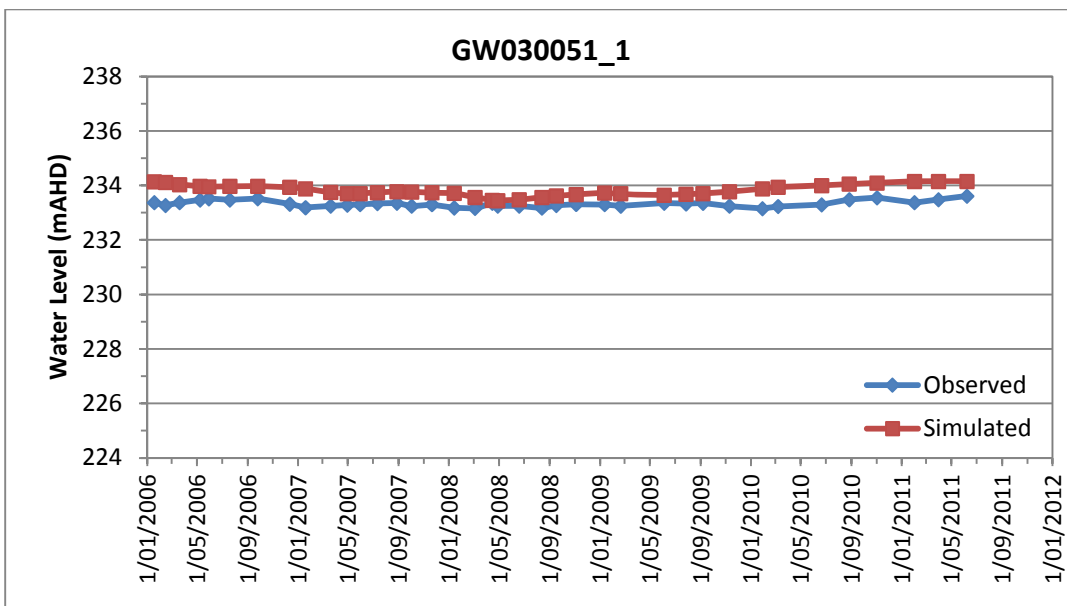
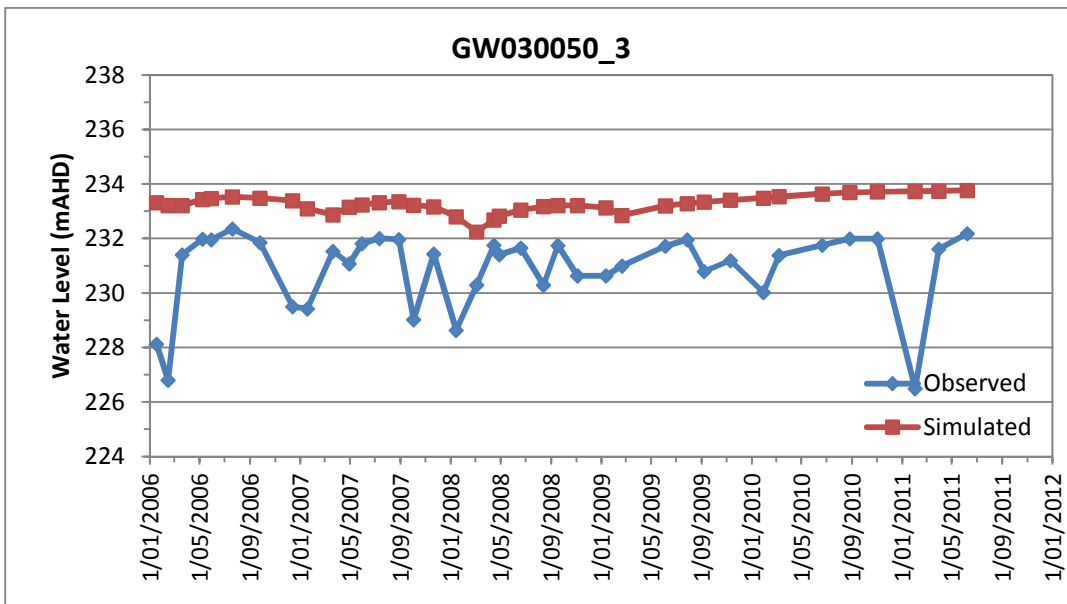
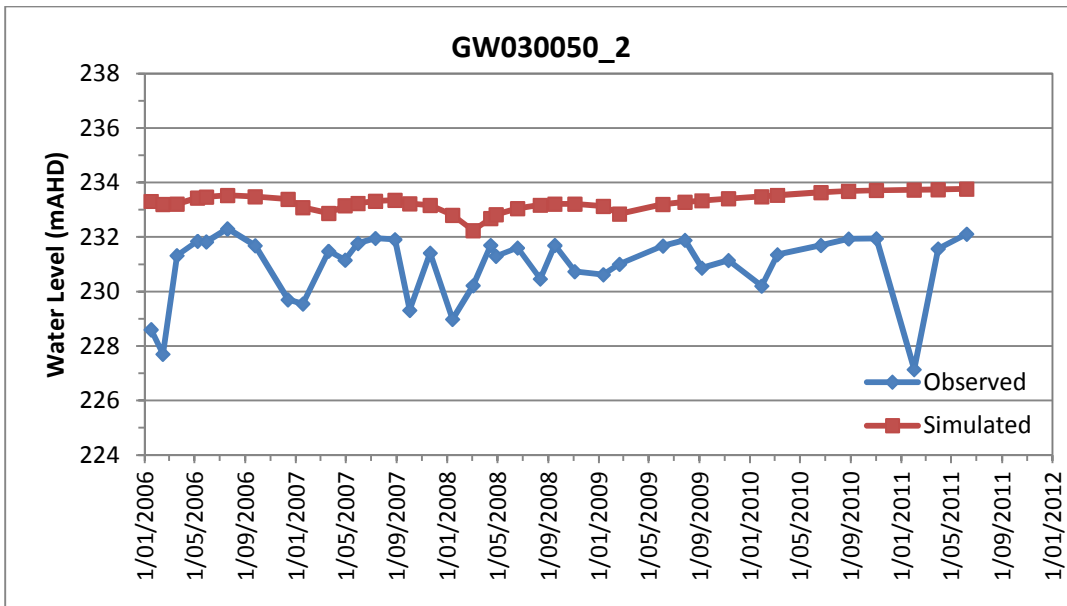


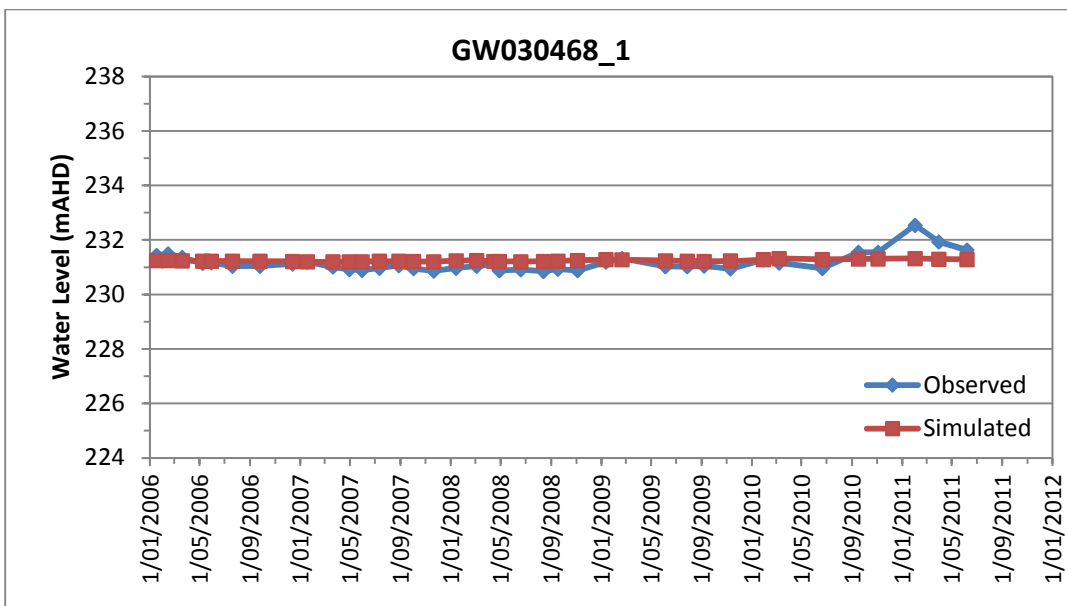
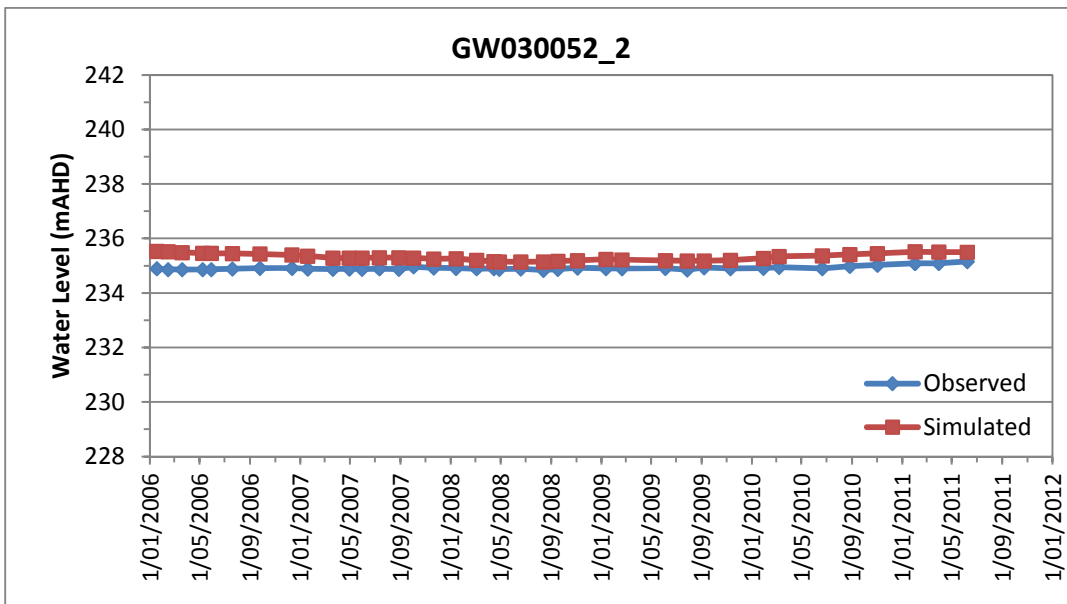
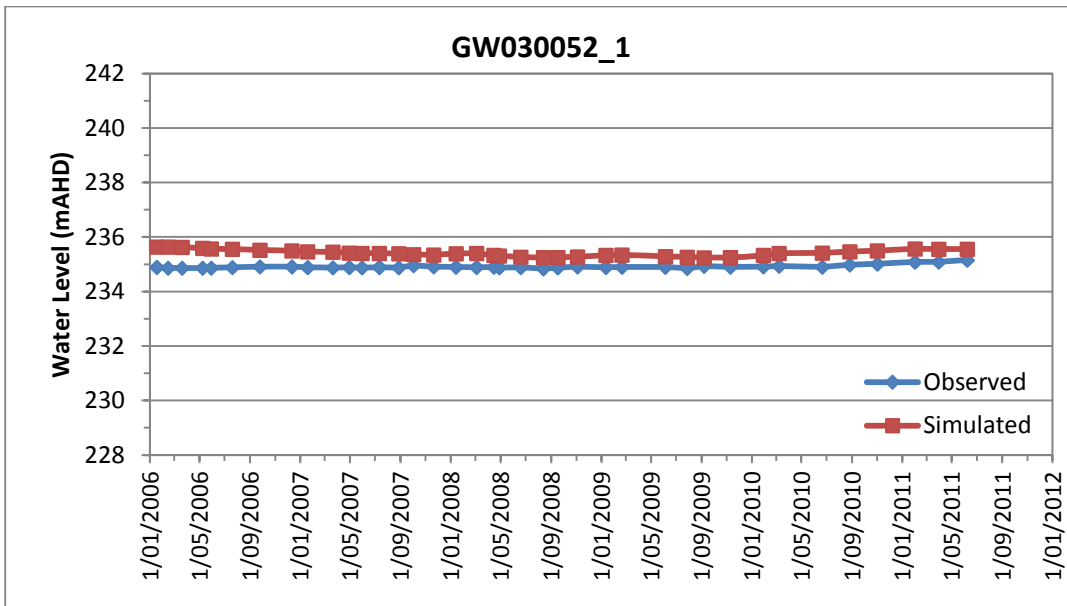
NSW Office of Water Monitoring Network – Alluvium (Zone 4)

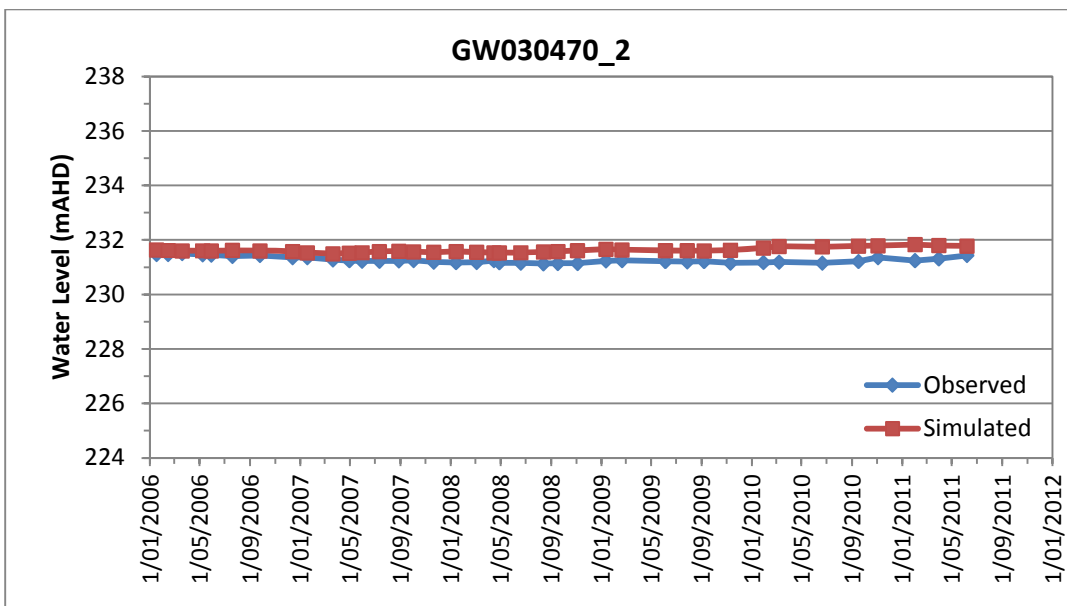
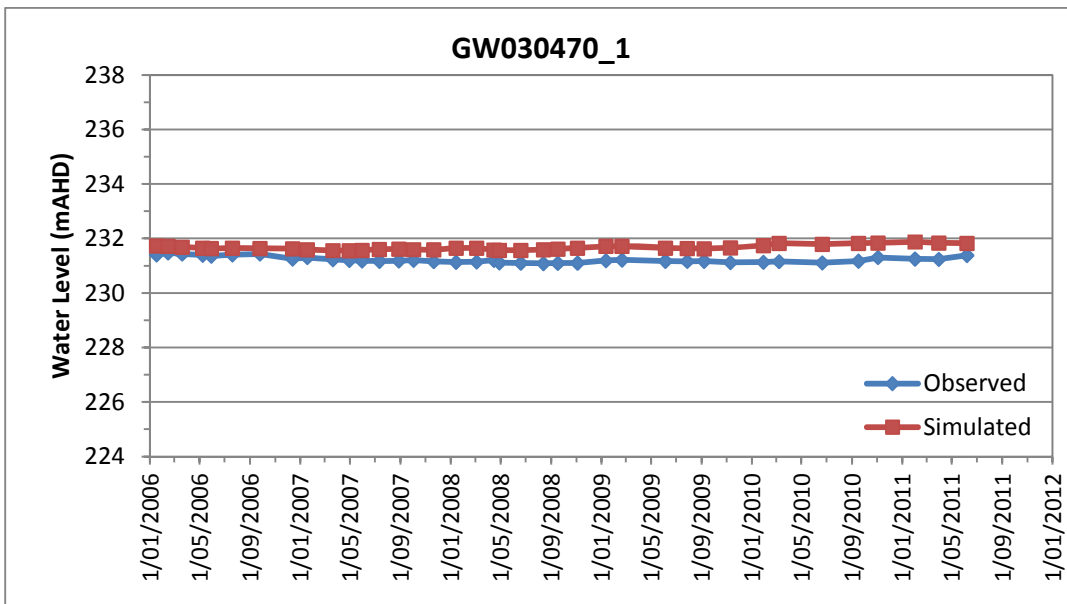
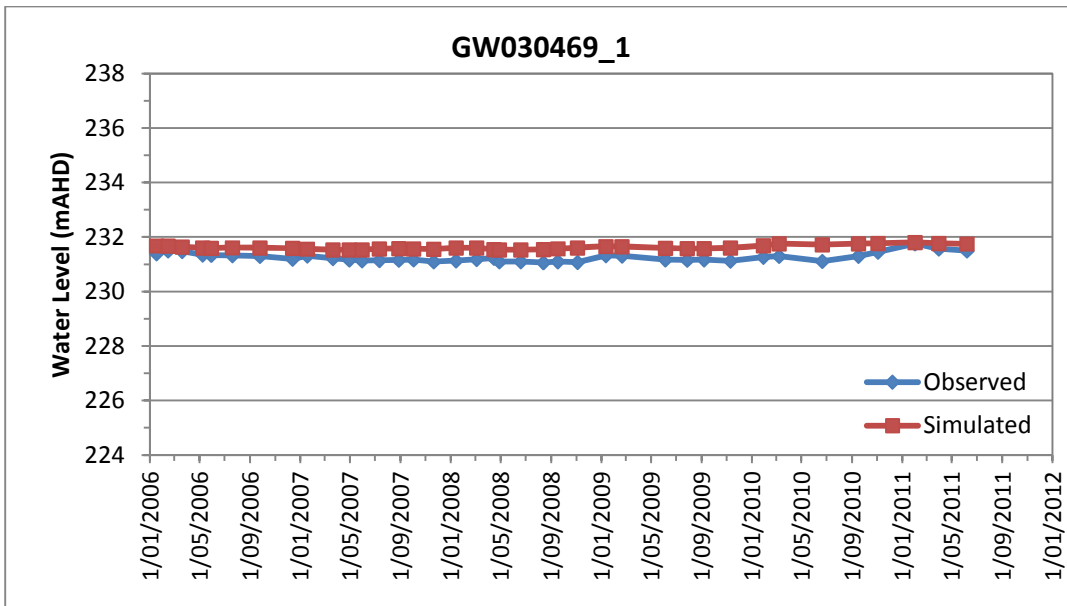
GW030048_1
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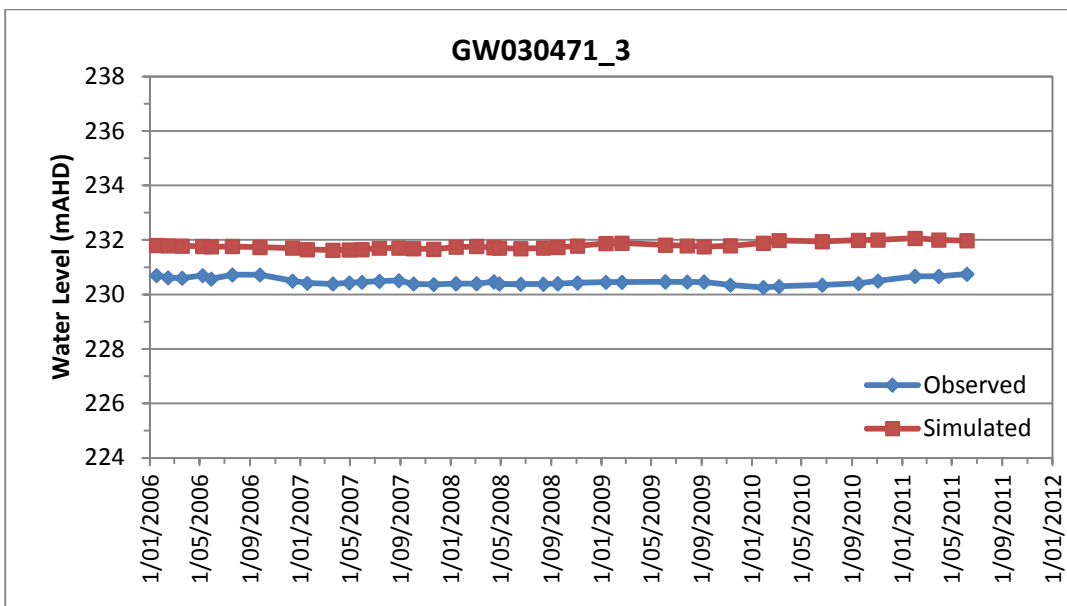
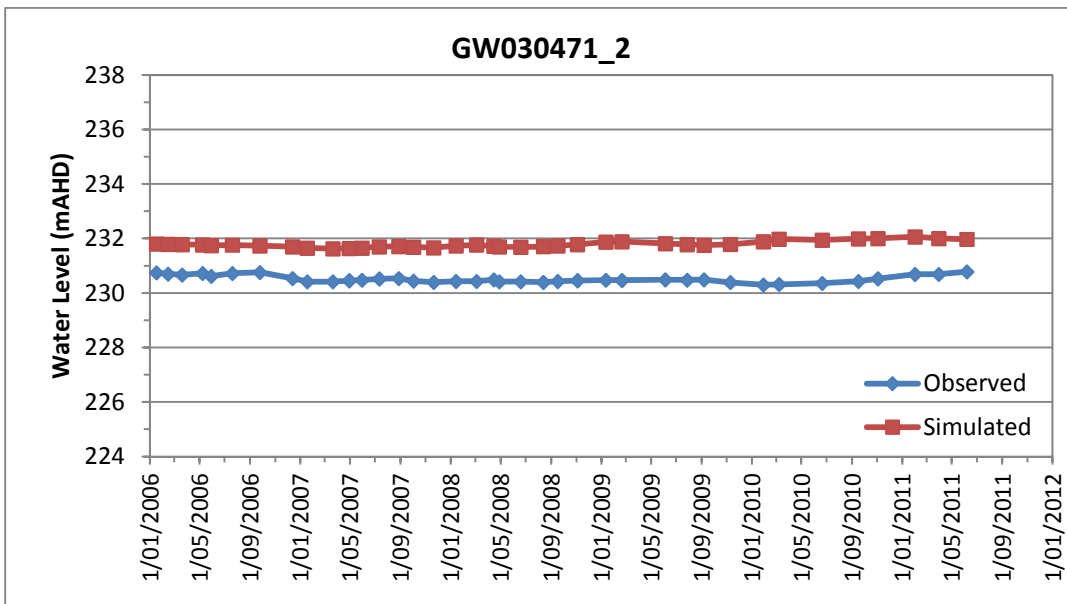
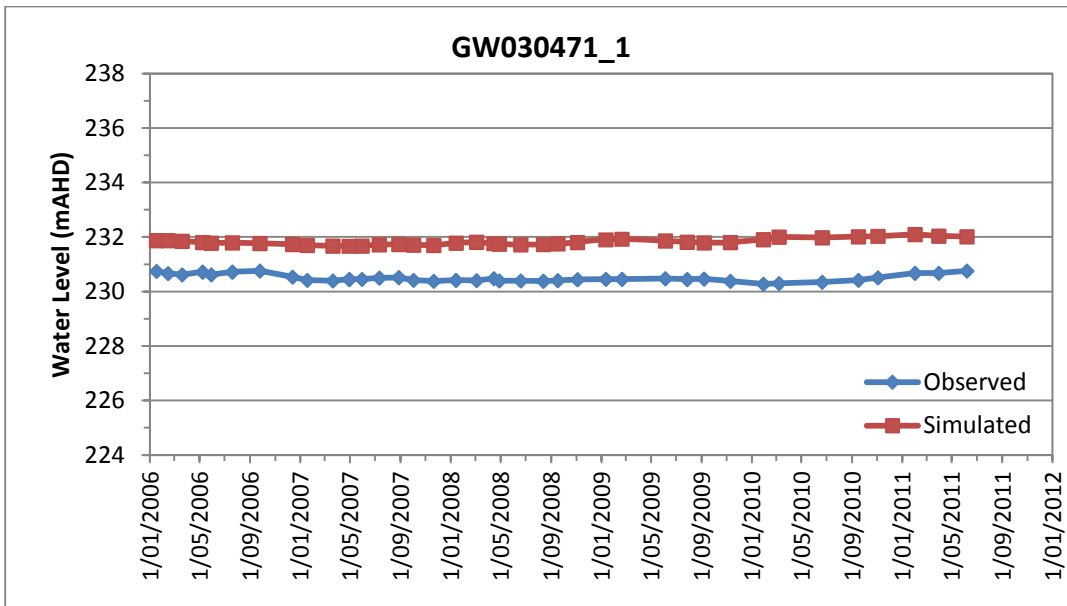


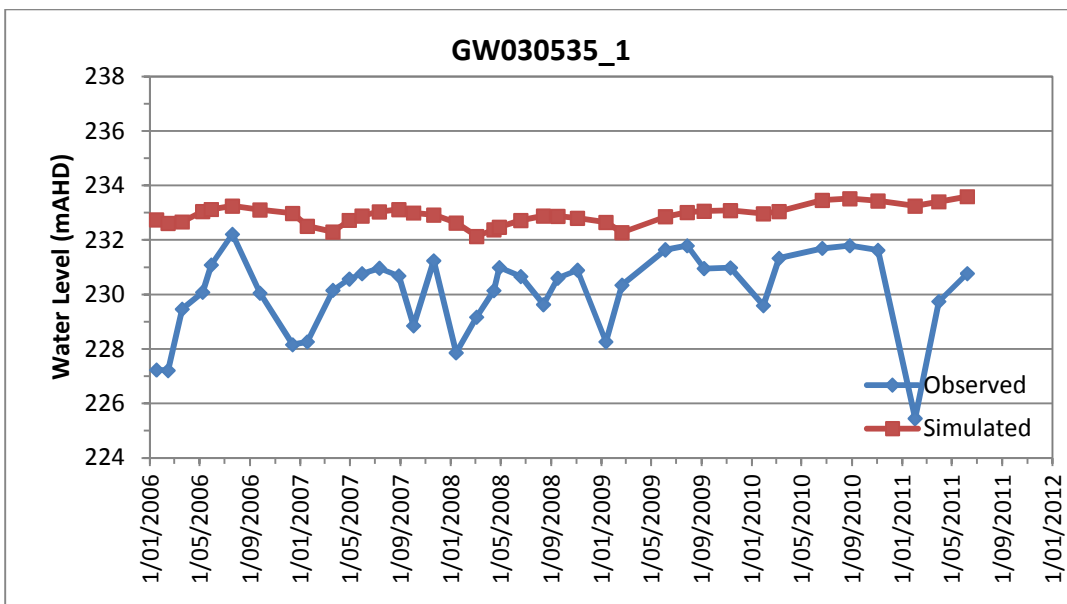
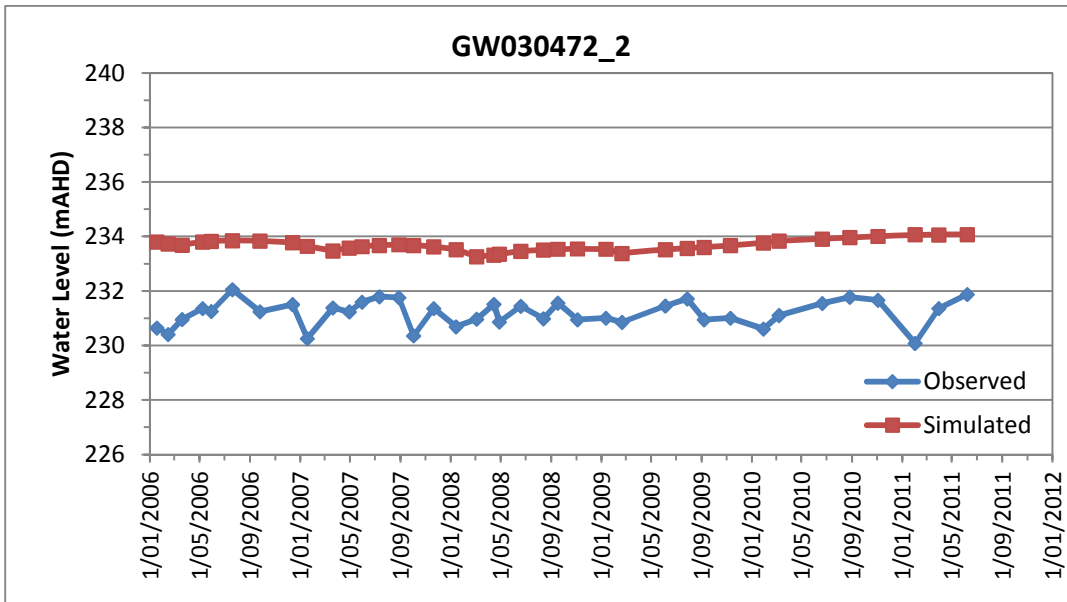
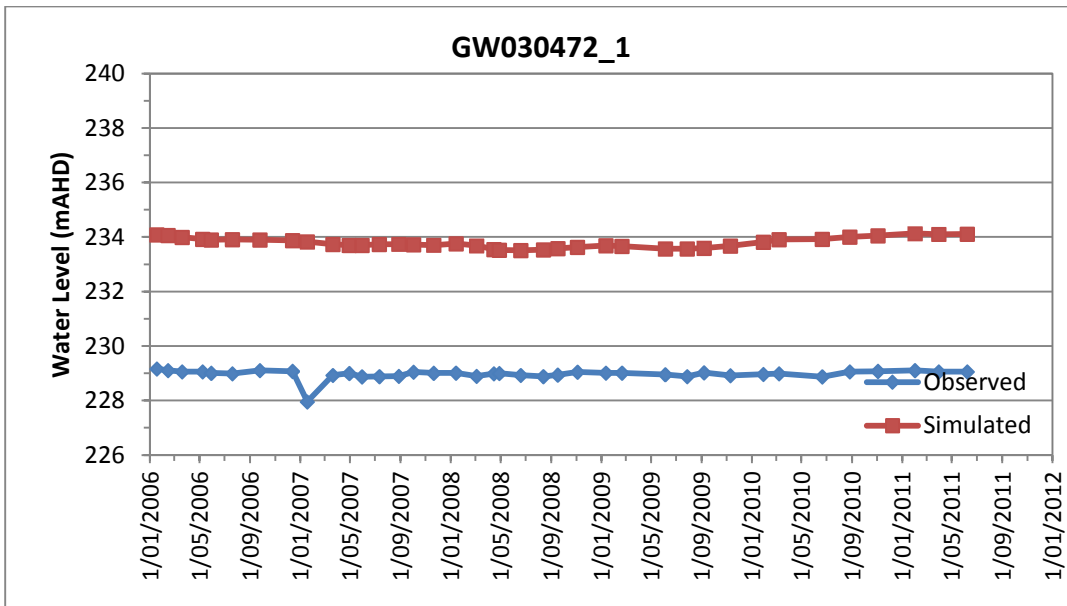


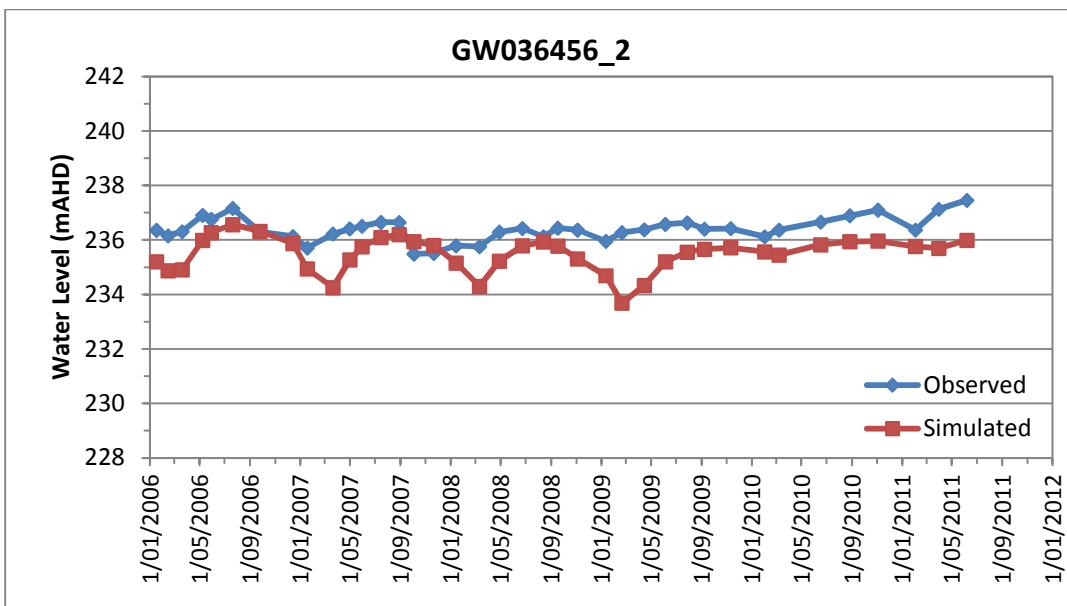
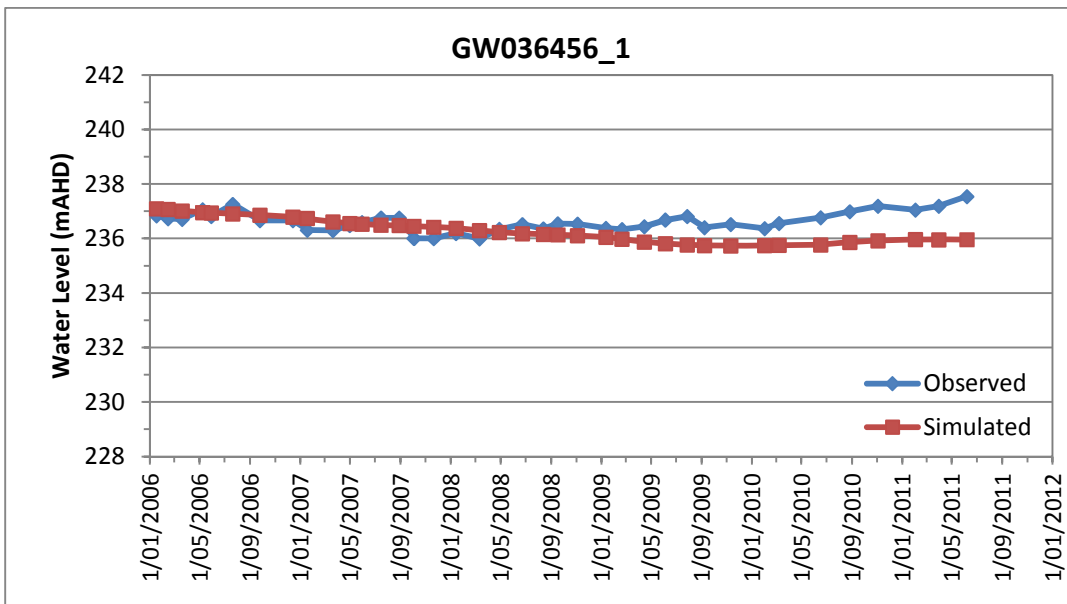
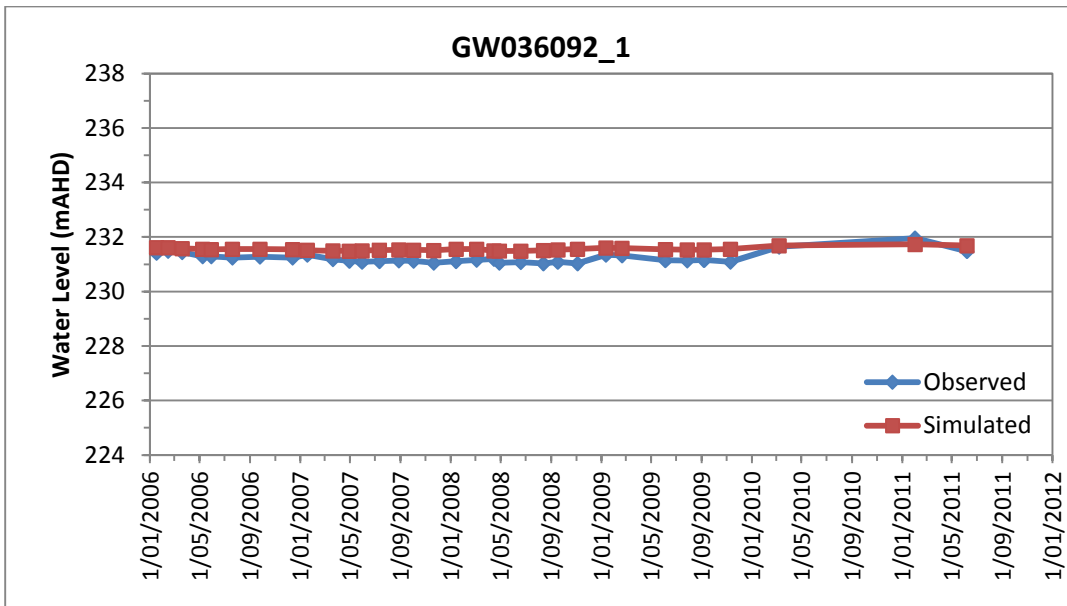


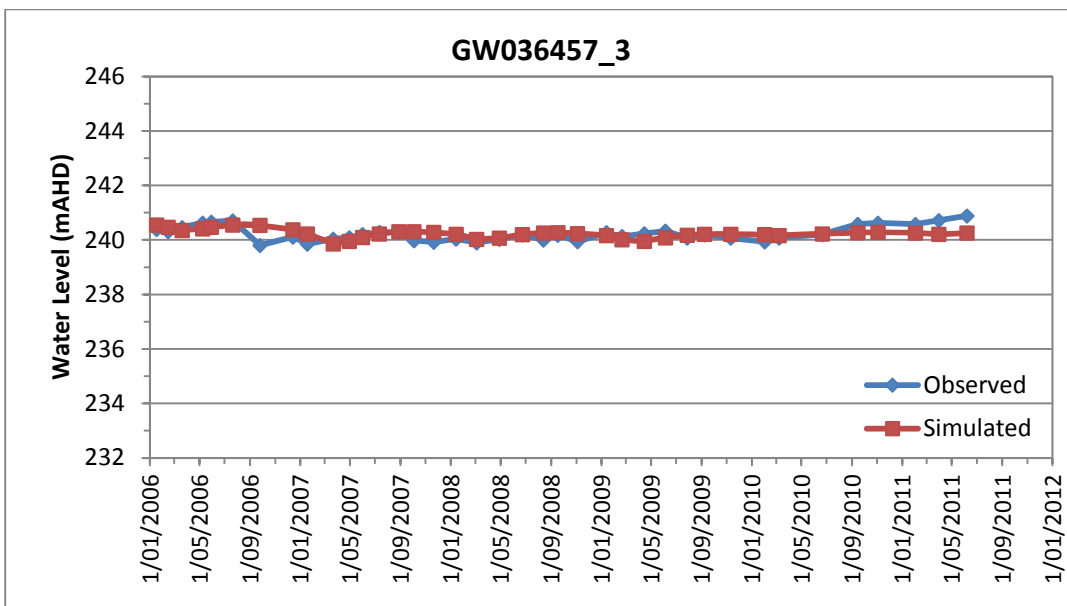
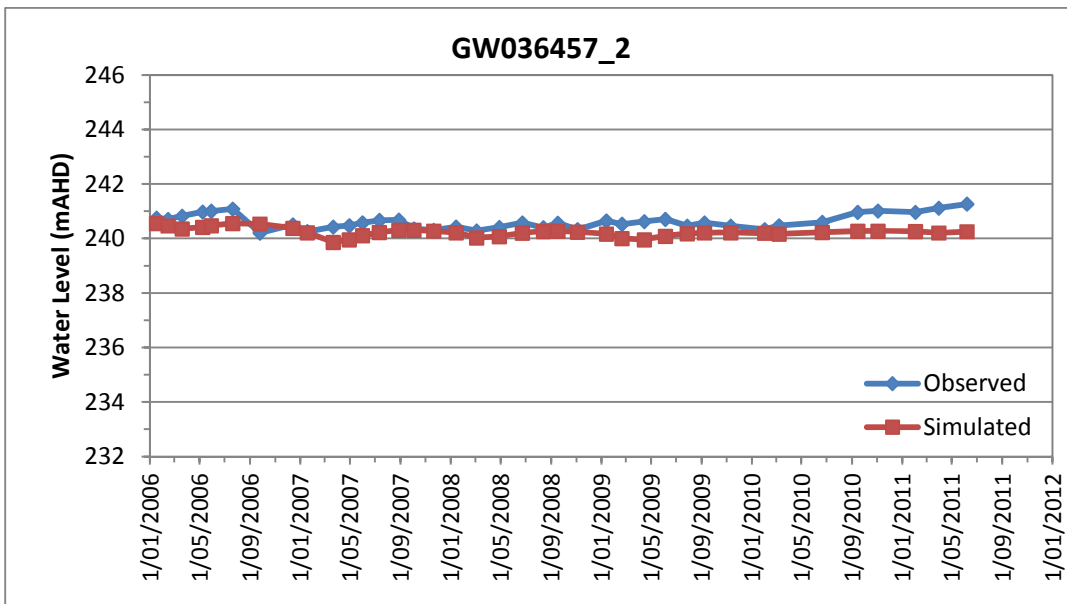
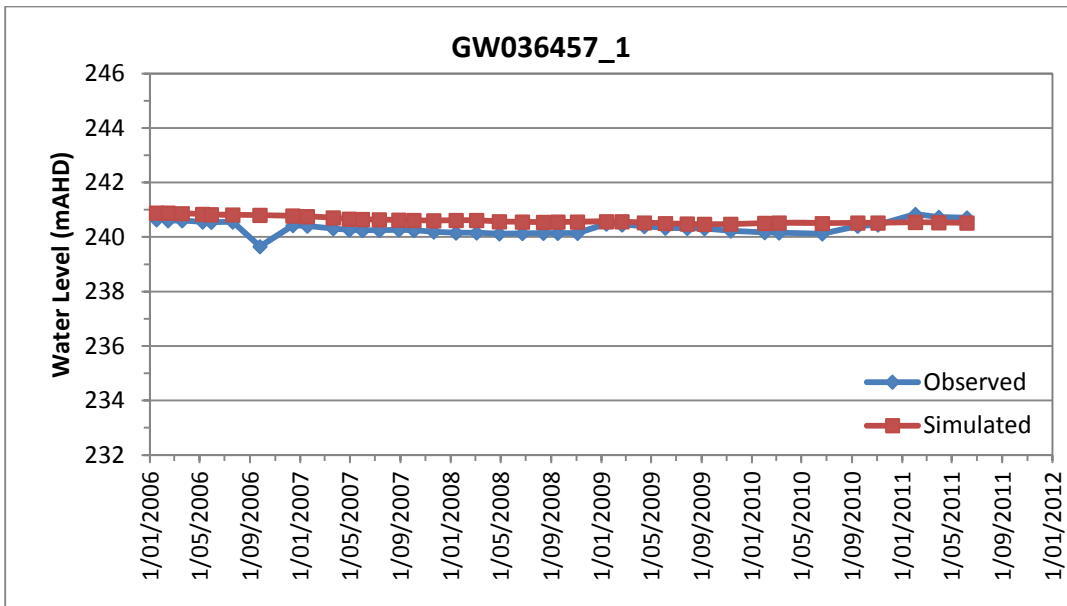


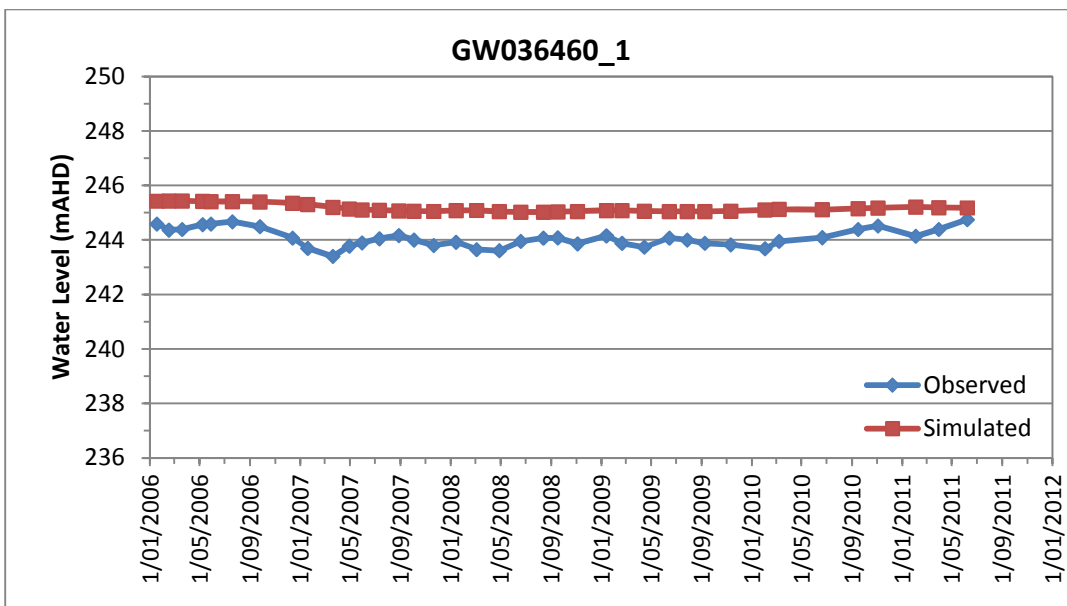
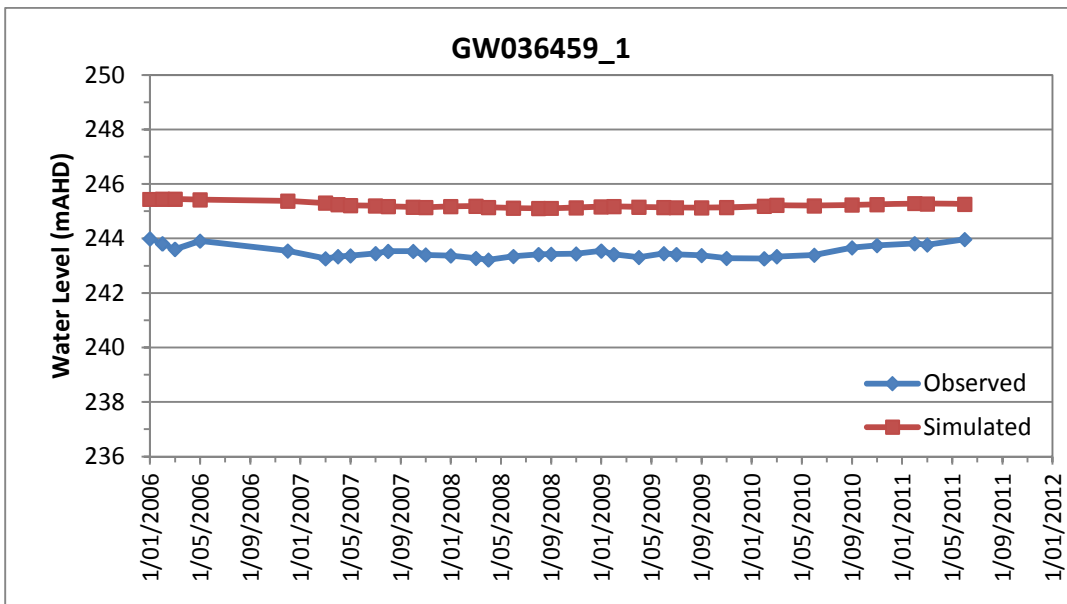
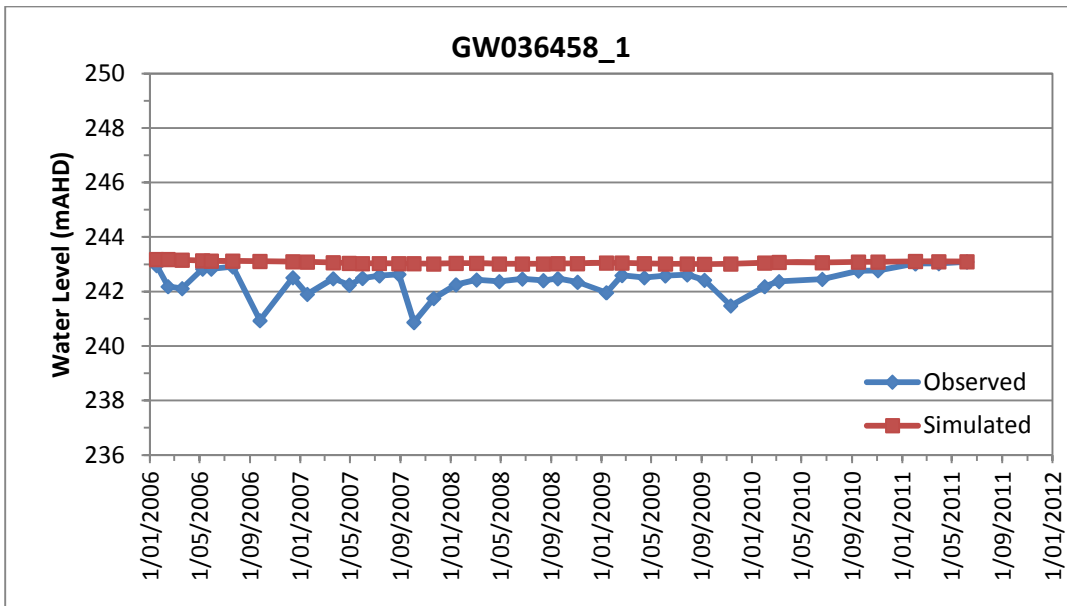


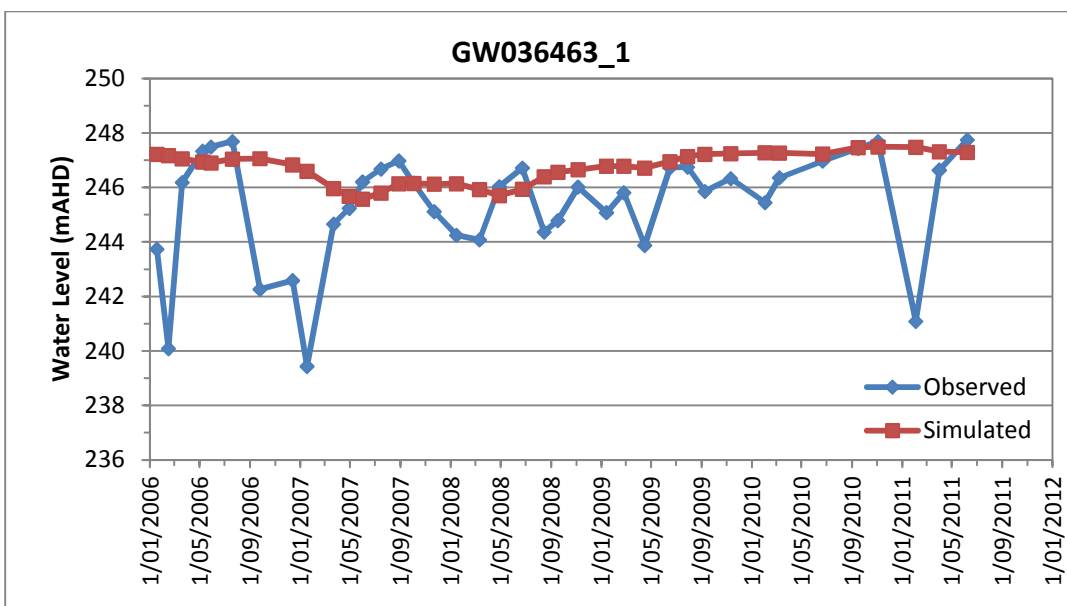
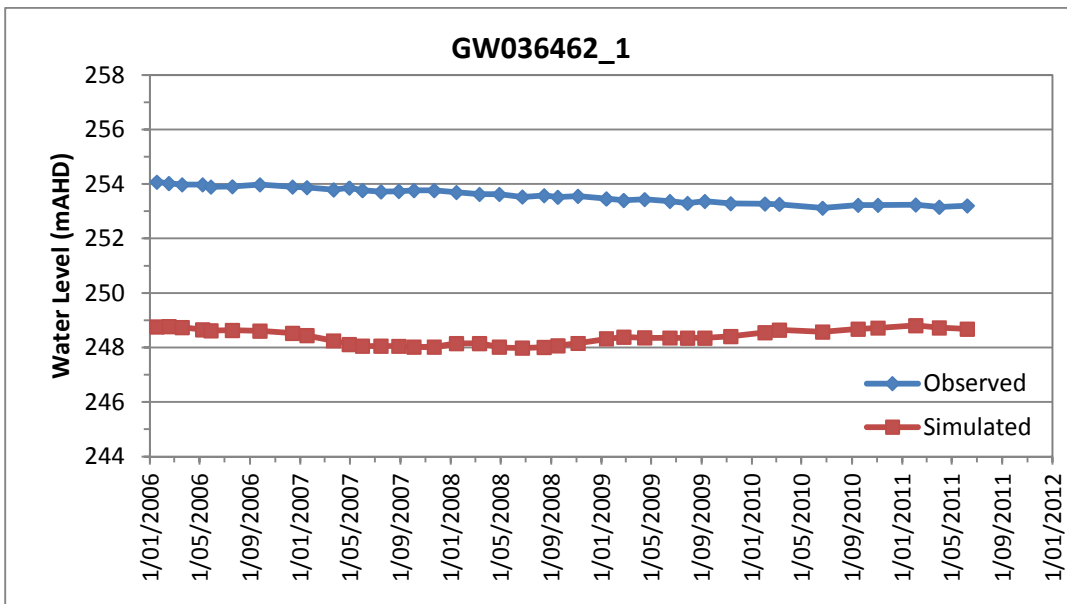
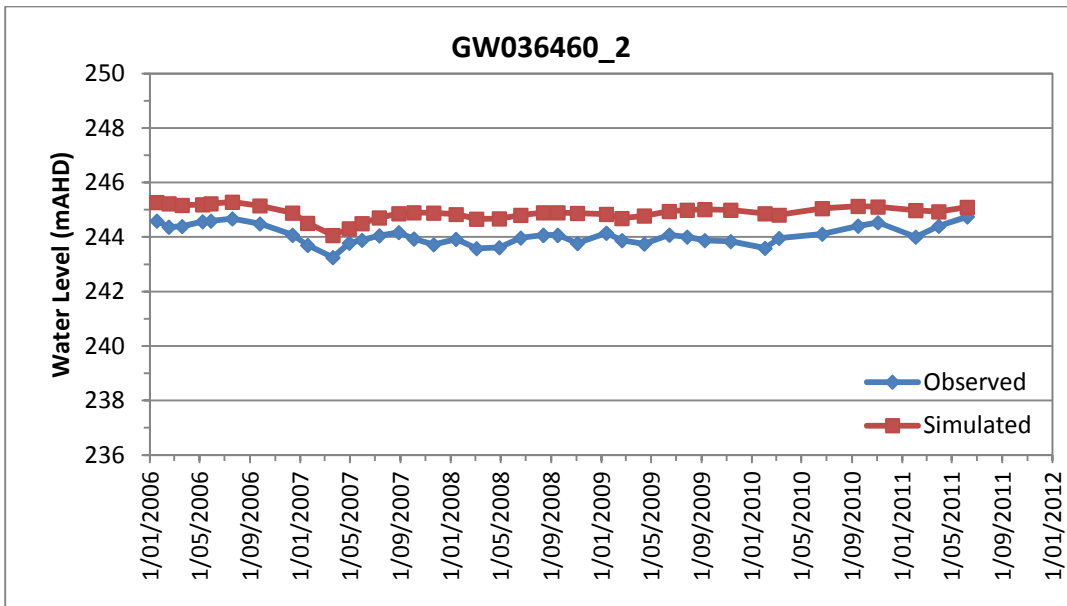


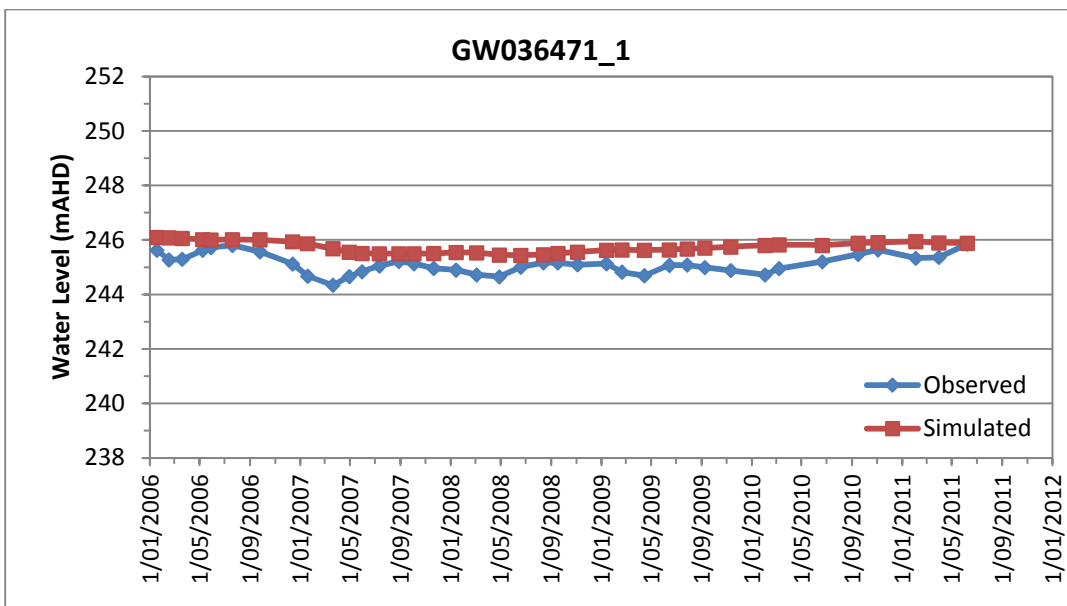
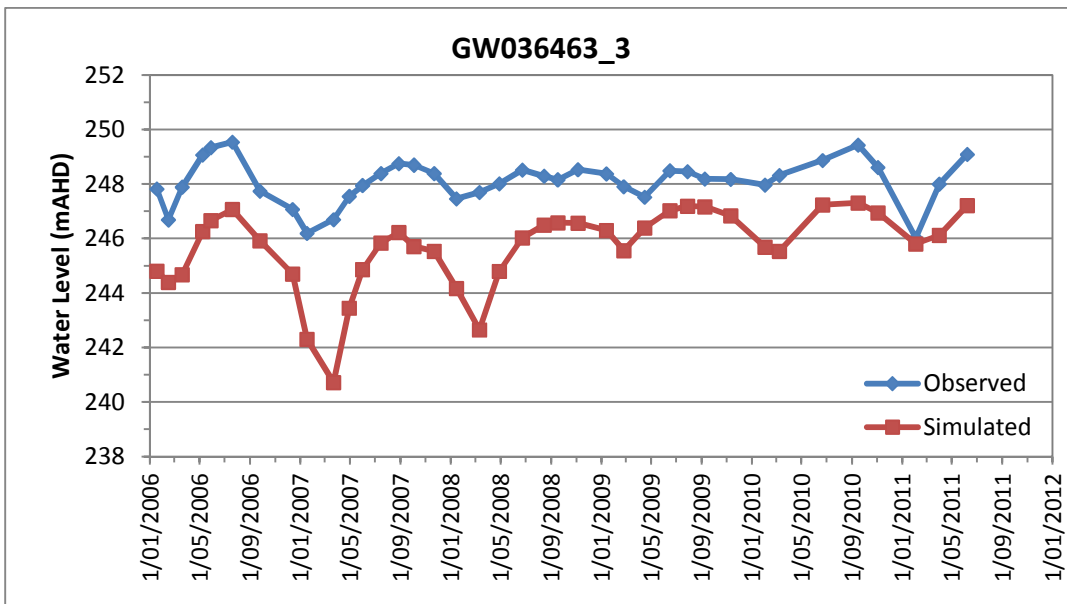
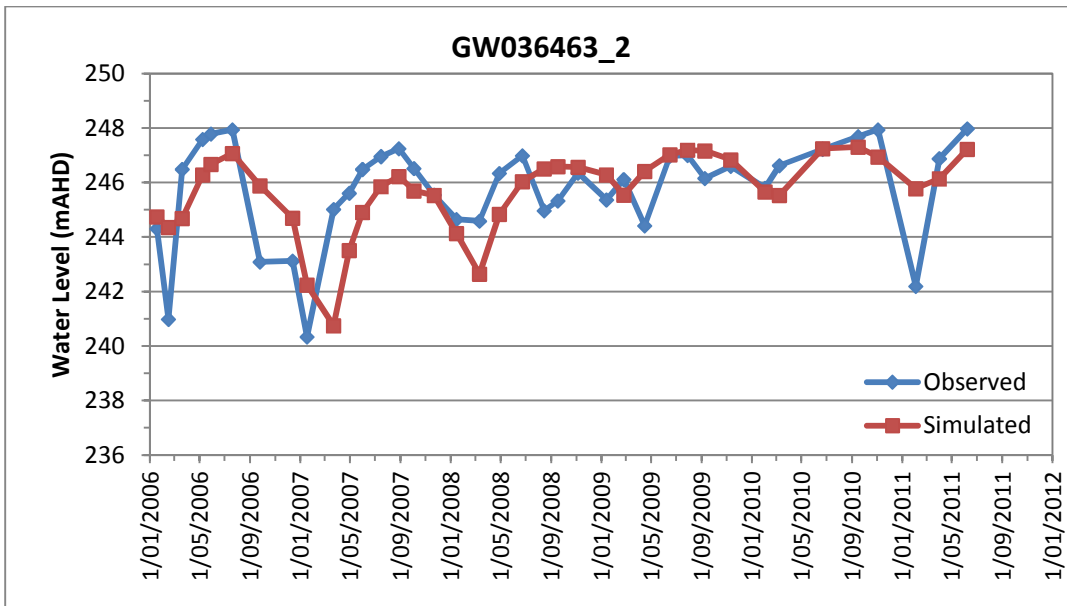


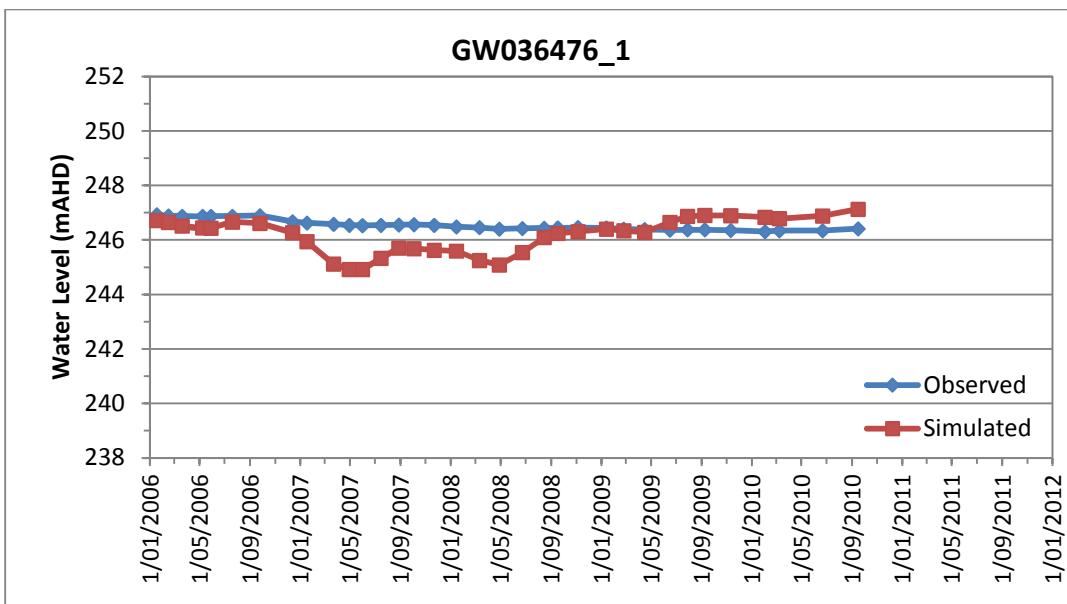
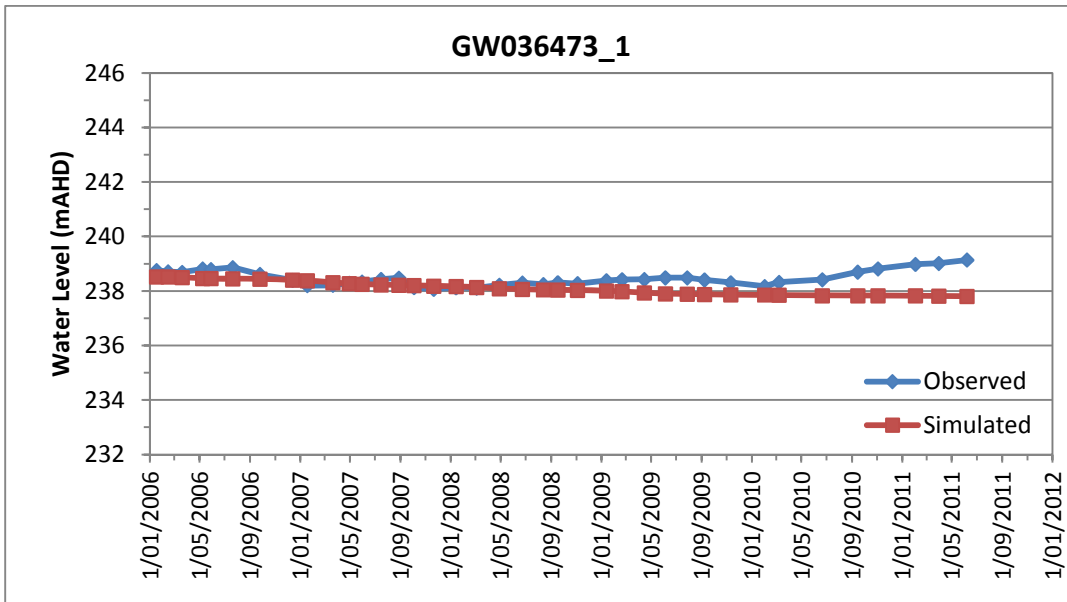
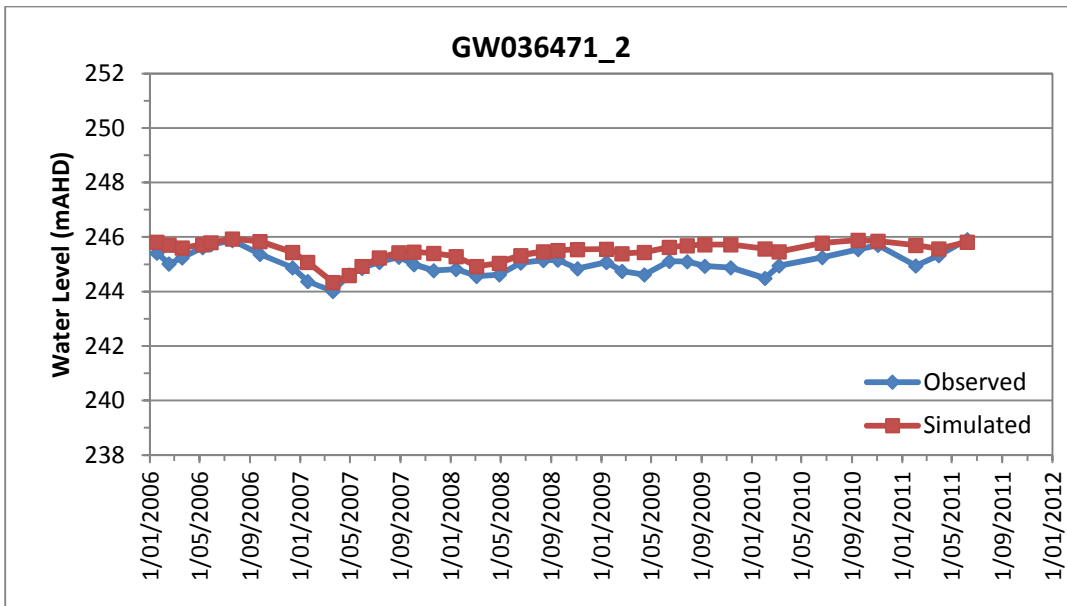


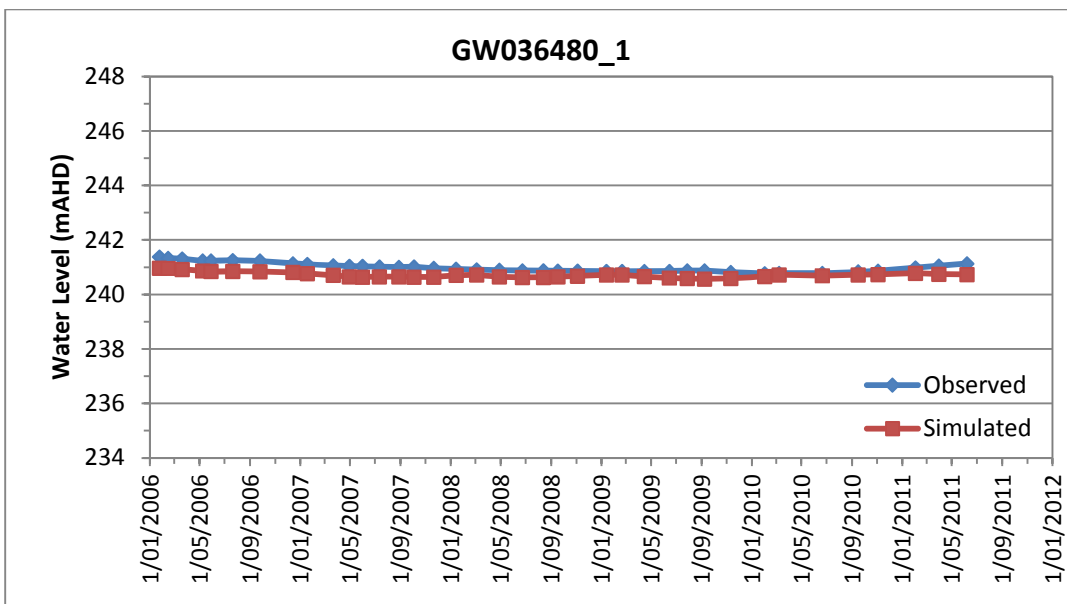
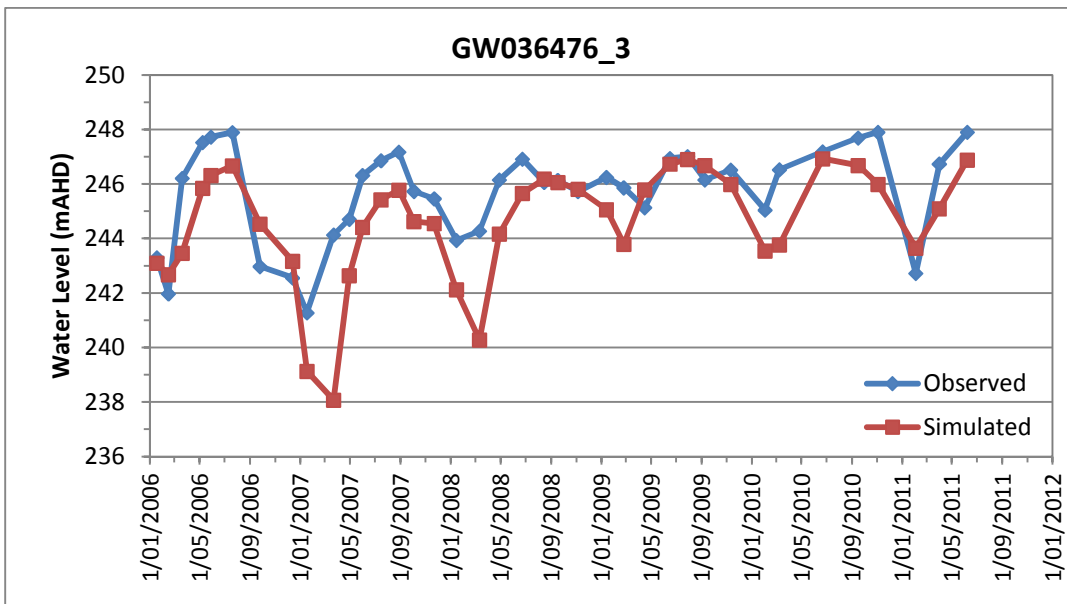
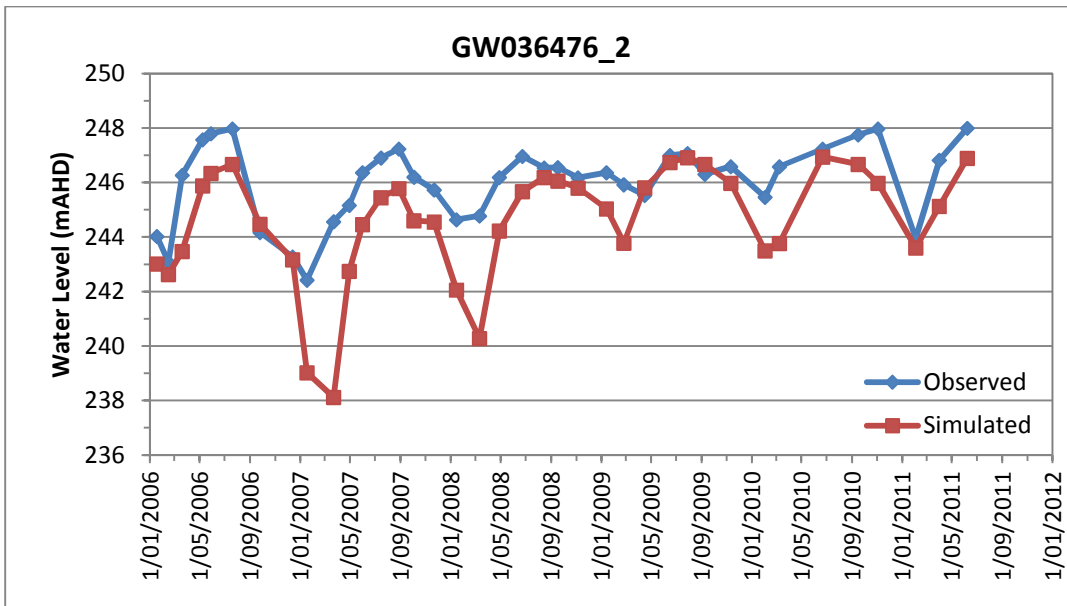


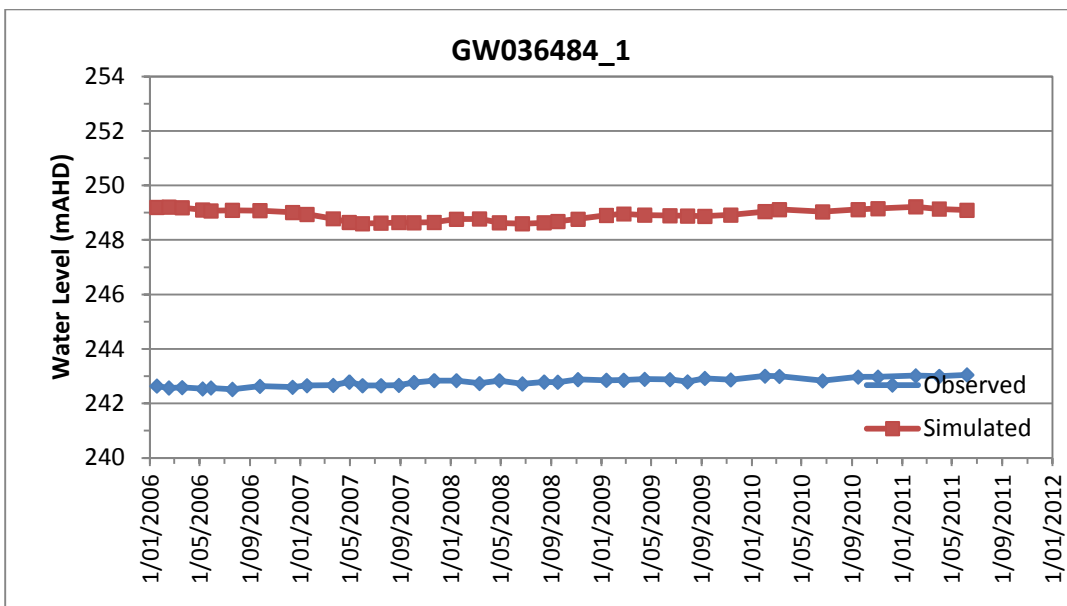
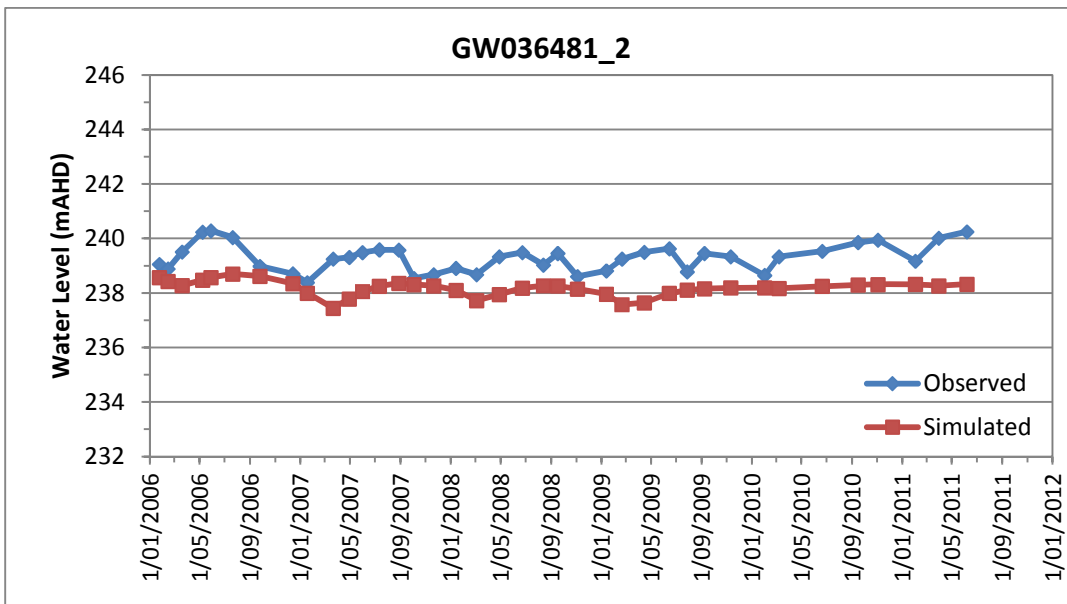
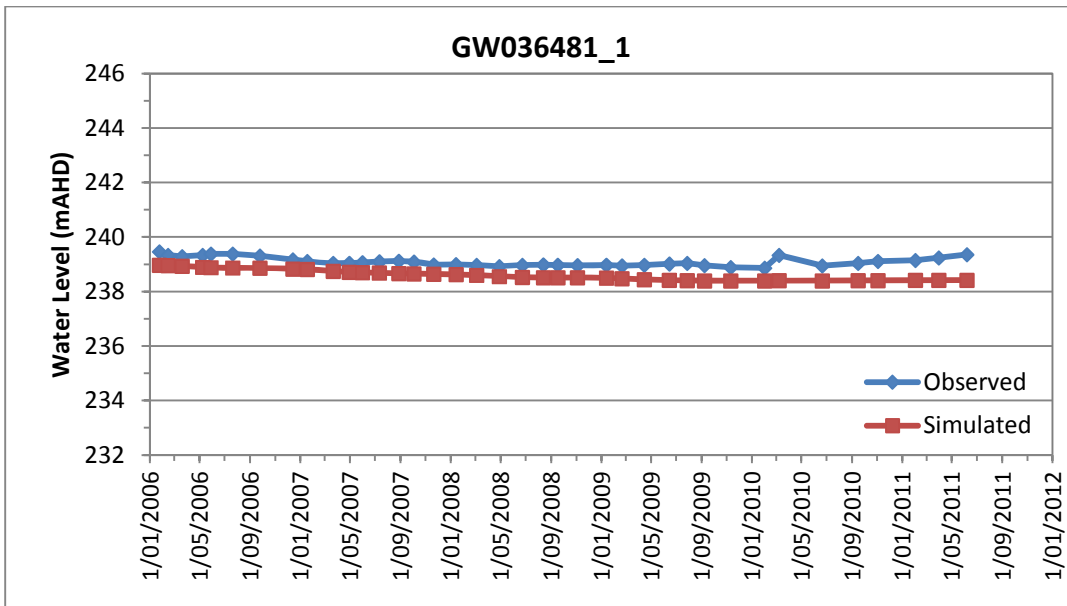


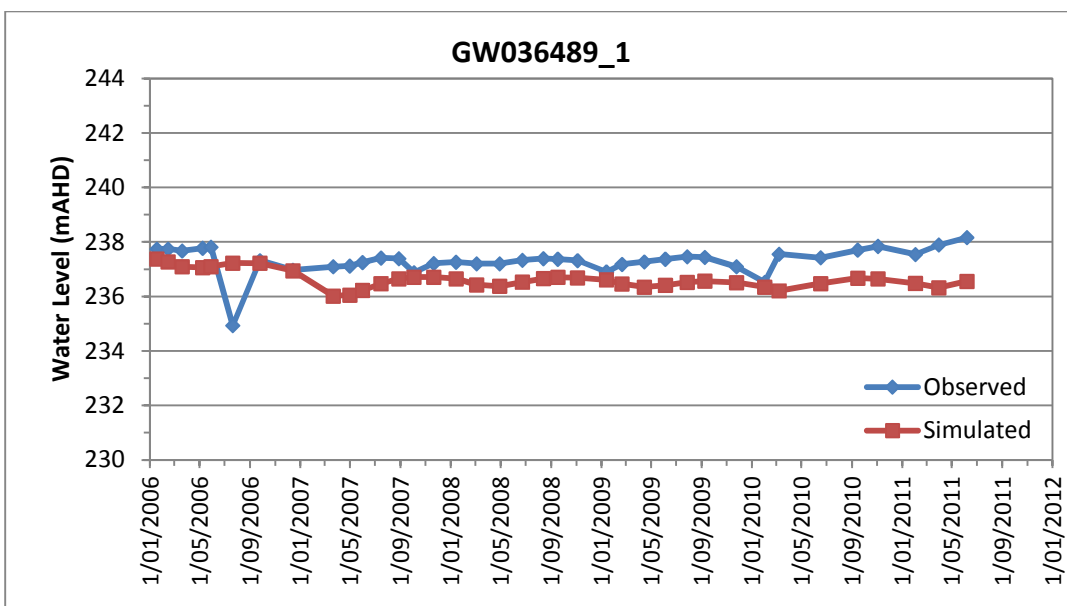
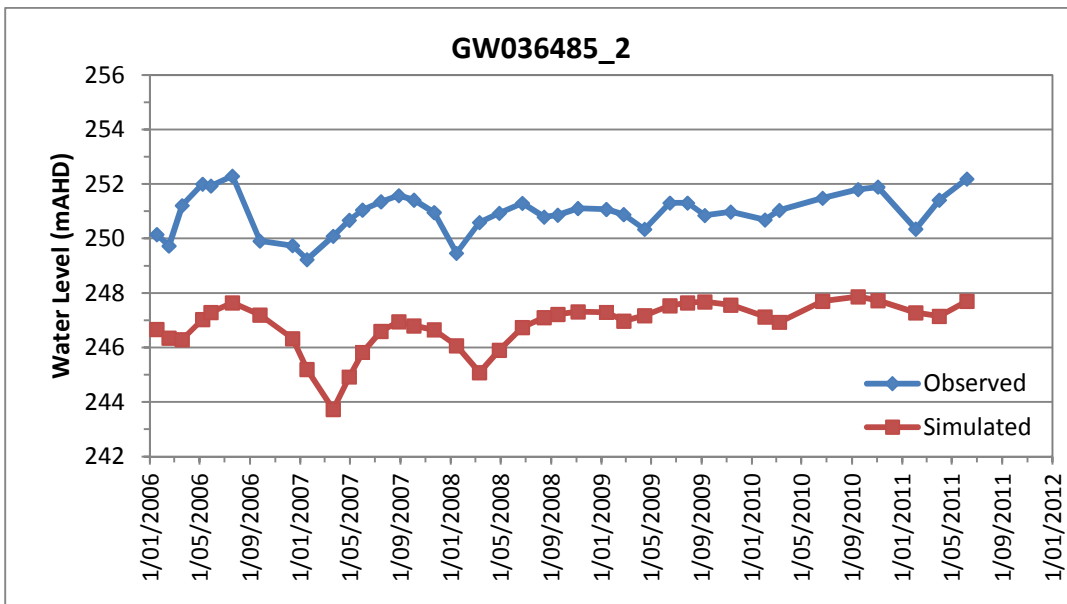
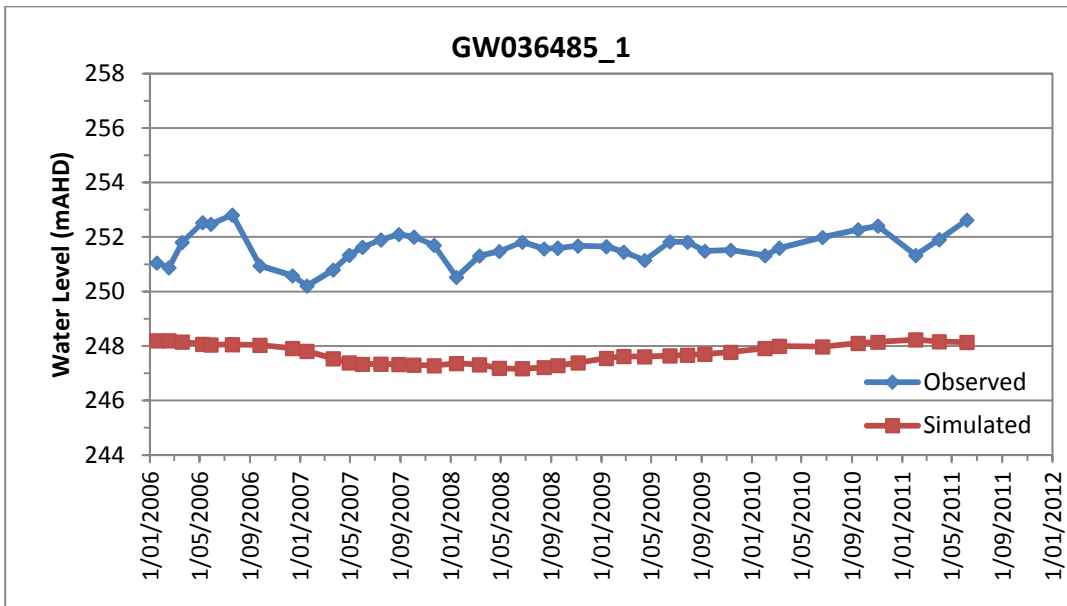


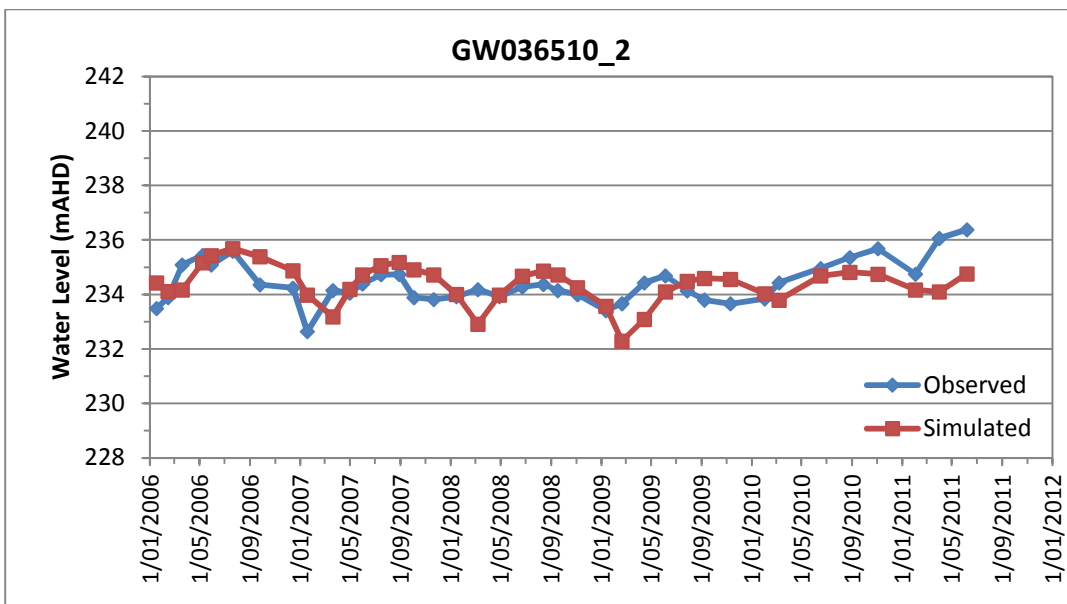
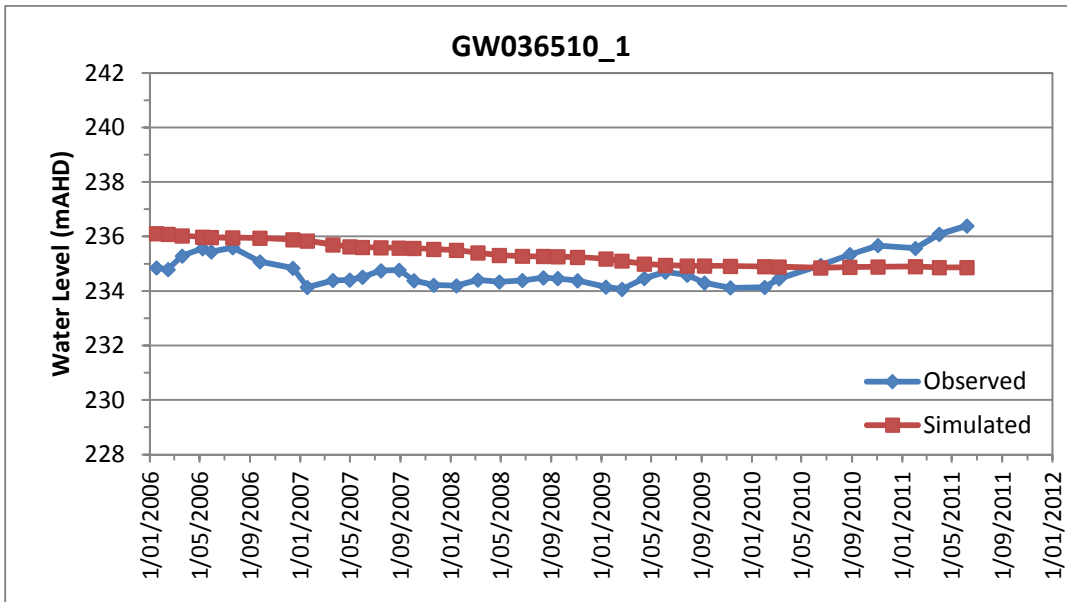
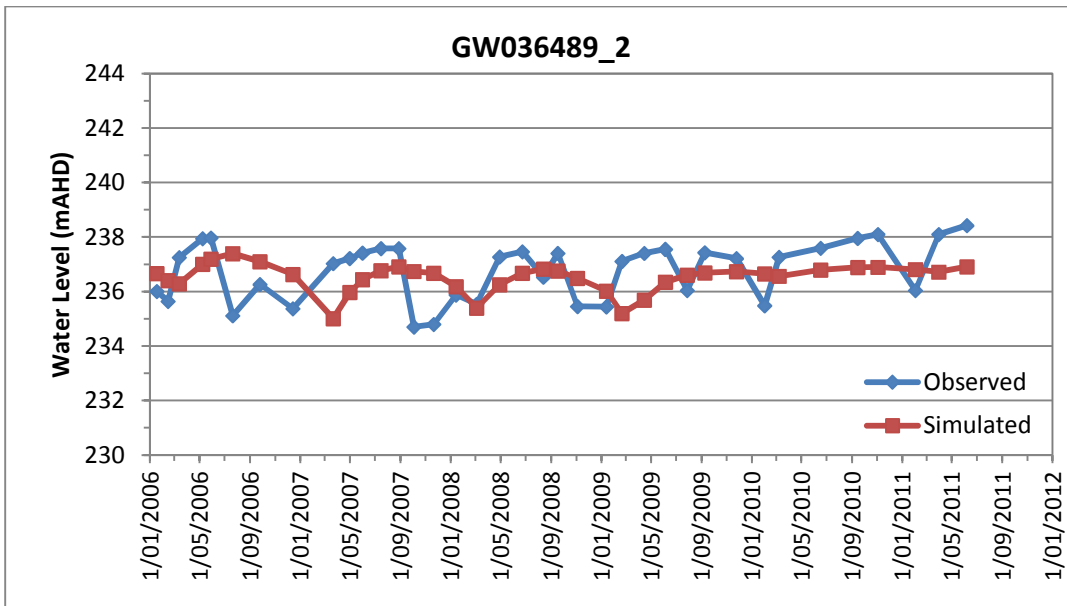


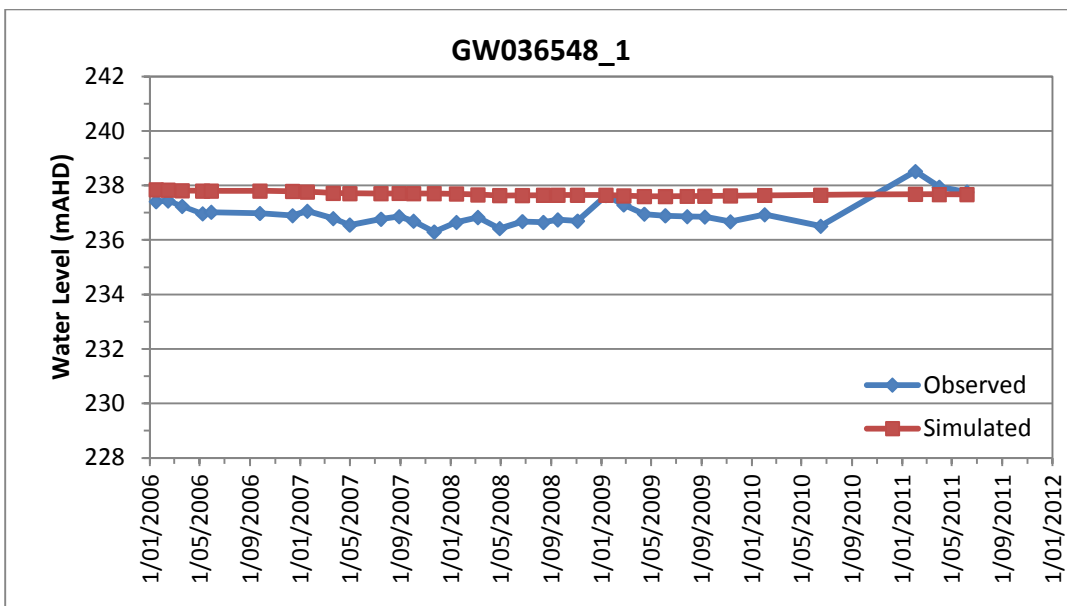
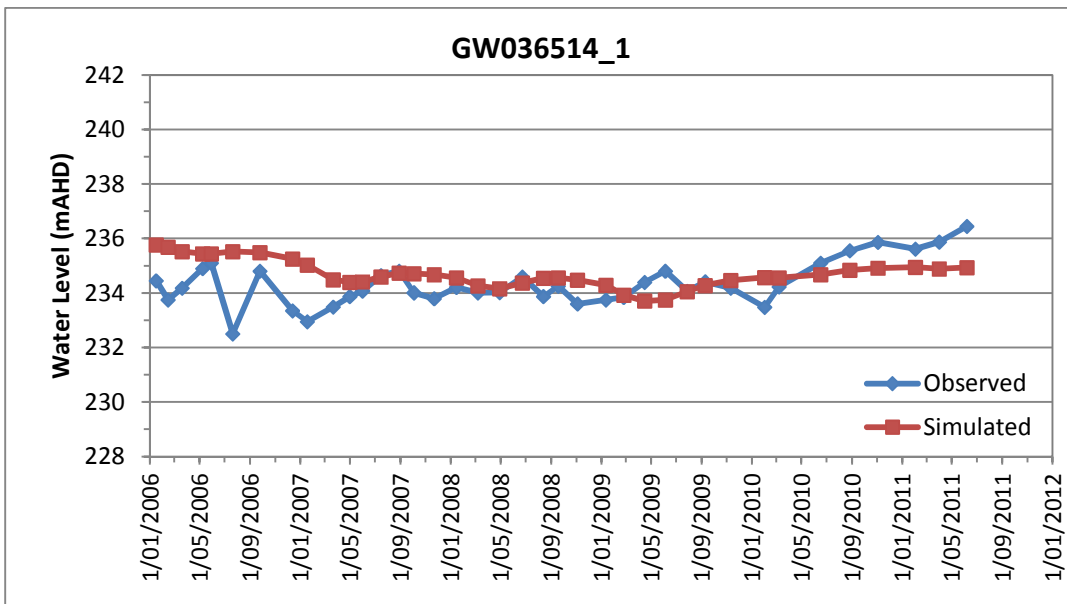
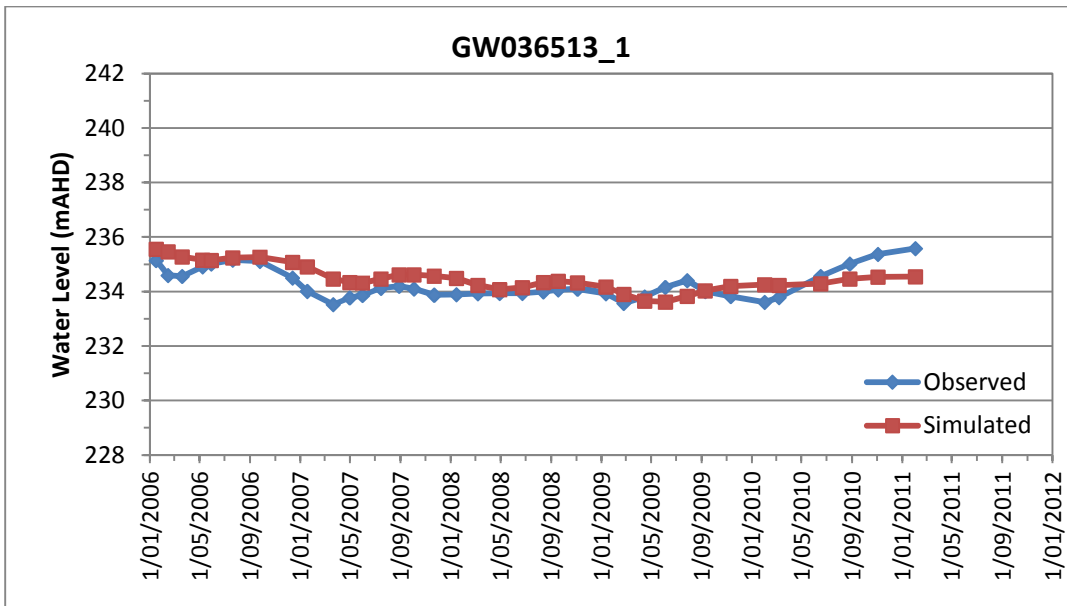


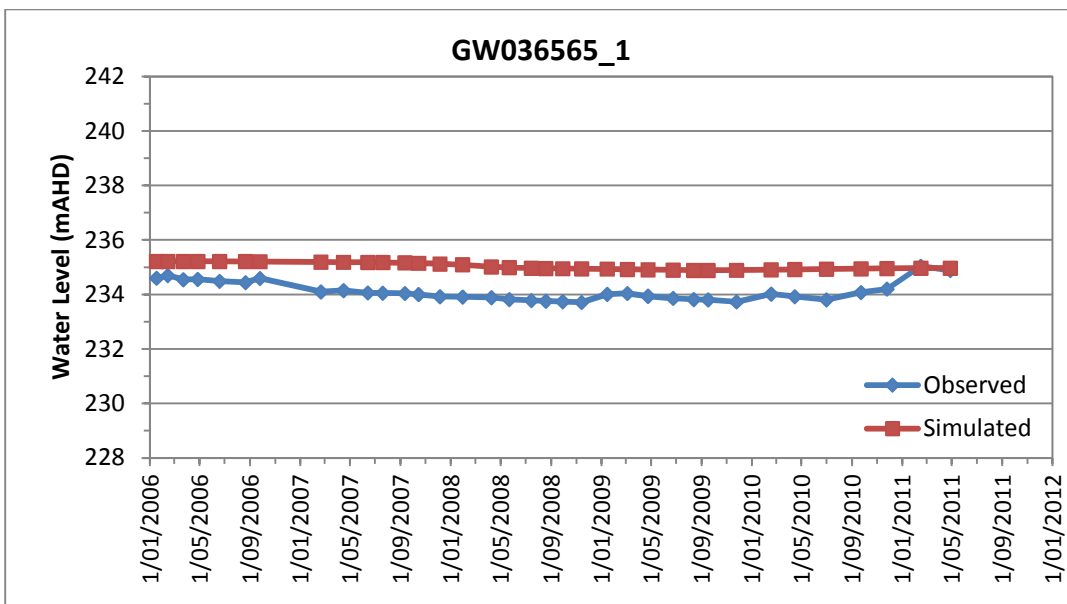
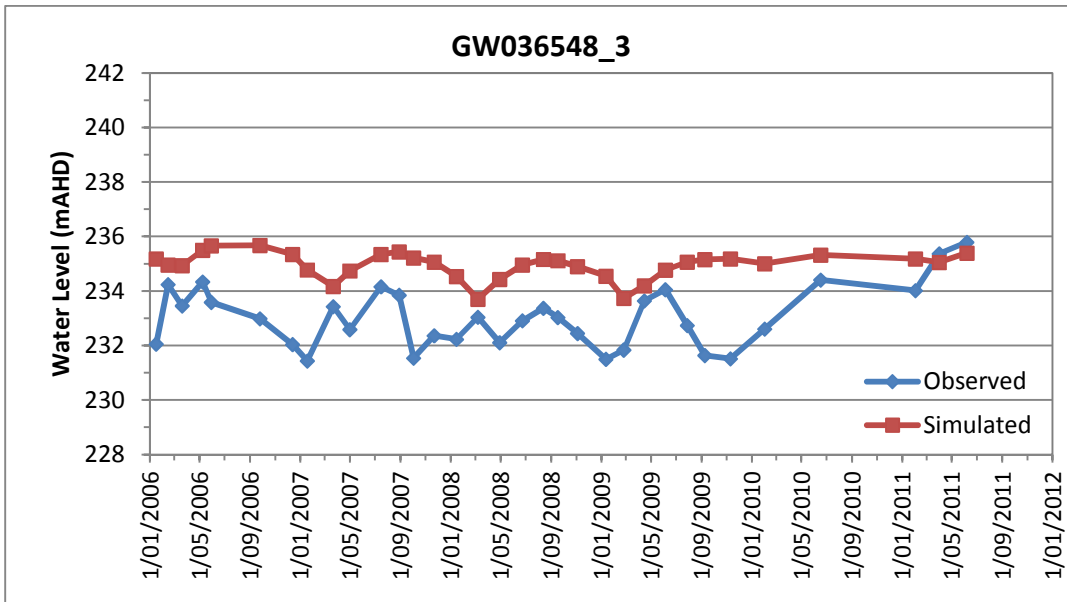
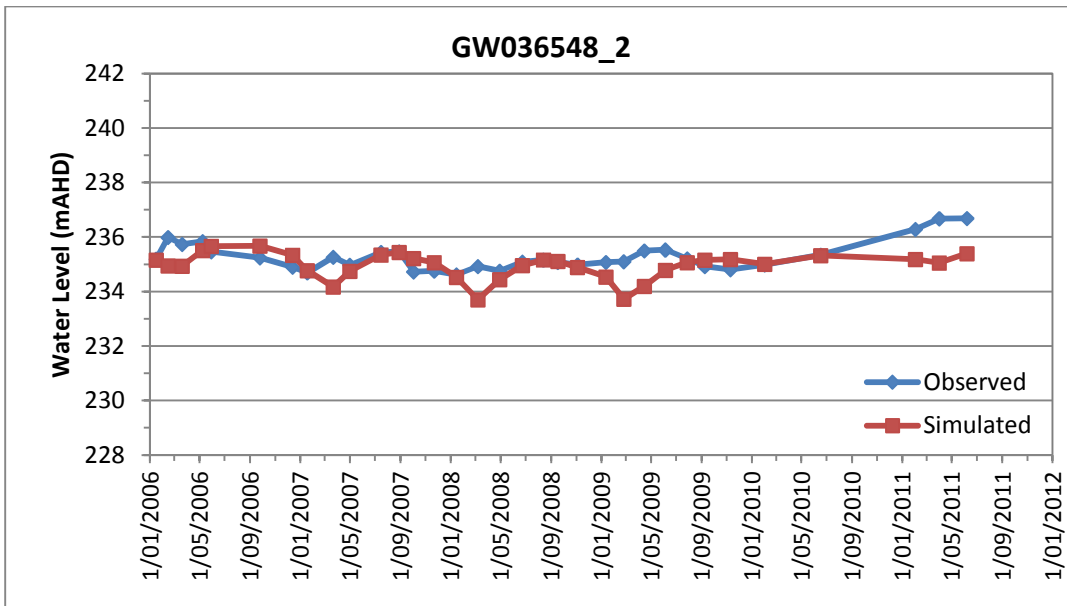


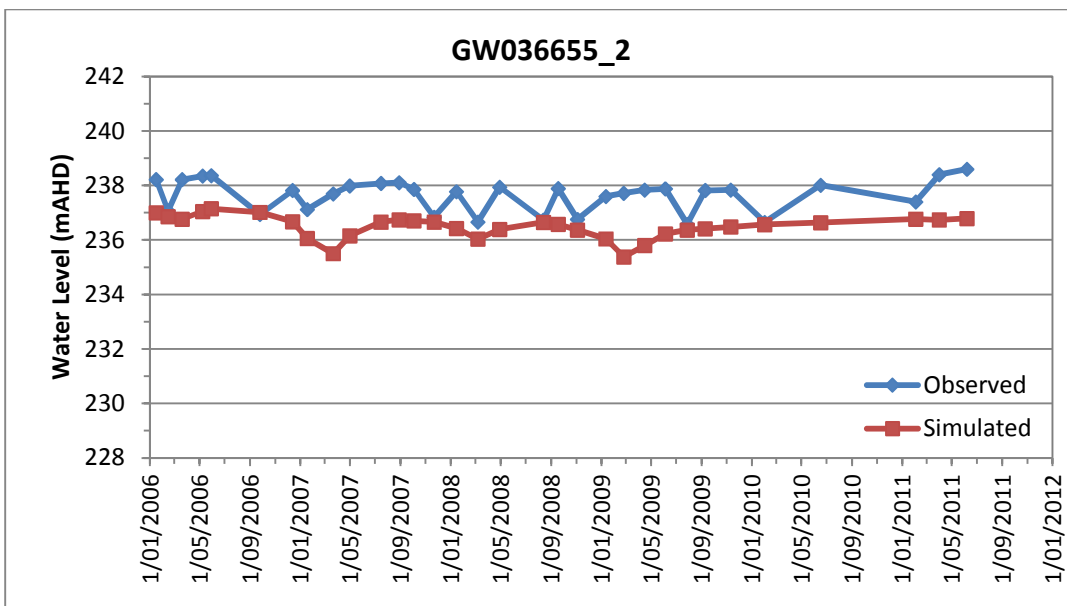
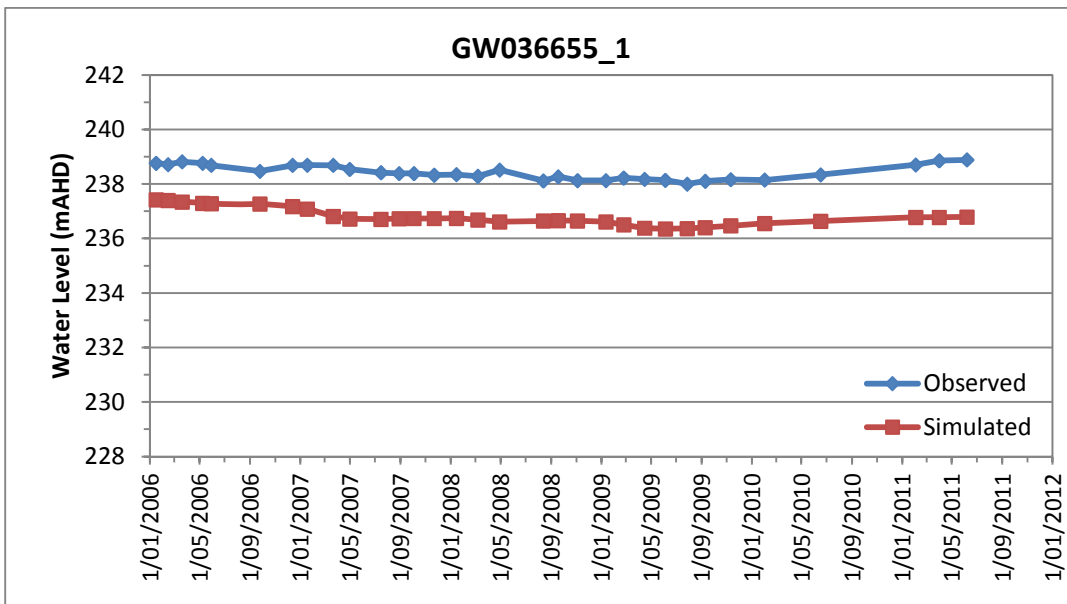
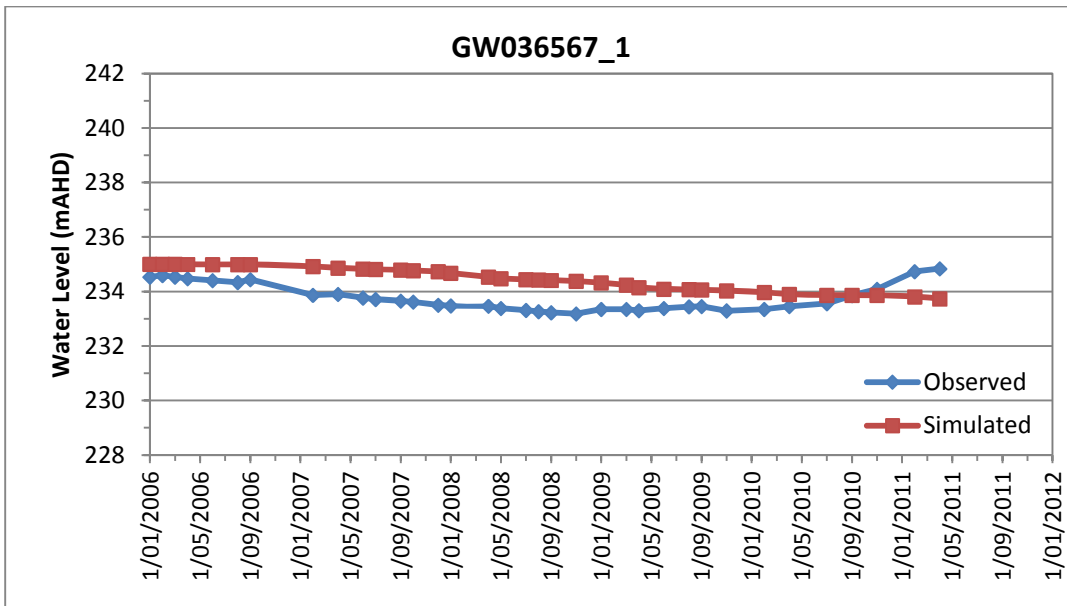












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GW036433_1

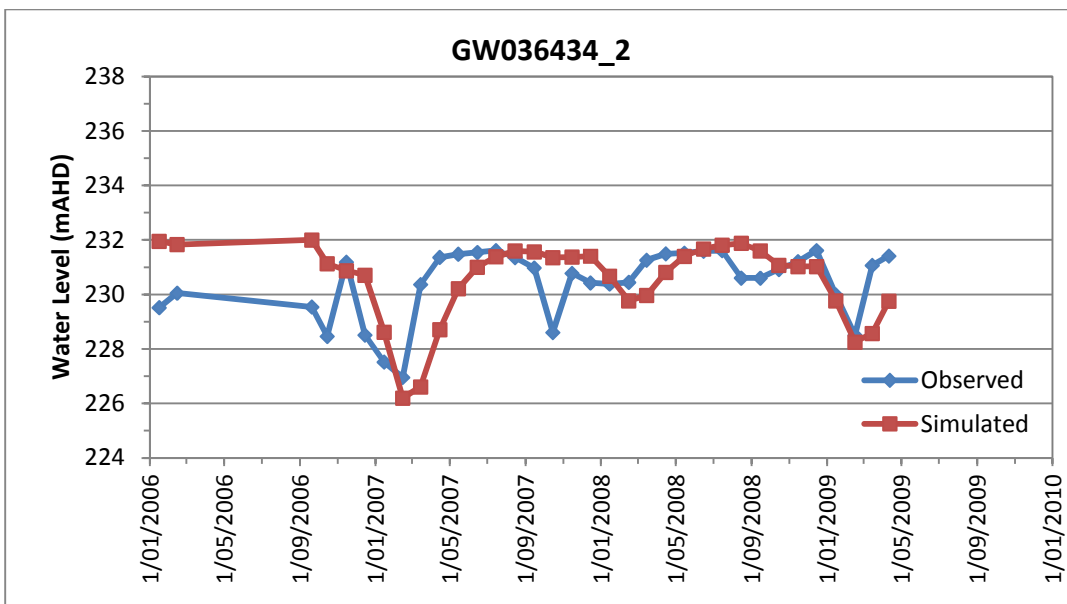
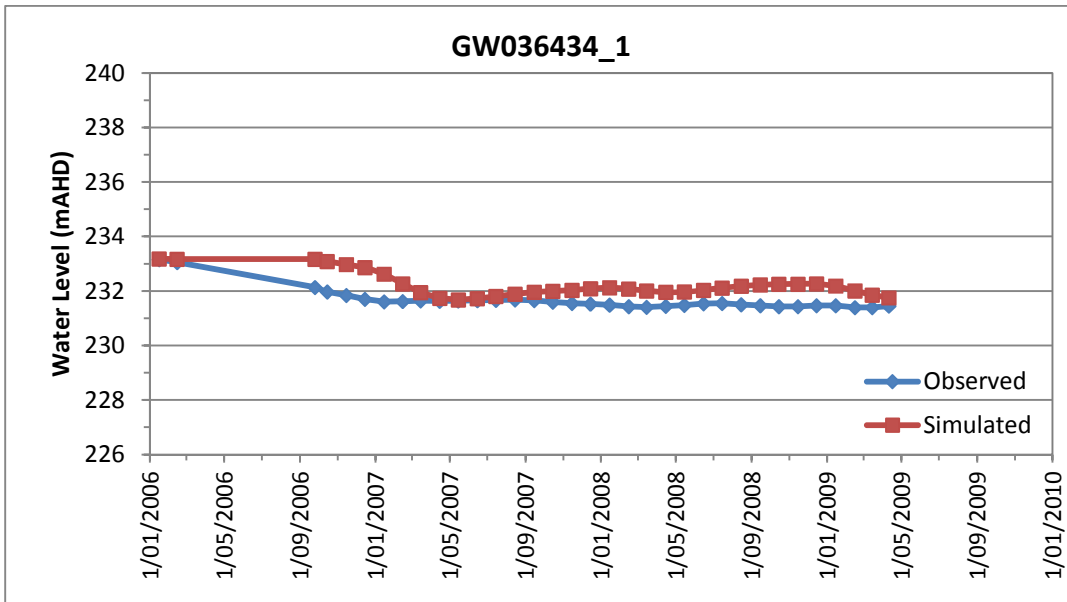
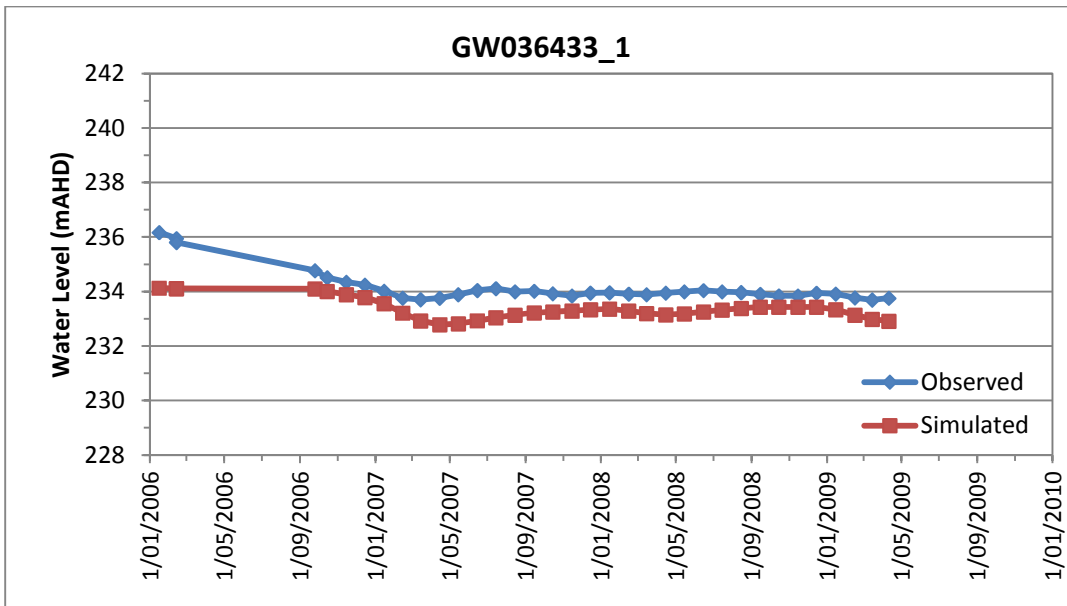
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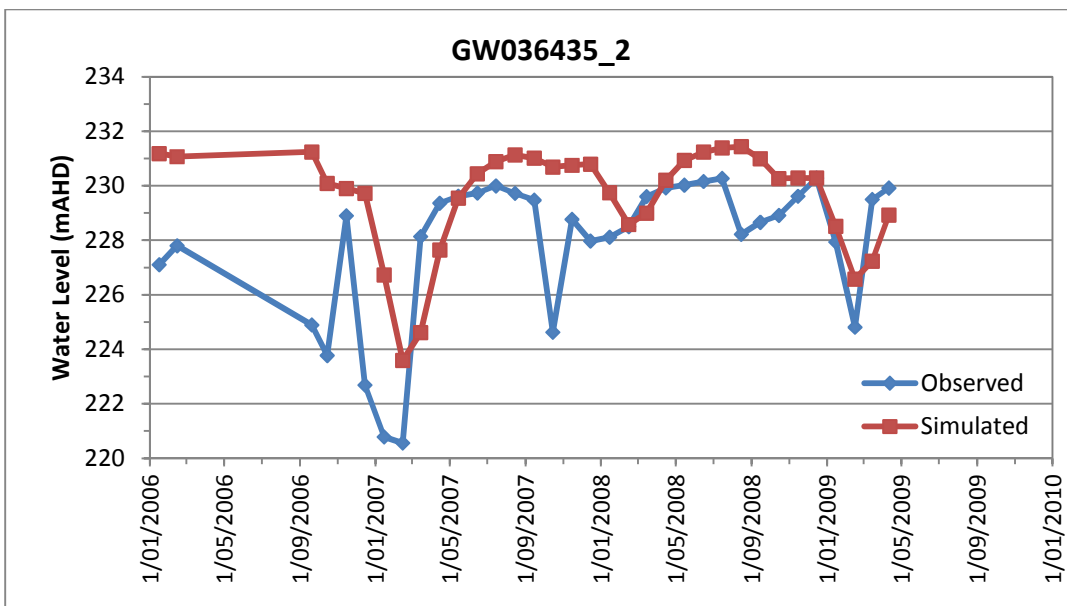
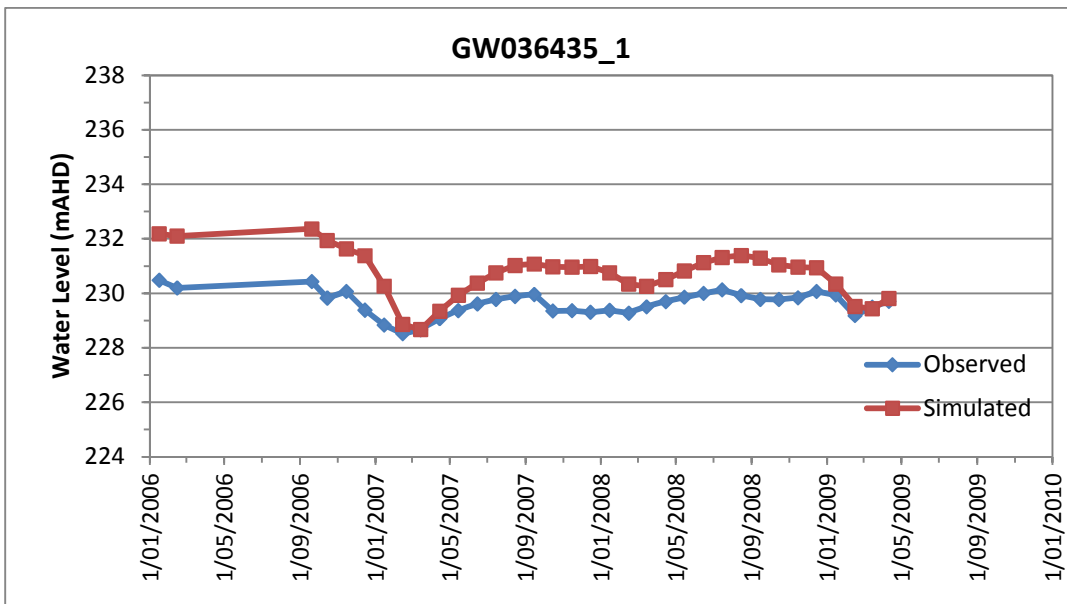
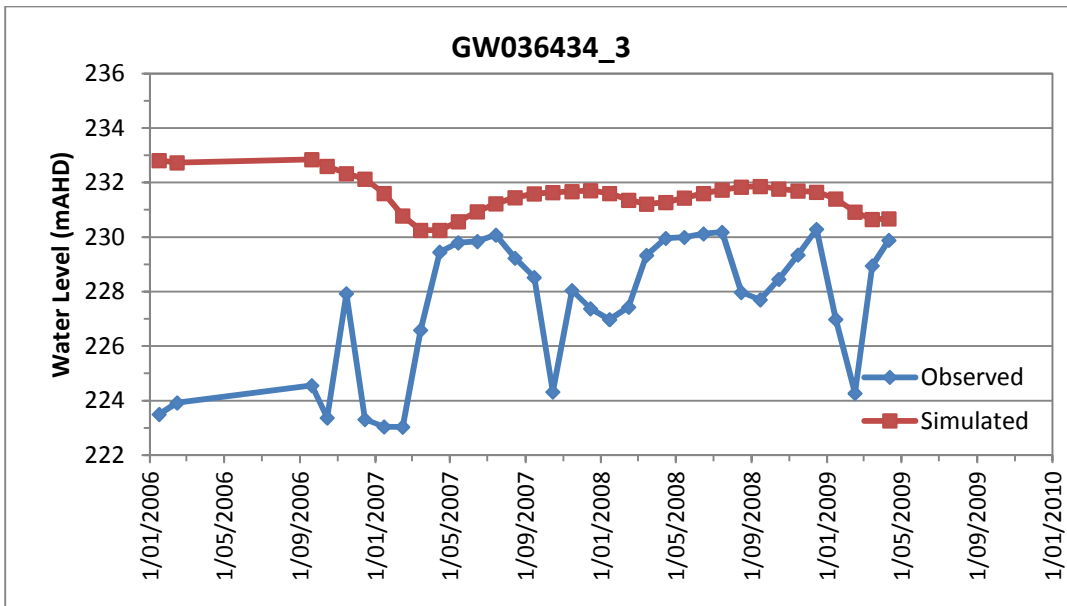
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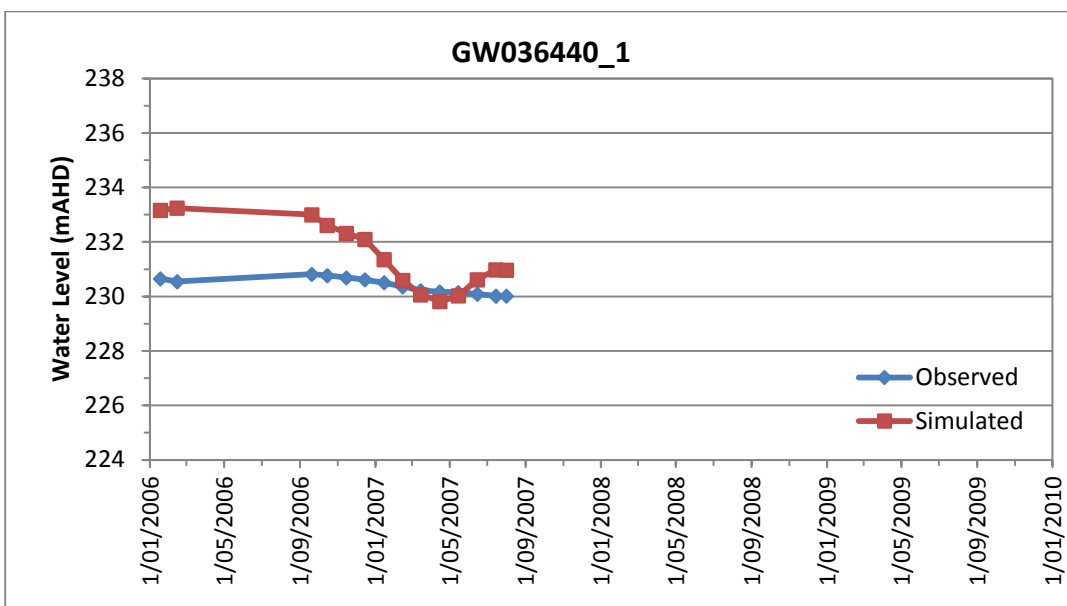
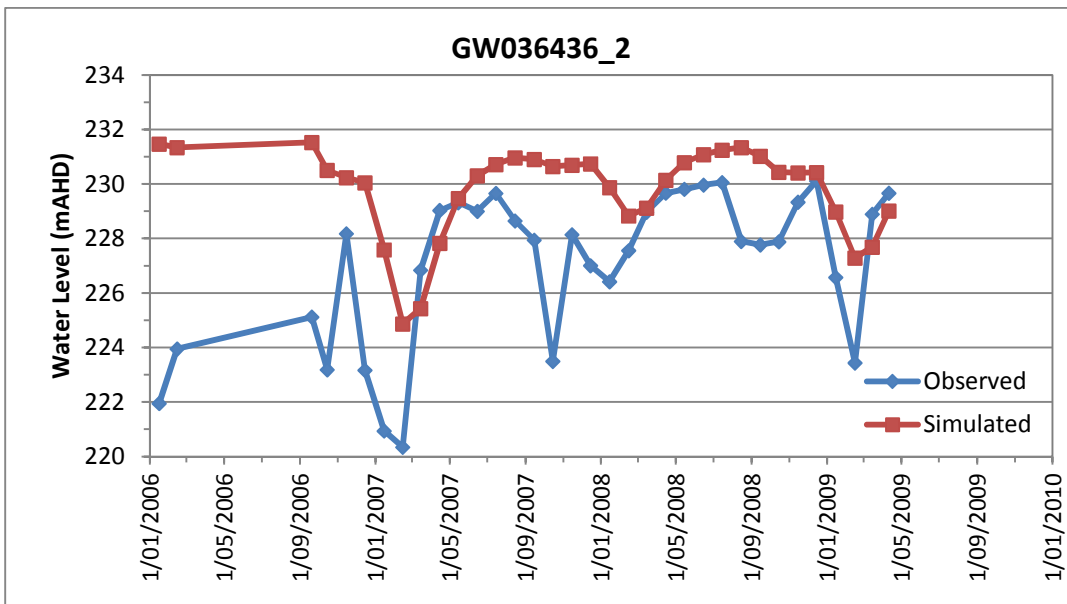
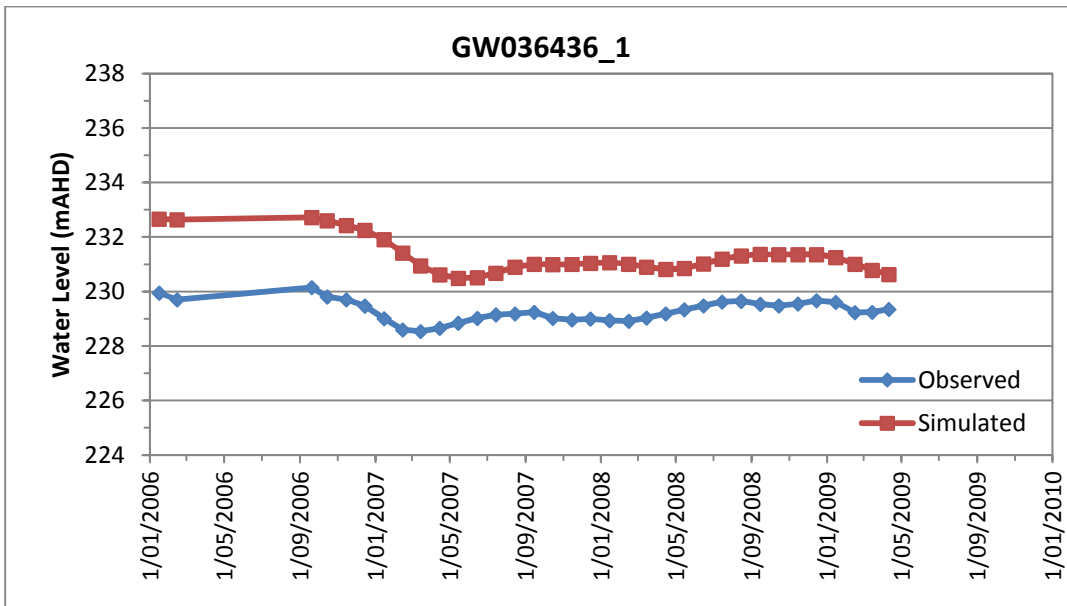
GW036436_1, GW036436_2

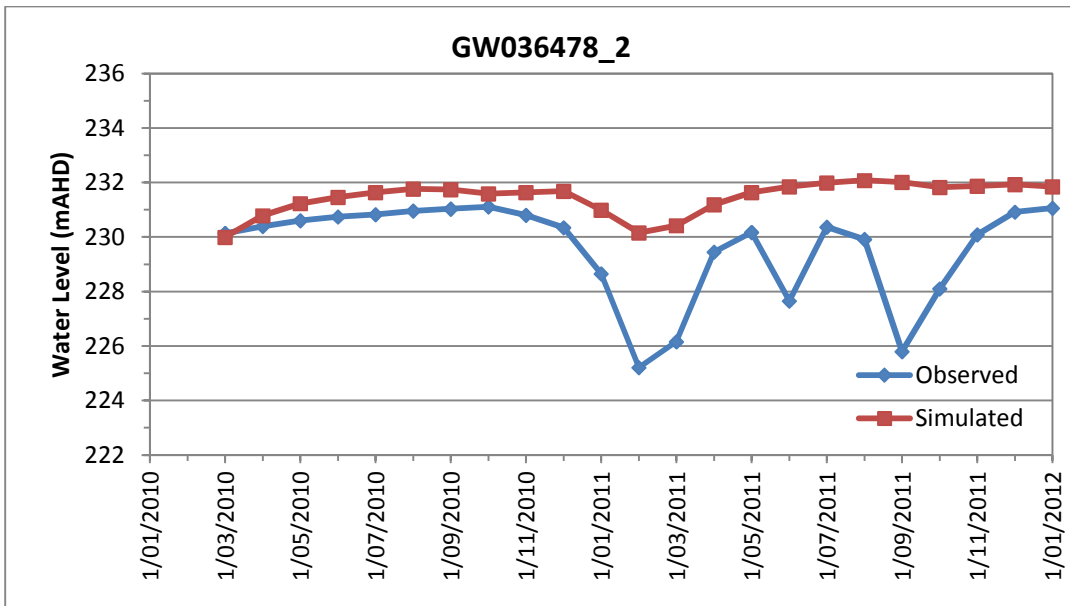
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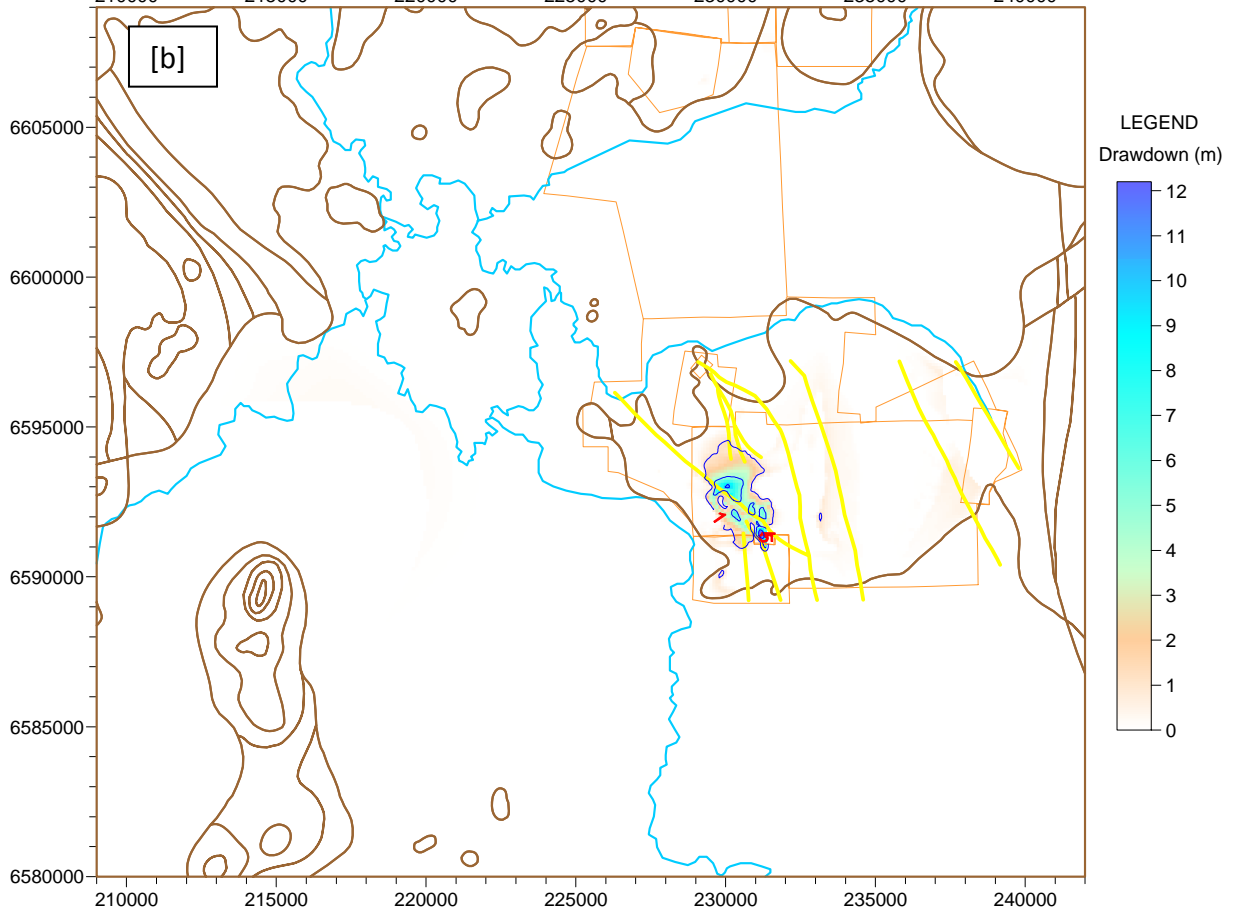
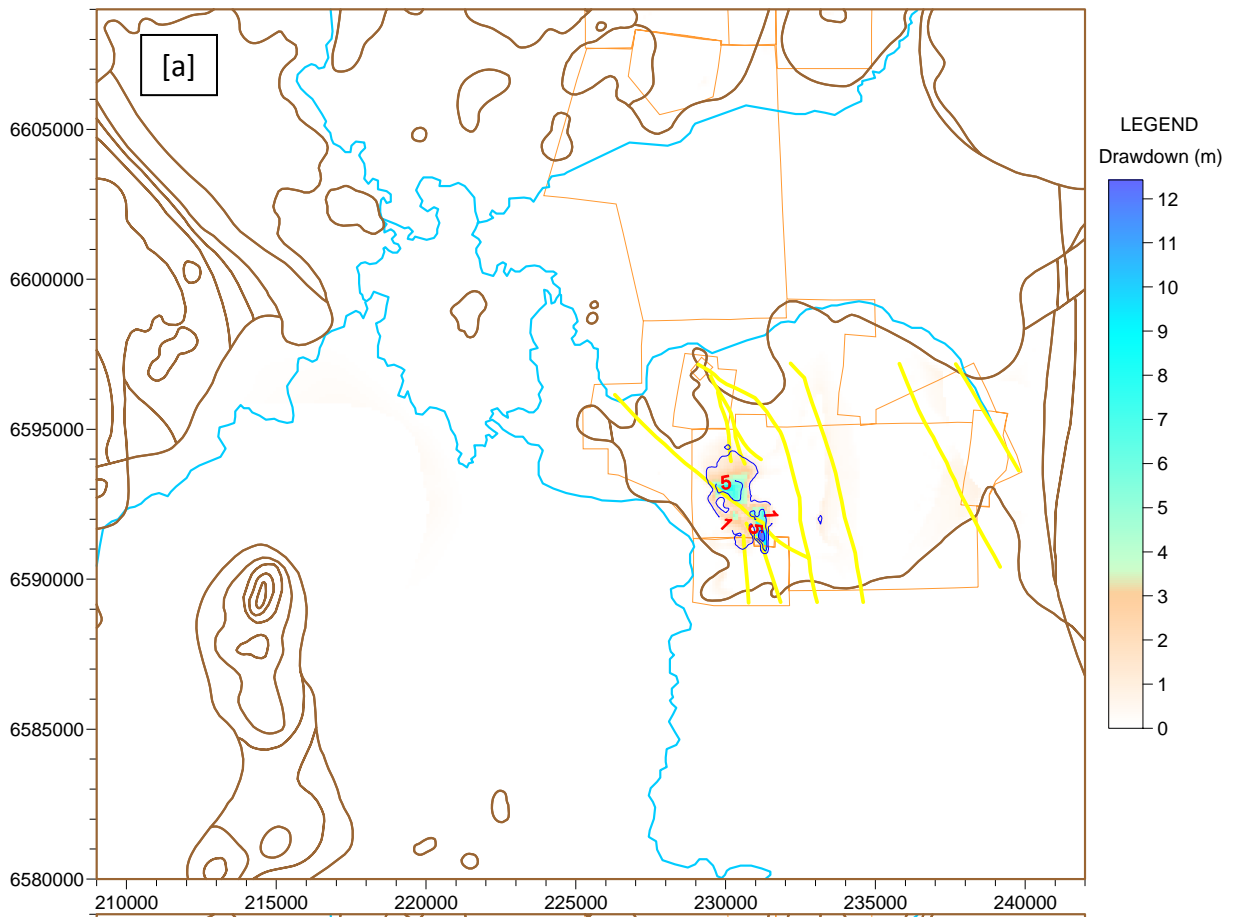




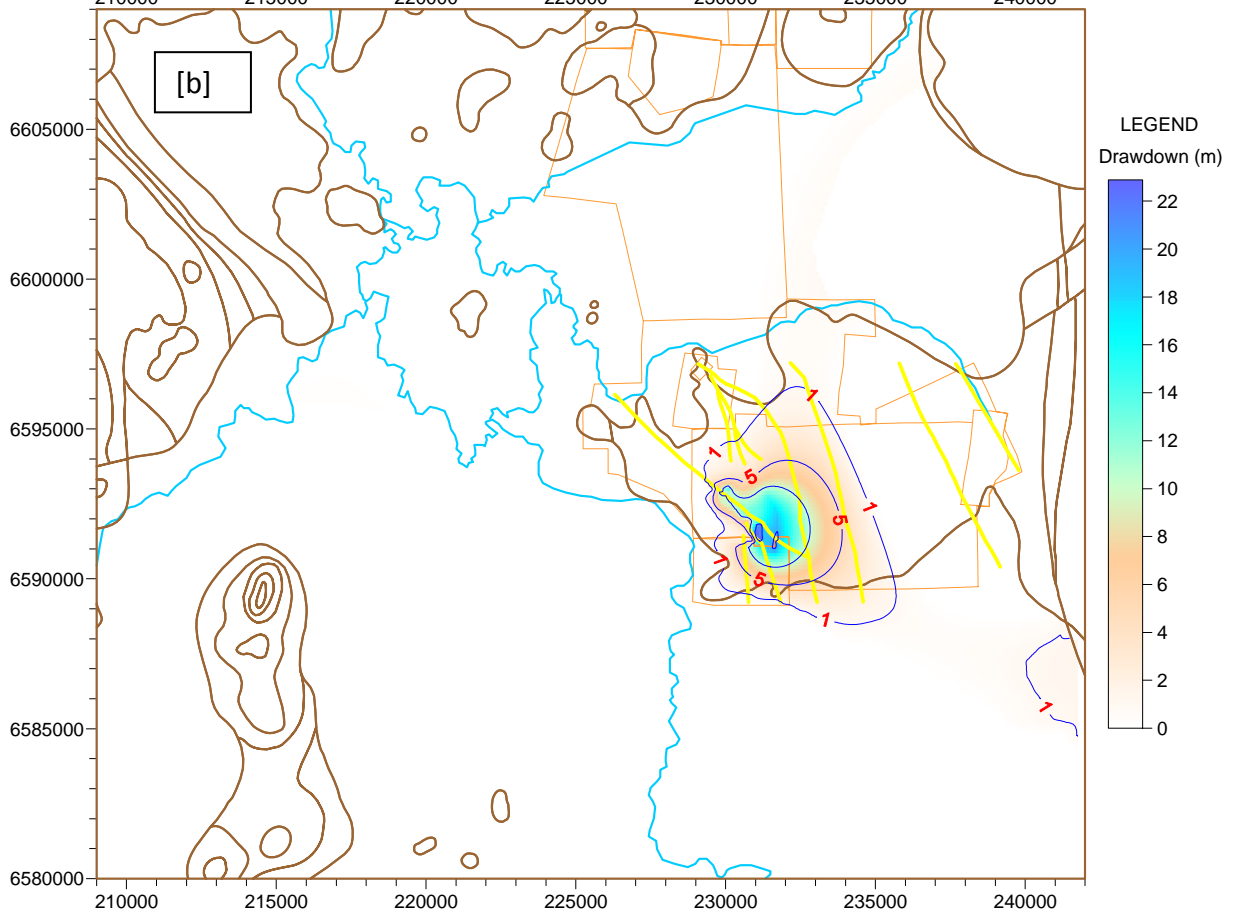
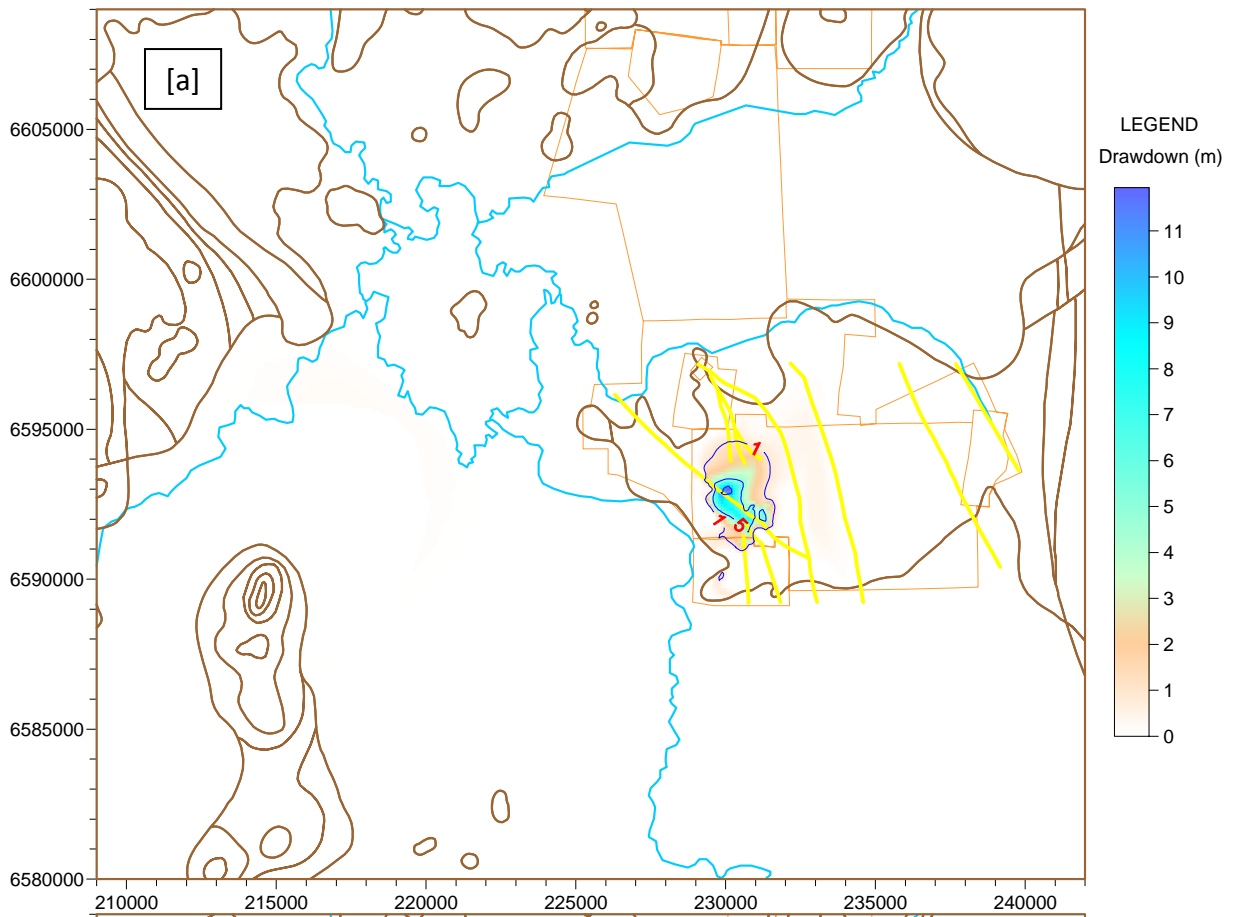


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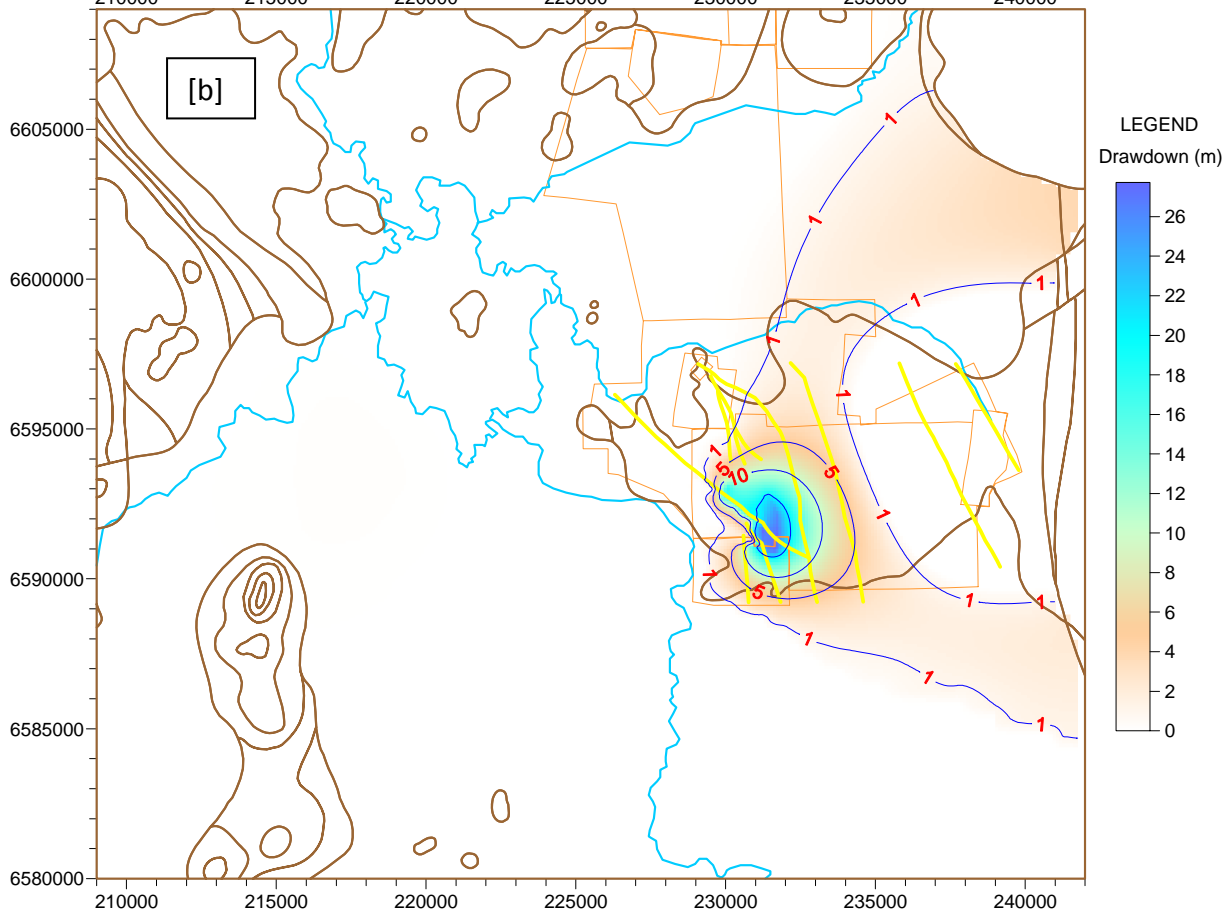
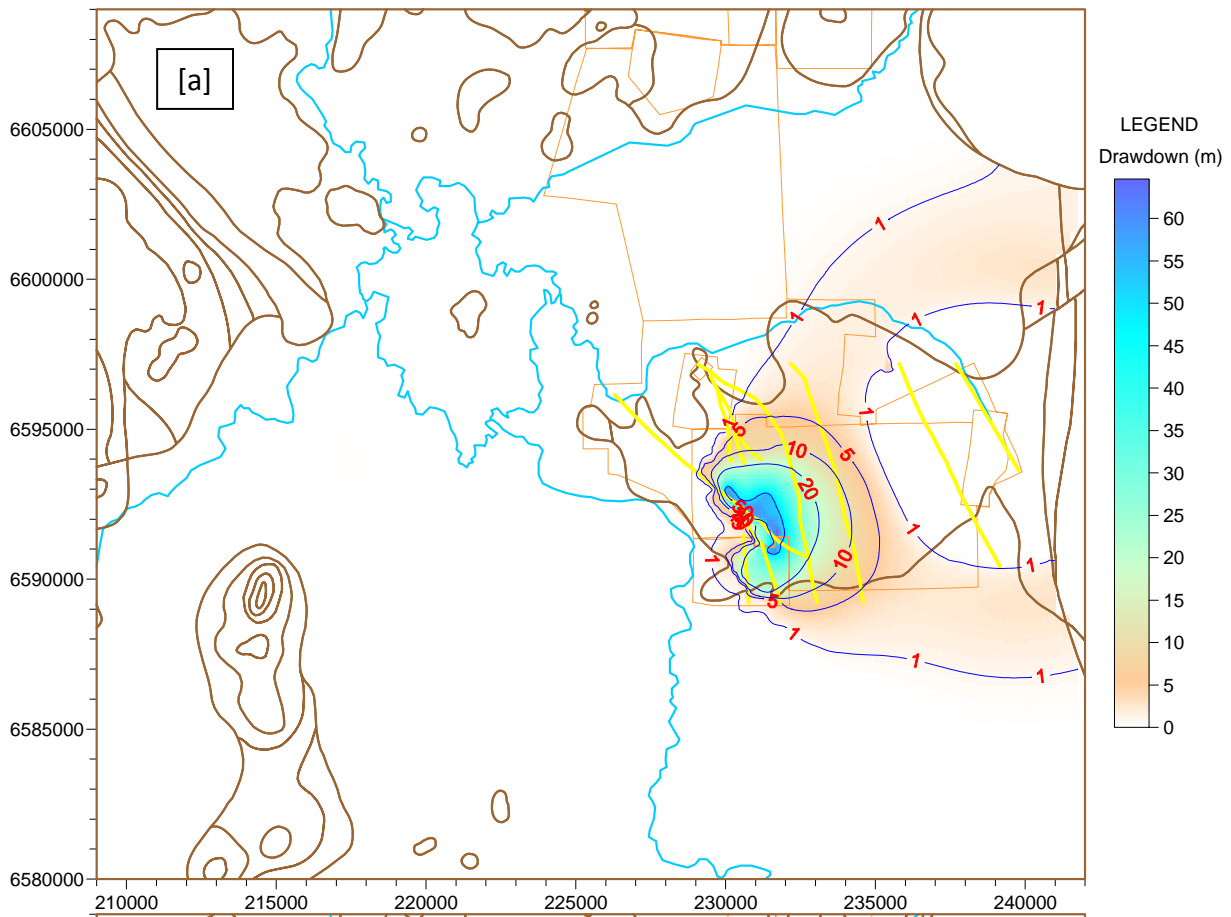
Five-Yearly Drawdown Contour Maps



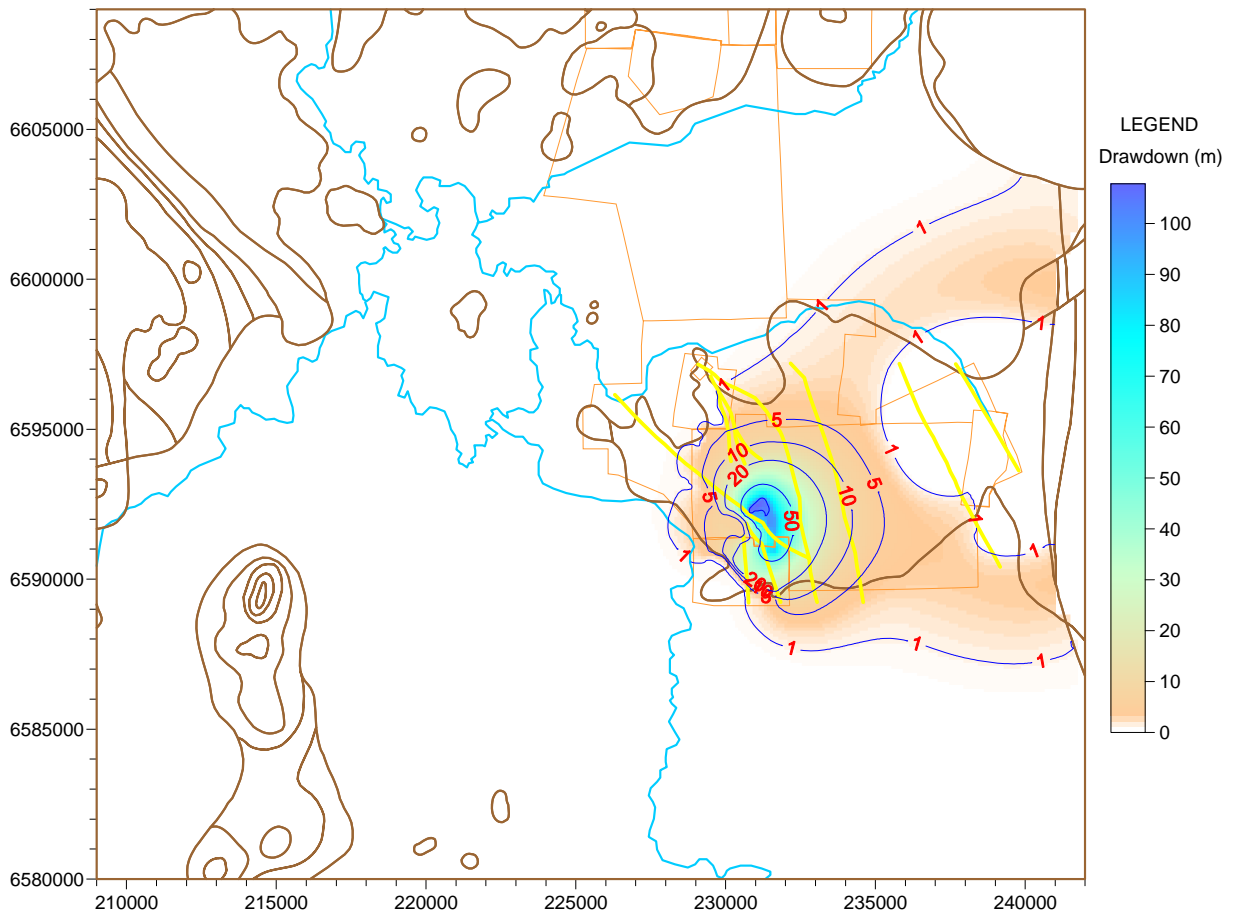
Predicted Drawdown Contours at the of End of Stress Period 5 (End of Year 2016) in Layers [a] 1 and [b] 2



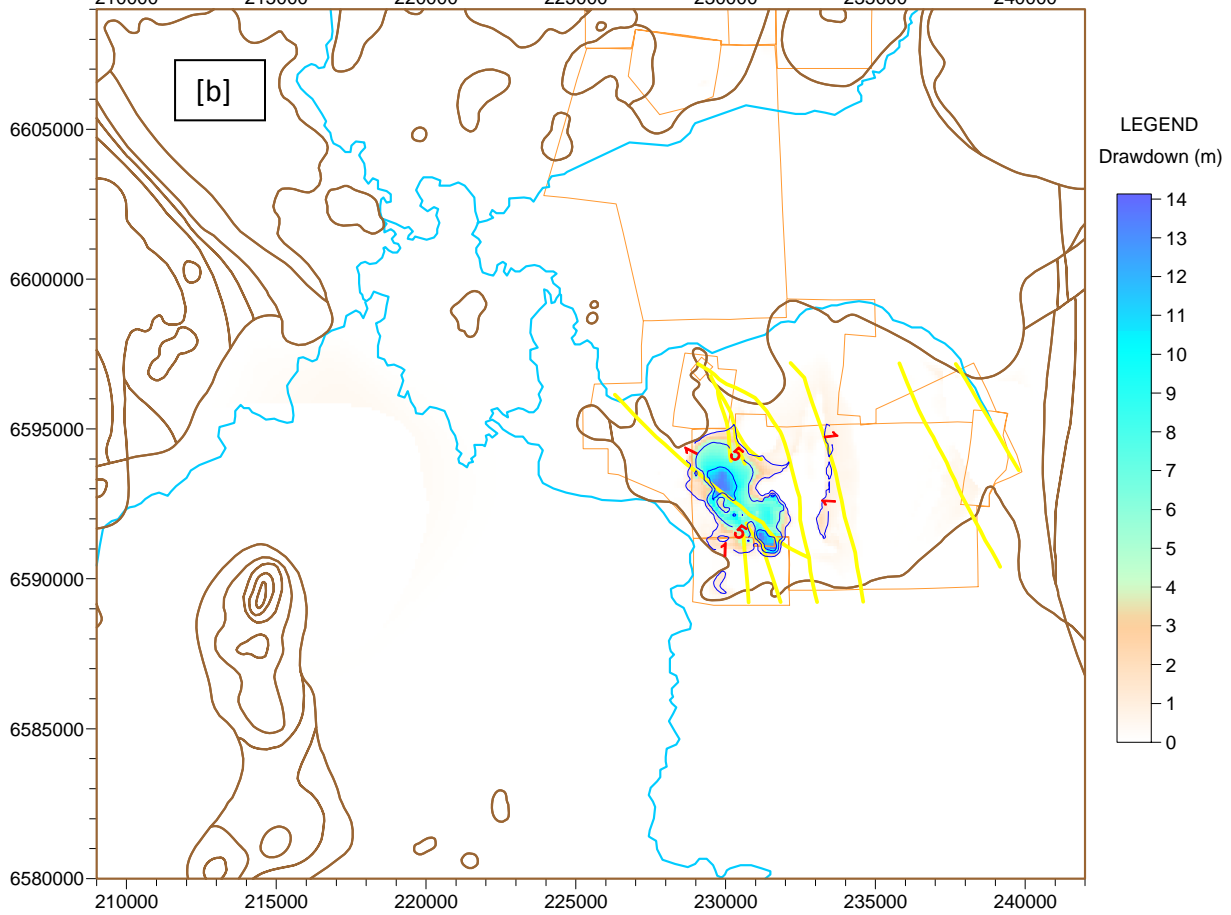
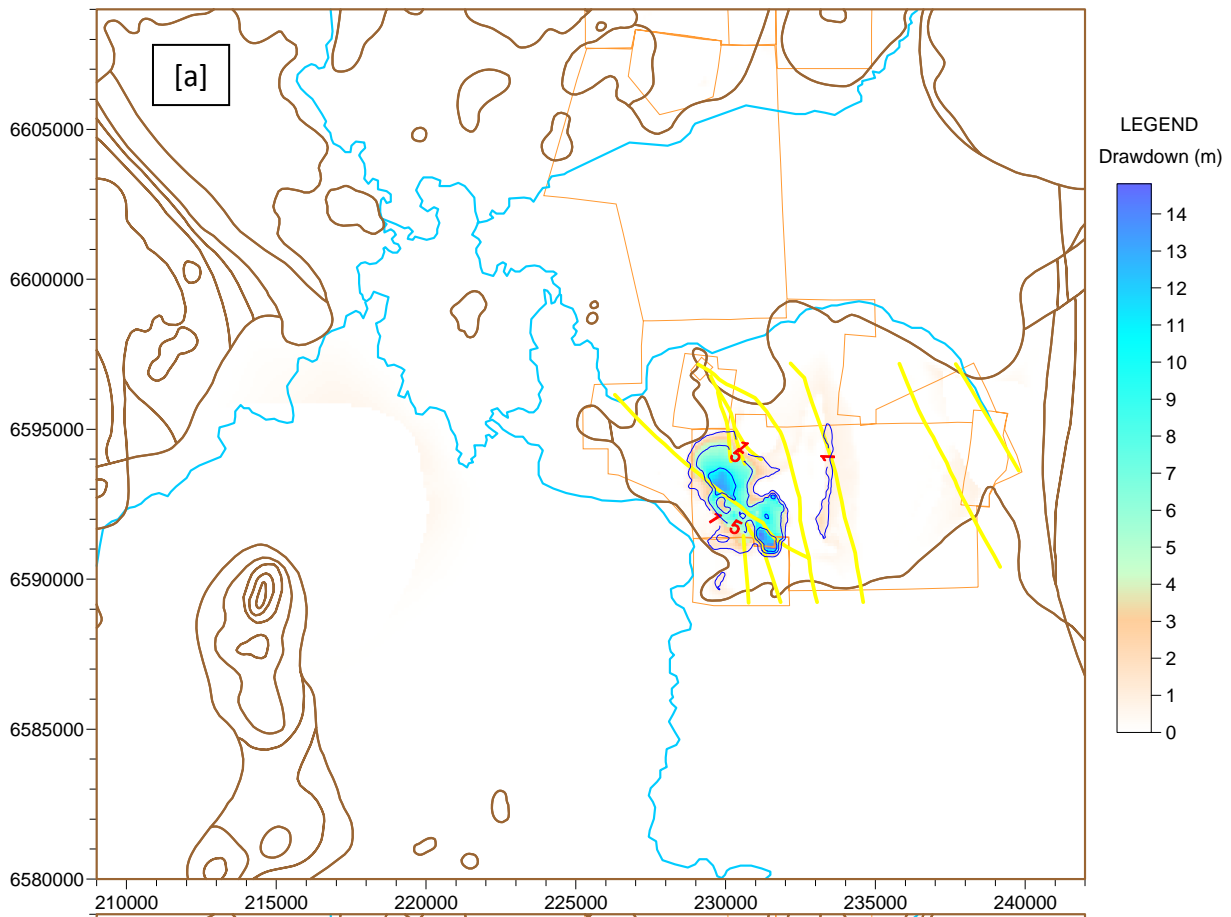
Predicted Drawdown Contours at the of End of Stress Period 5 (End of Year 2016) in Layers [a] 4 and [b] 6



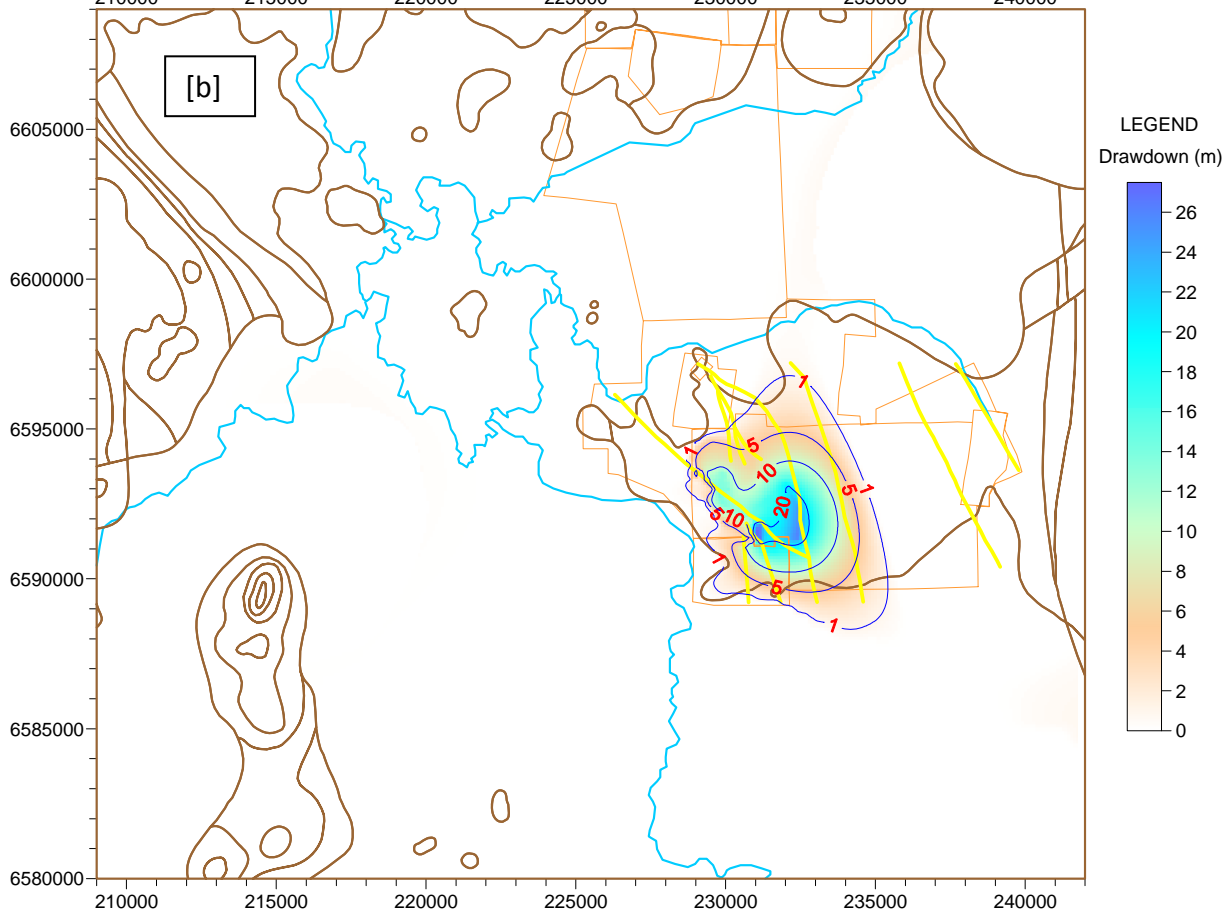
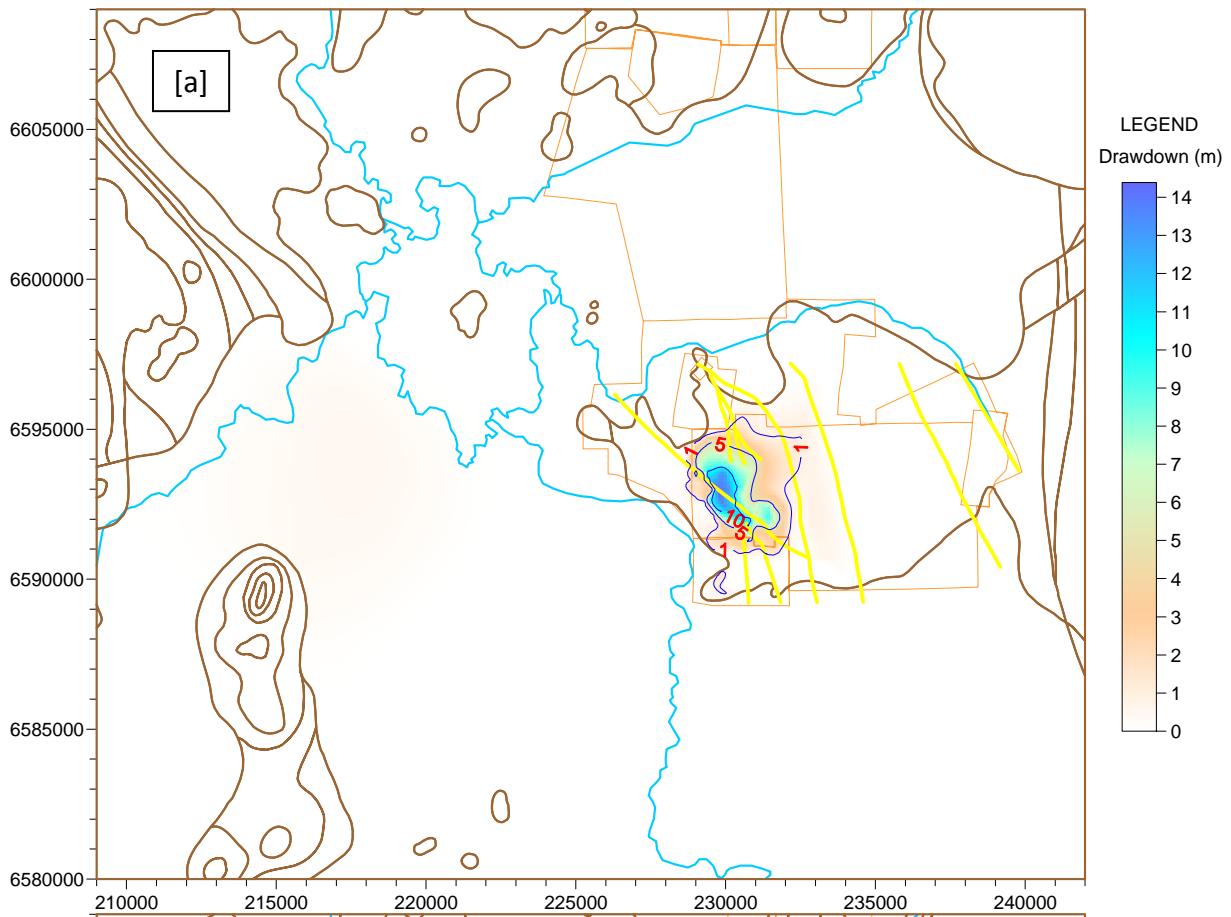
Predicted Drawdown Contours at the of End of Stress Period 5 (End of Year 2016) in Layers [a] 8 and [b] 10



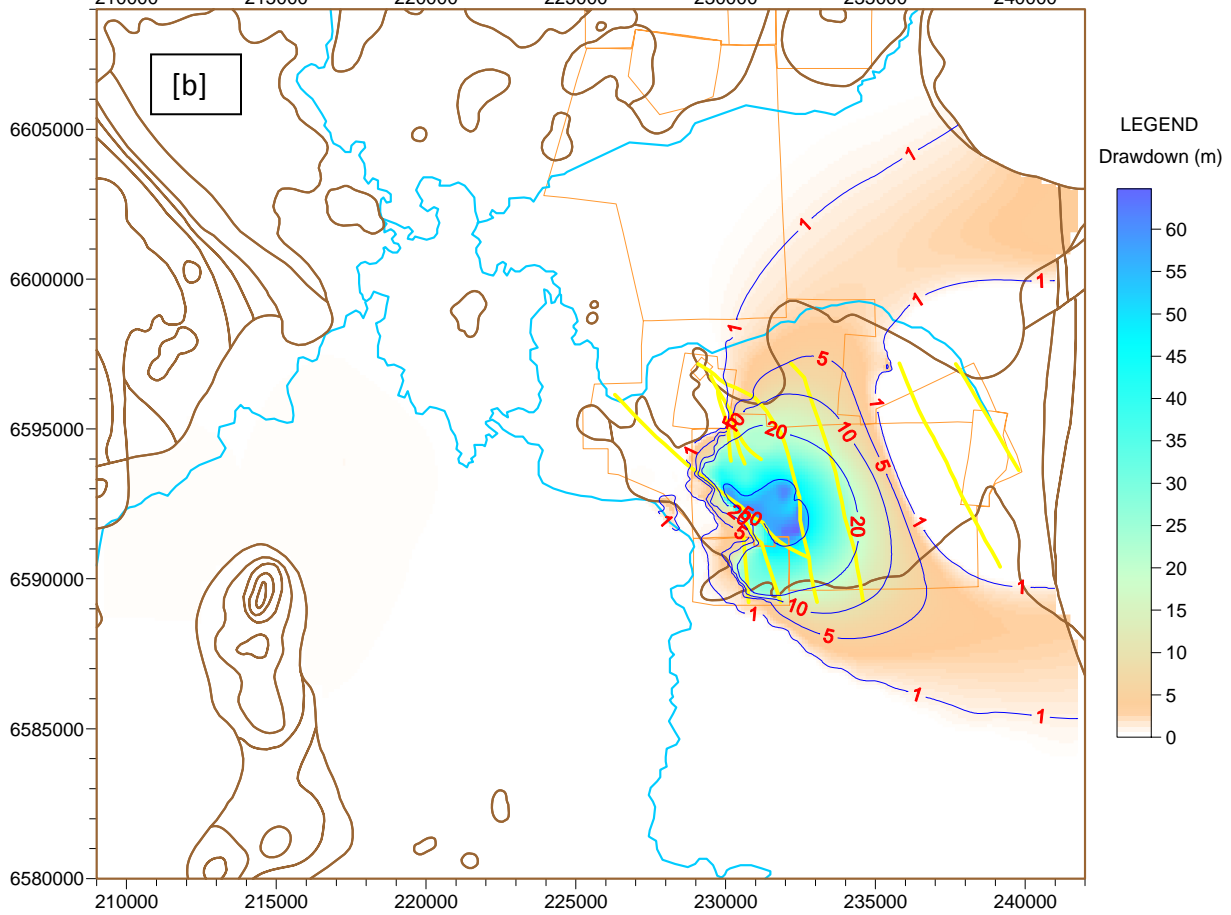
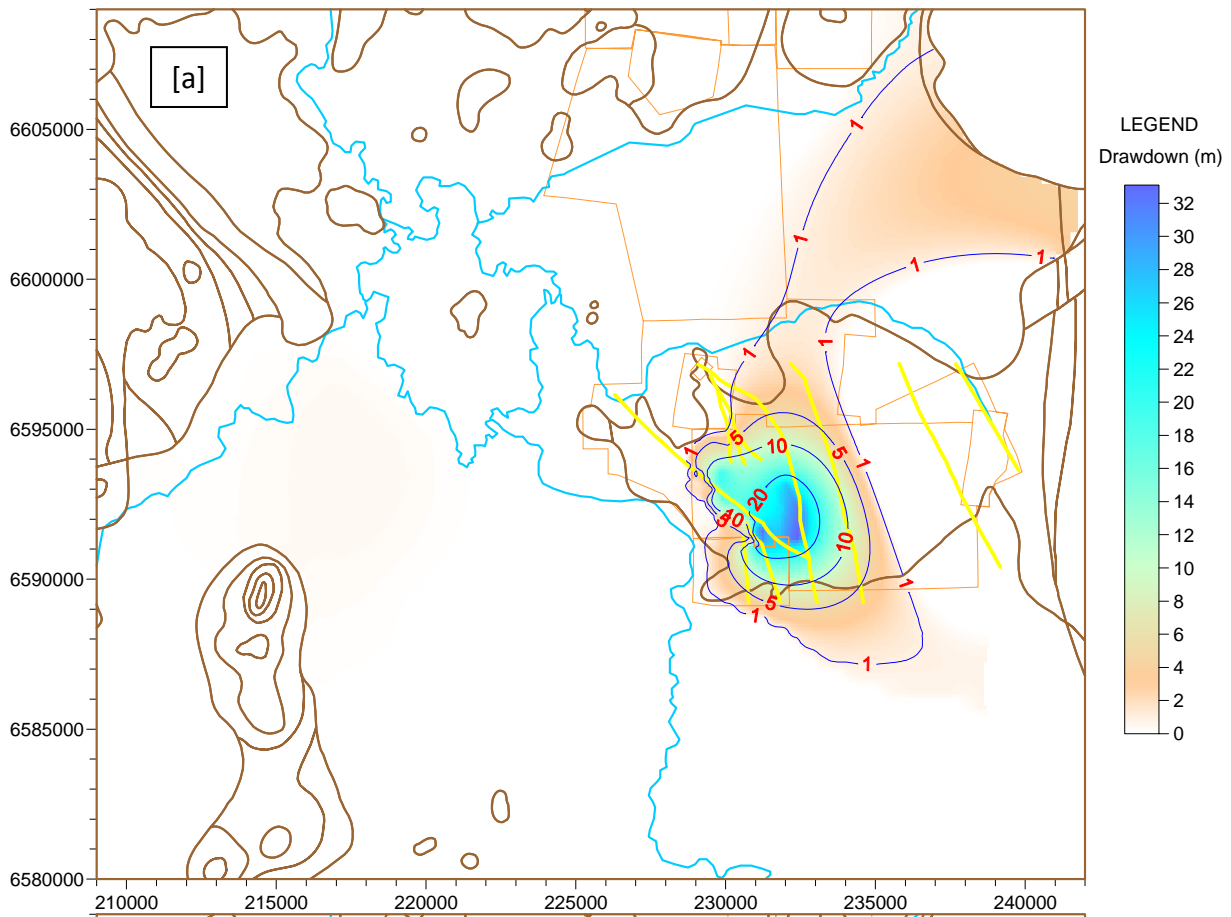
Predicted Drawdown Contours at the of End of Stress Period 5 (End of Year 2016) in Layer 12



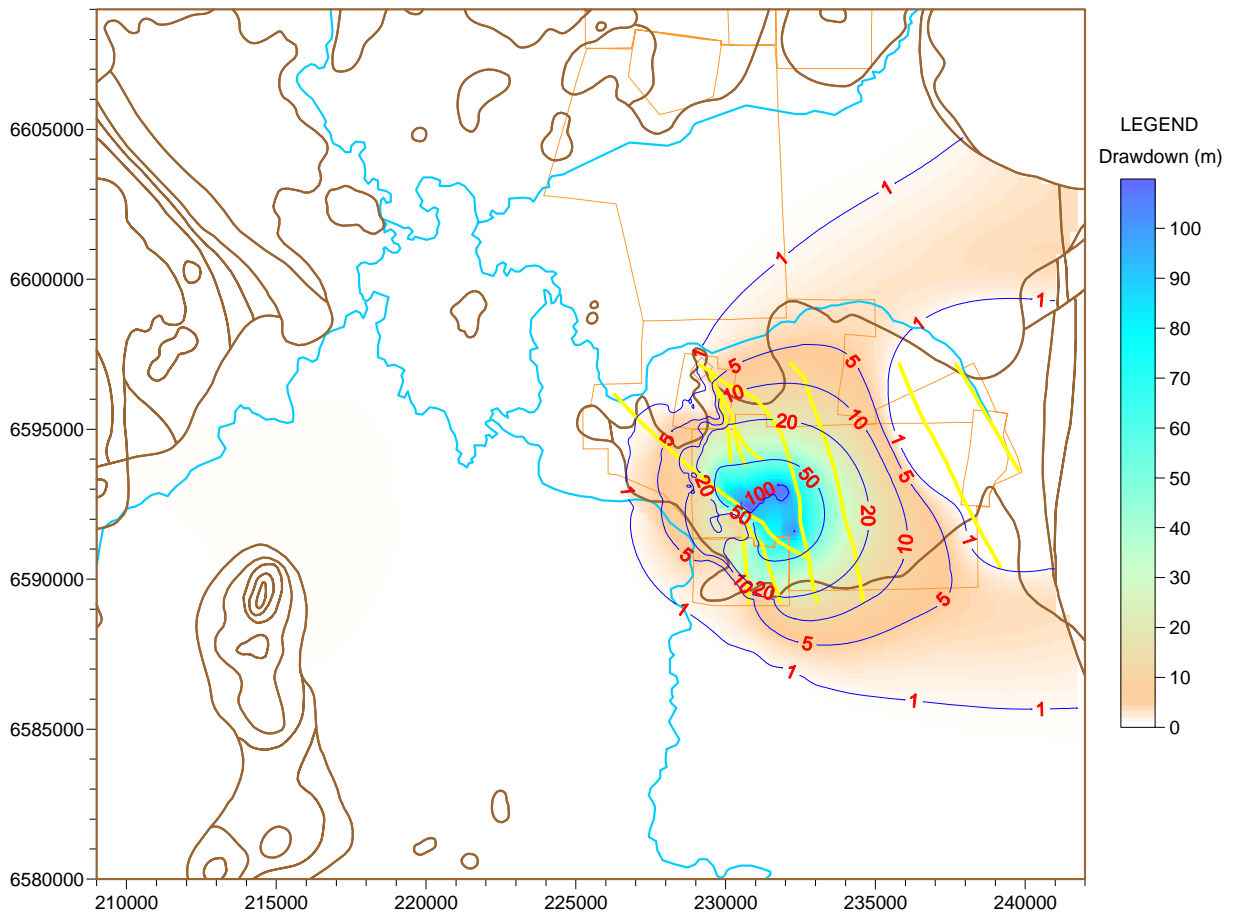
Predicted Drawdown Contours at the of End of Stress Period 10 (End of Year 2021) in Layers [a] 1 and [b] 2



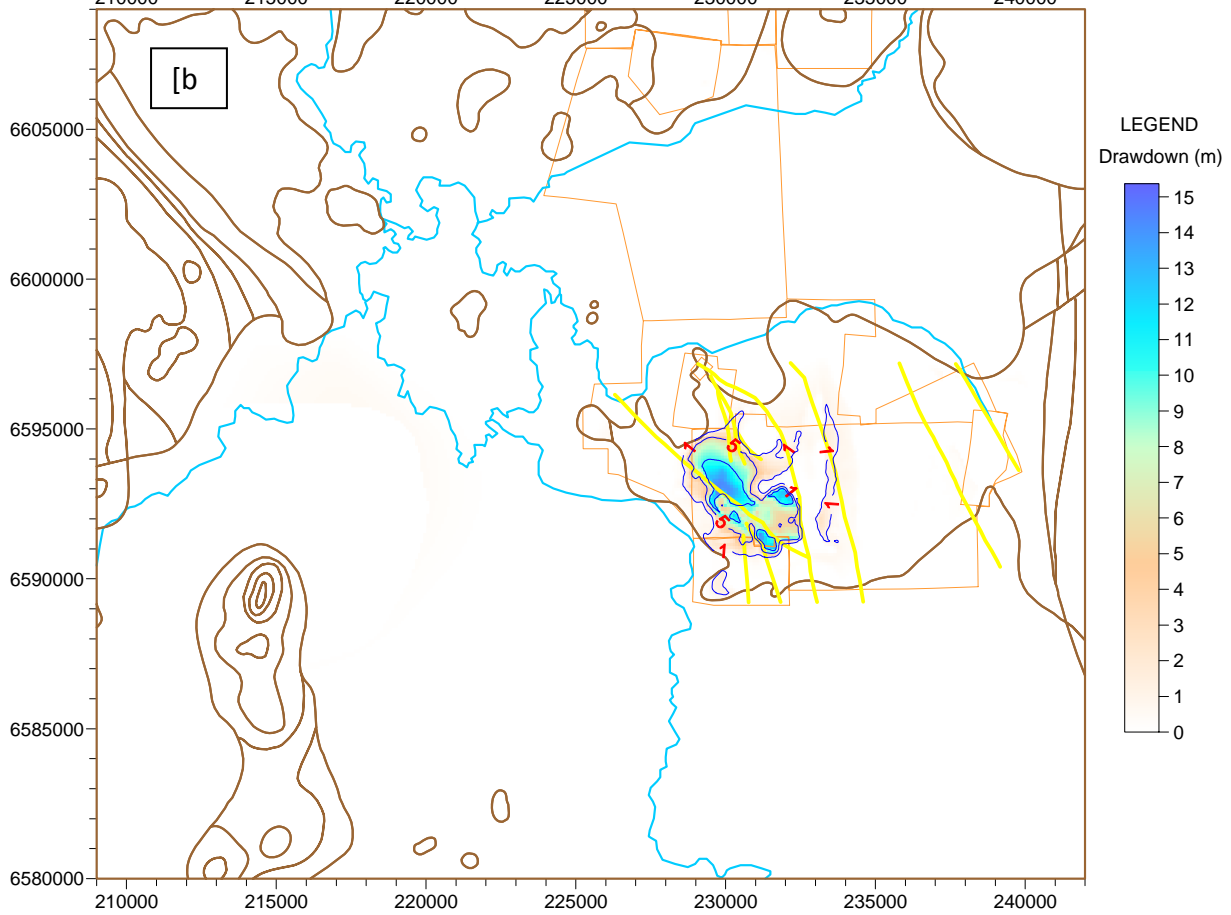
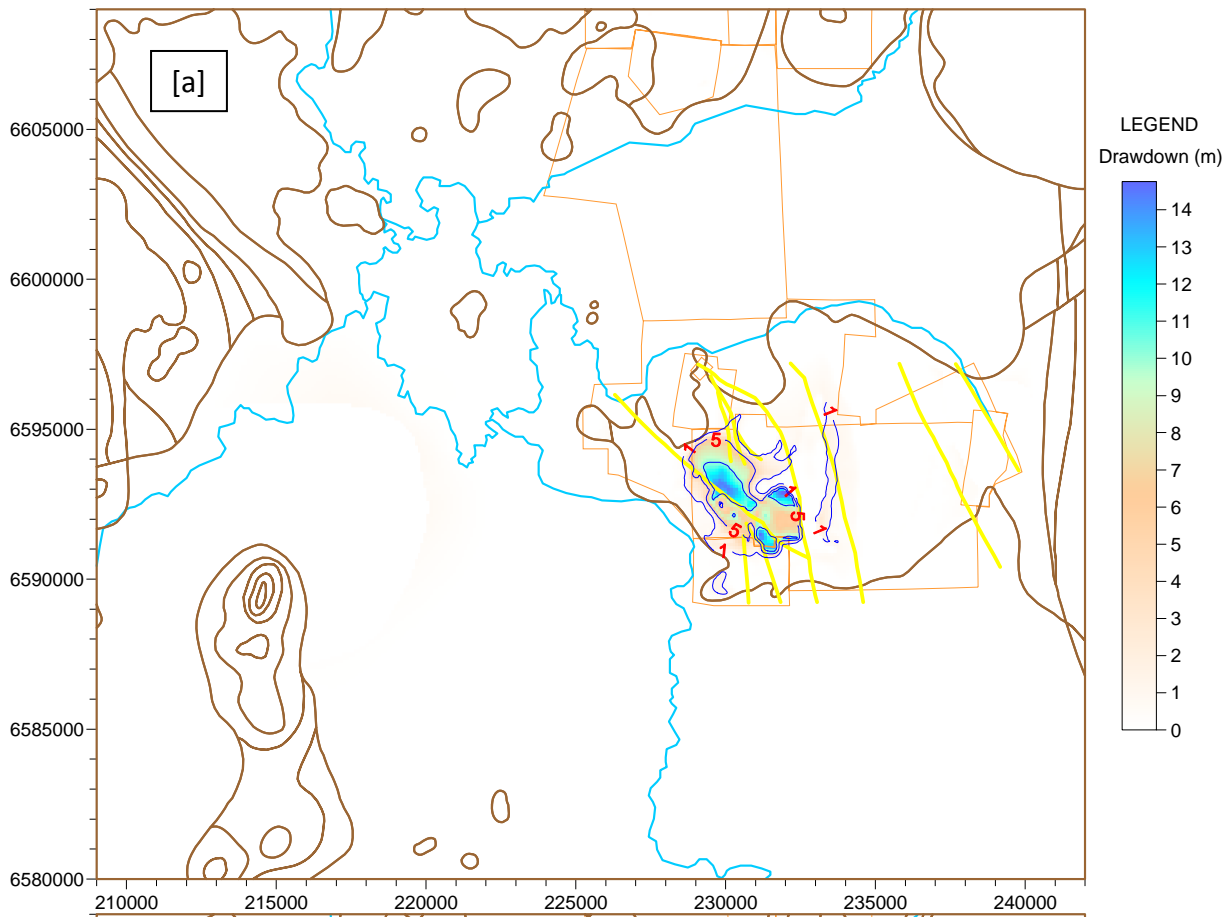
Predicted Drawdown Contours at the of End of Stress Period 10 (End of Year 2021) in Layers [a] 4 and [b] 6



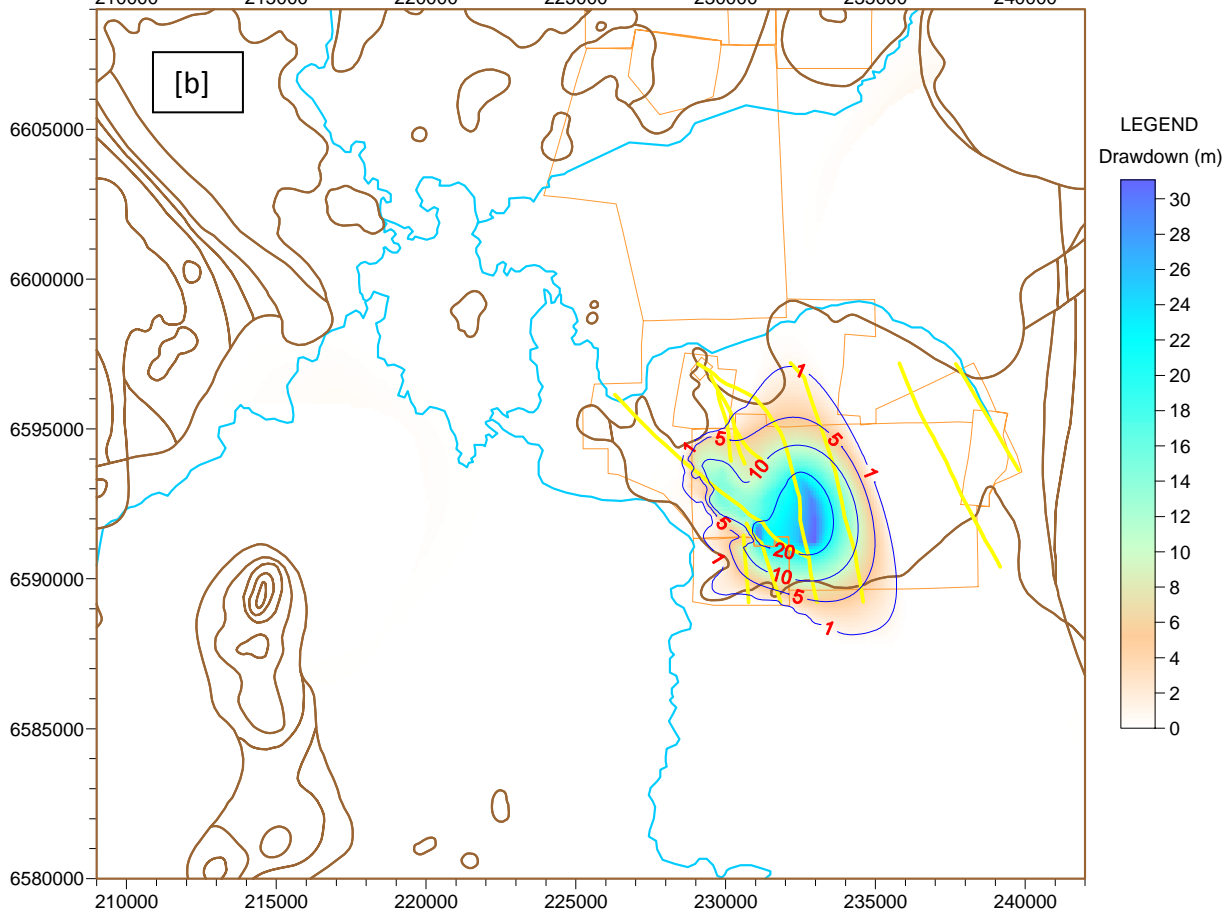
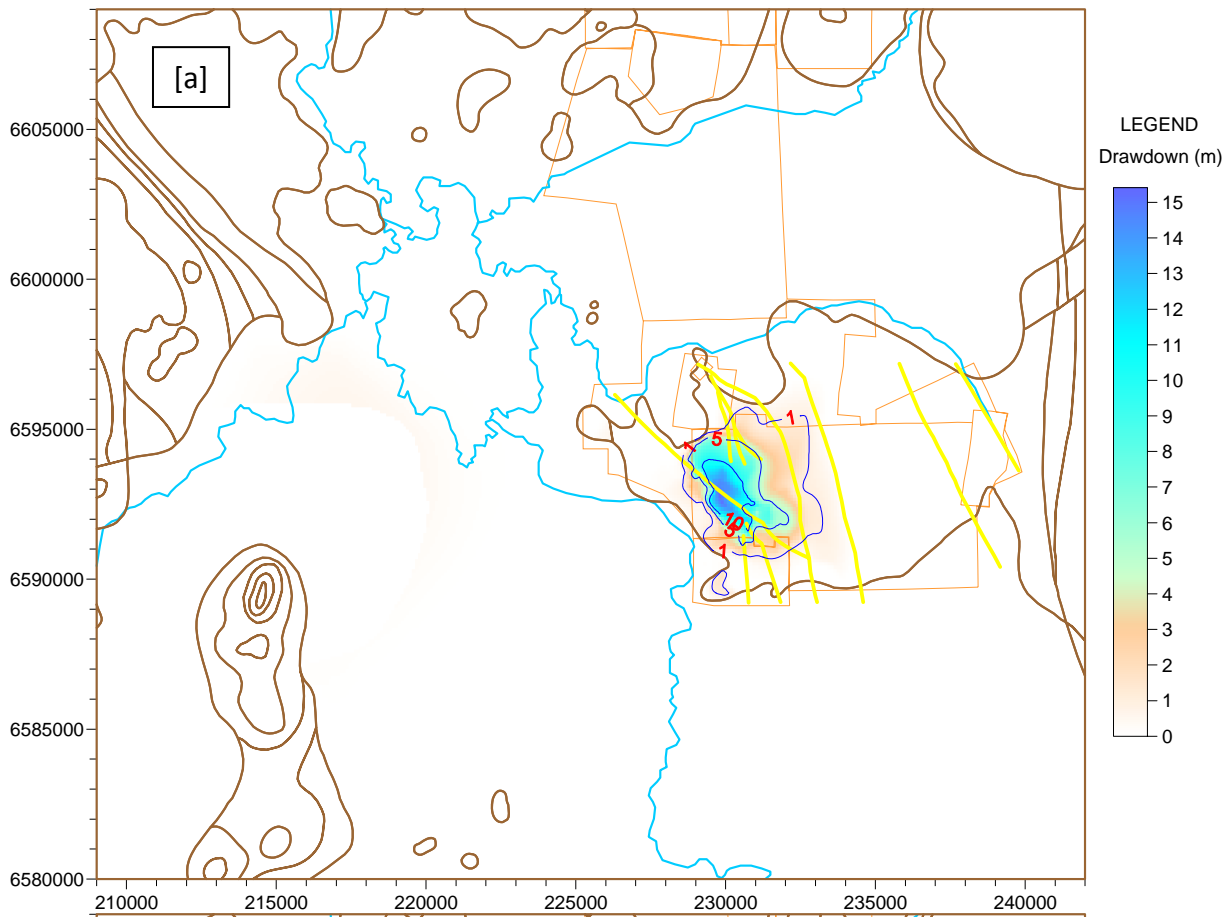
Predicted Drawdown Contours at the of End of Stress Period 10 (End of Year 2021) in Layers [a] 8 and [b] 10



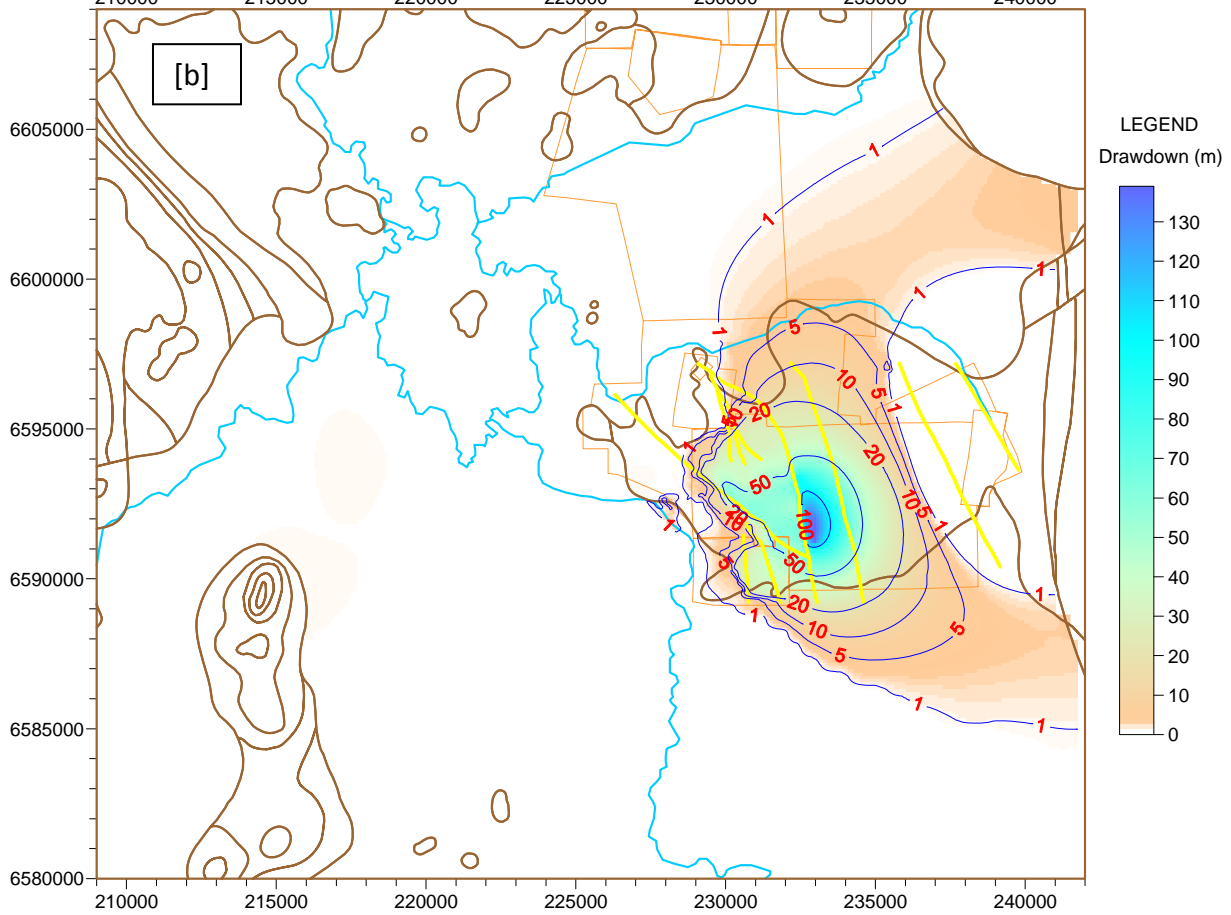
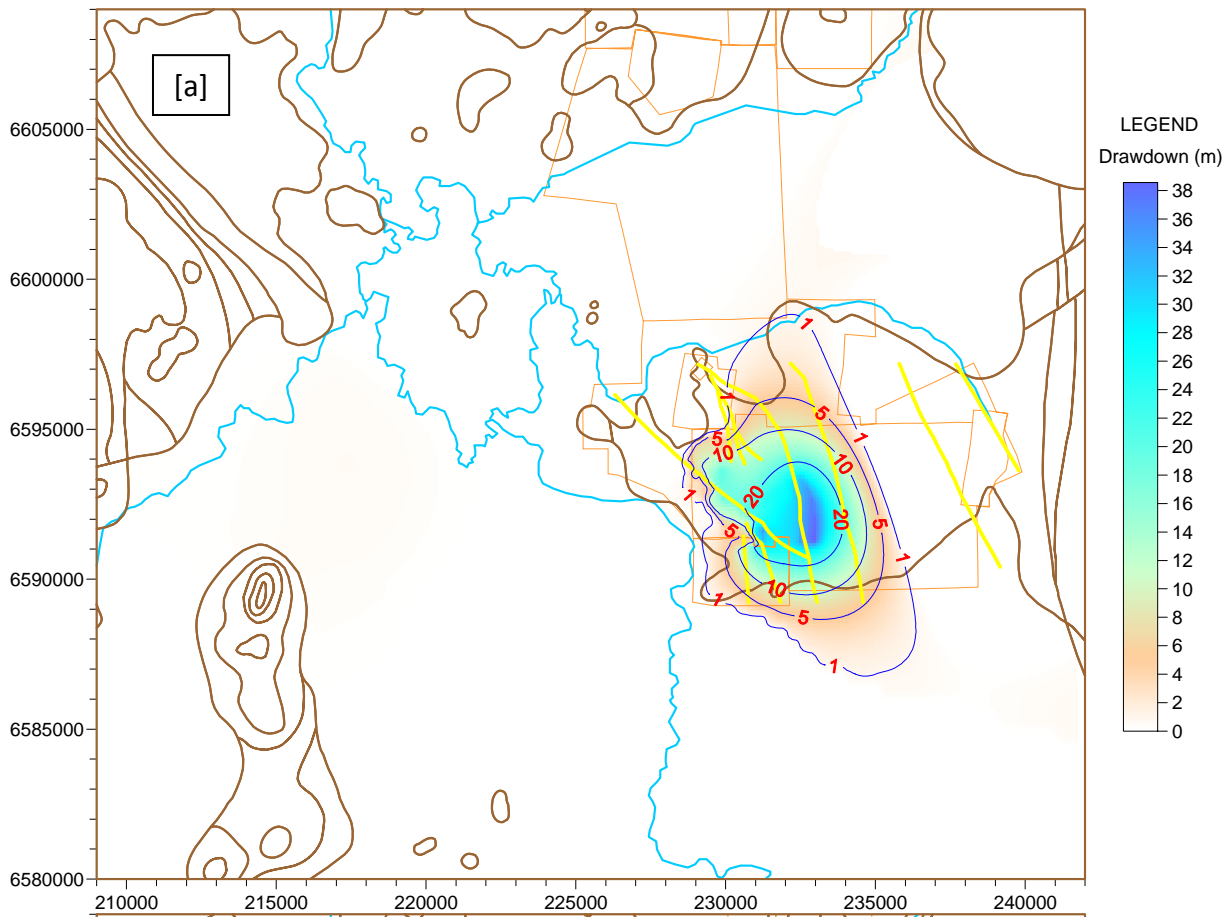
Predicted Drawdown Contours at the of End of Stress Period 10 (End of Year 2021) in Layer 12



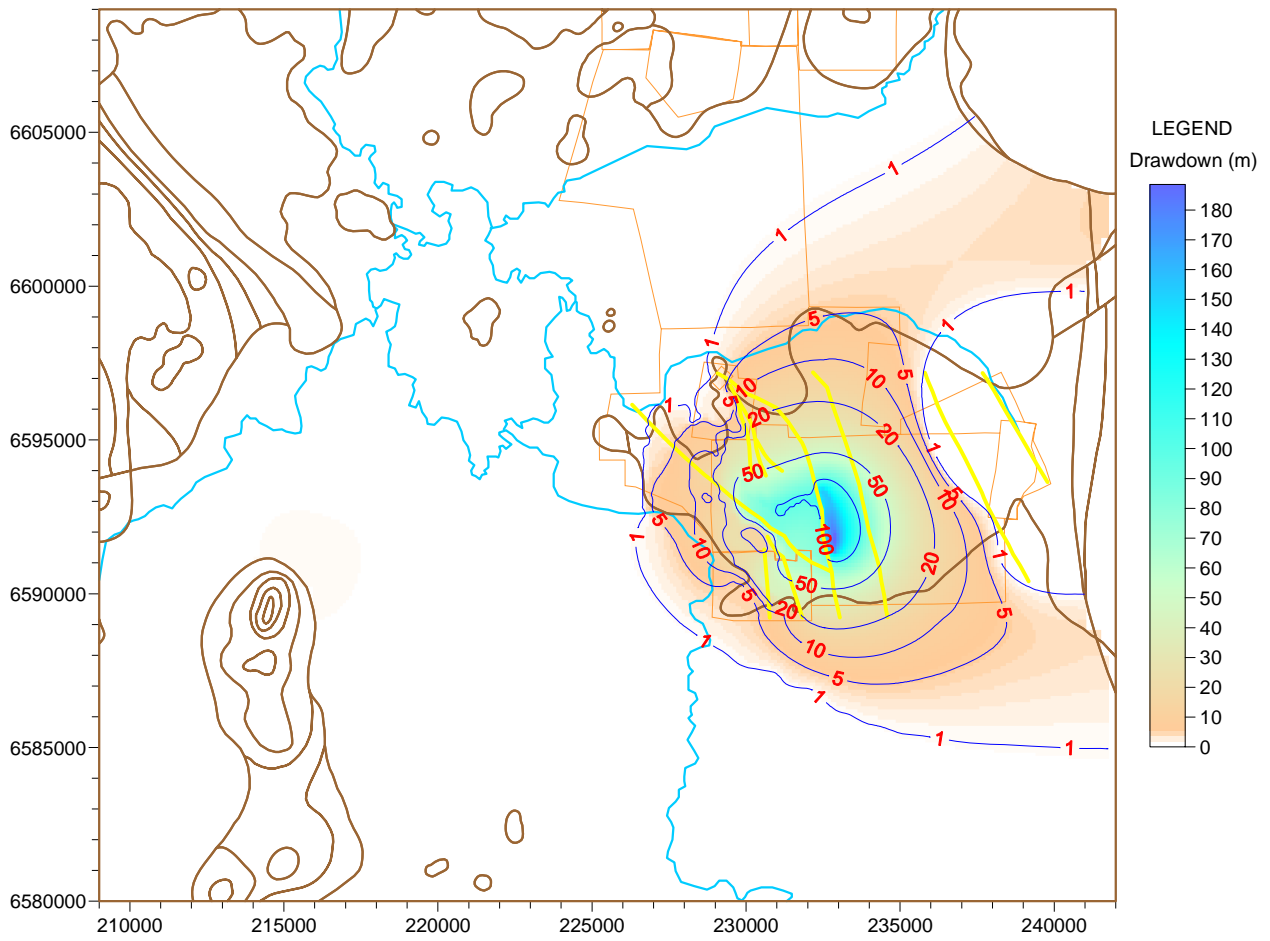
Predicted Drawdown Contours at the of End of Stress Period 15 (End of Year 2026) in Layers [a] 1 and [b] 2



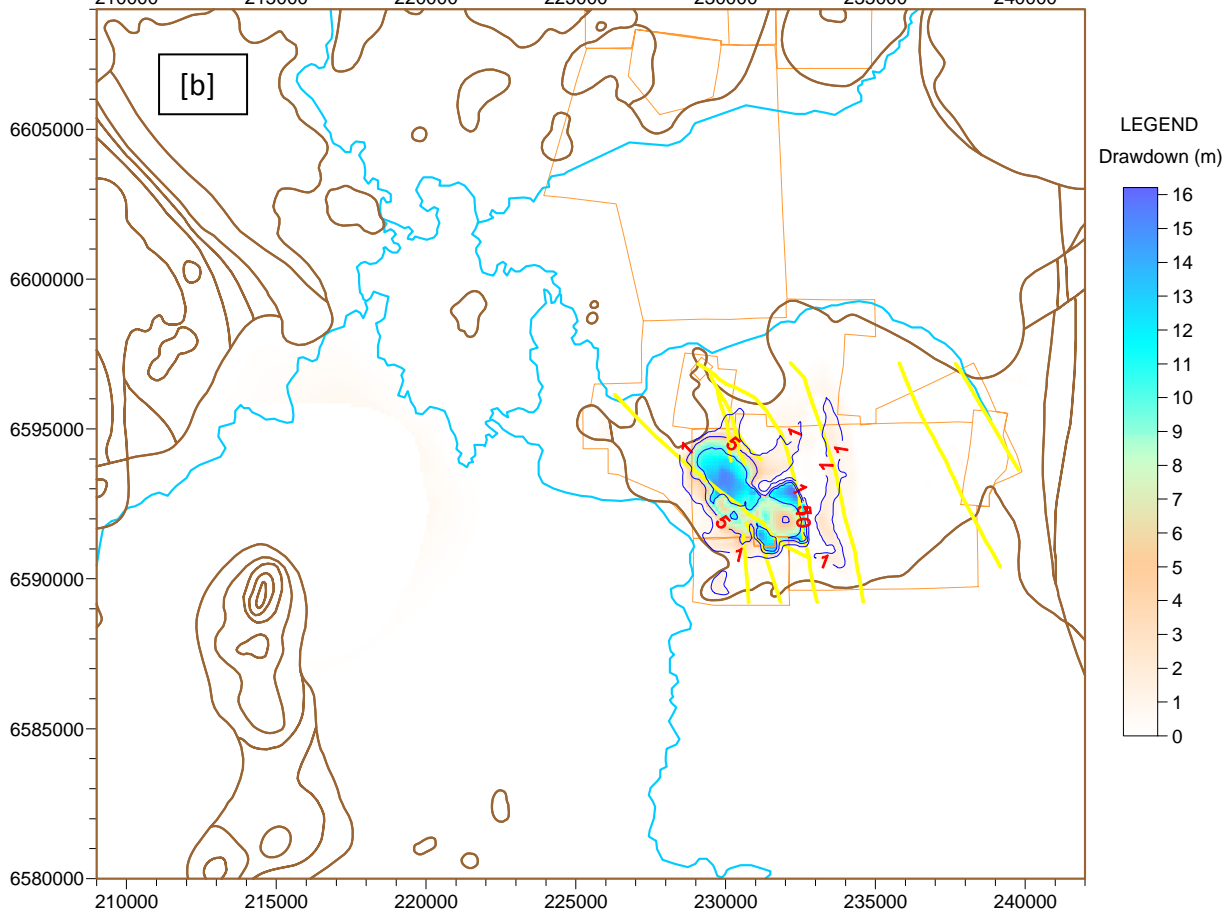
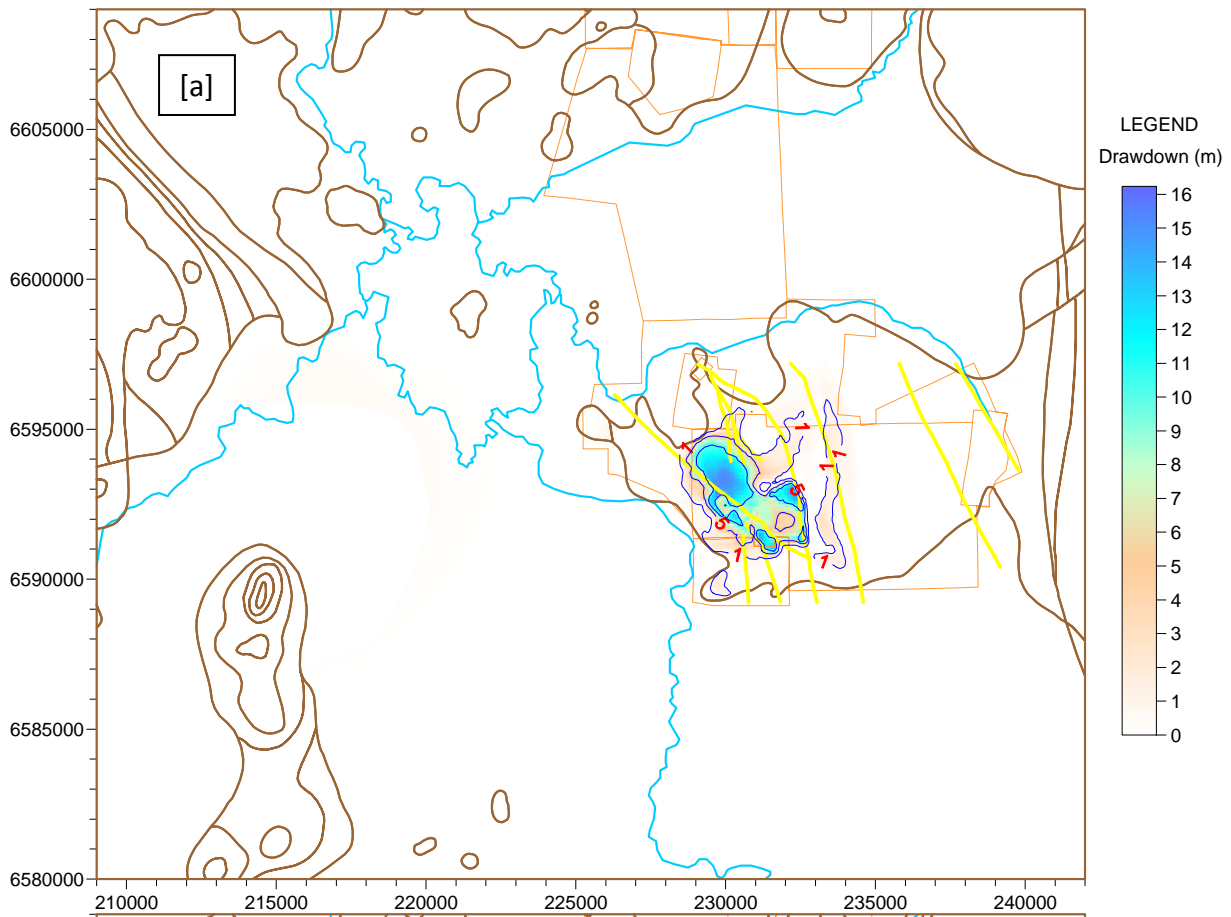
Predicted Drawdown Contours at the of End of Stress Period 15 (End of Year 2026) in Layers [a] 4 and [b] 6



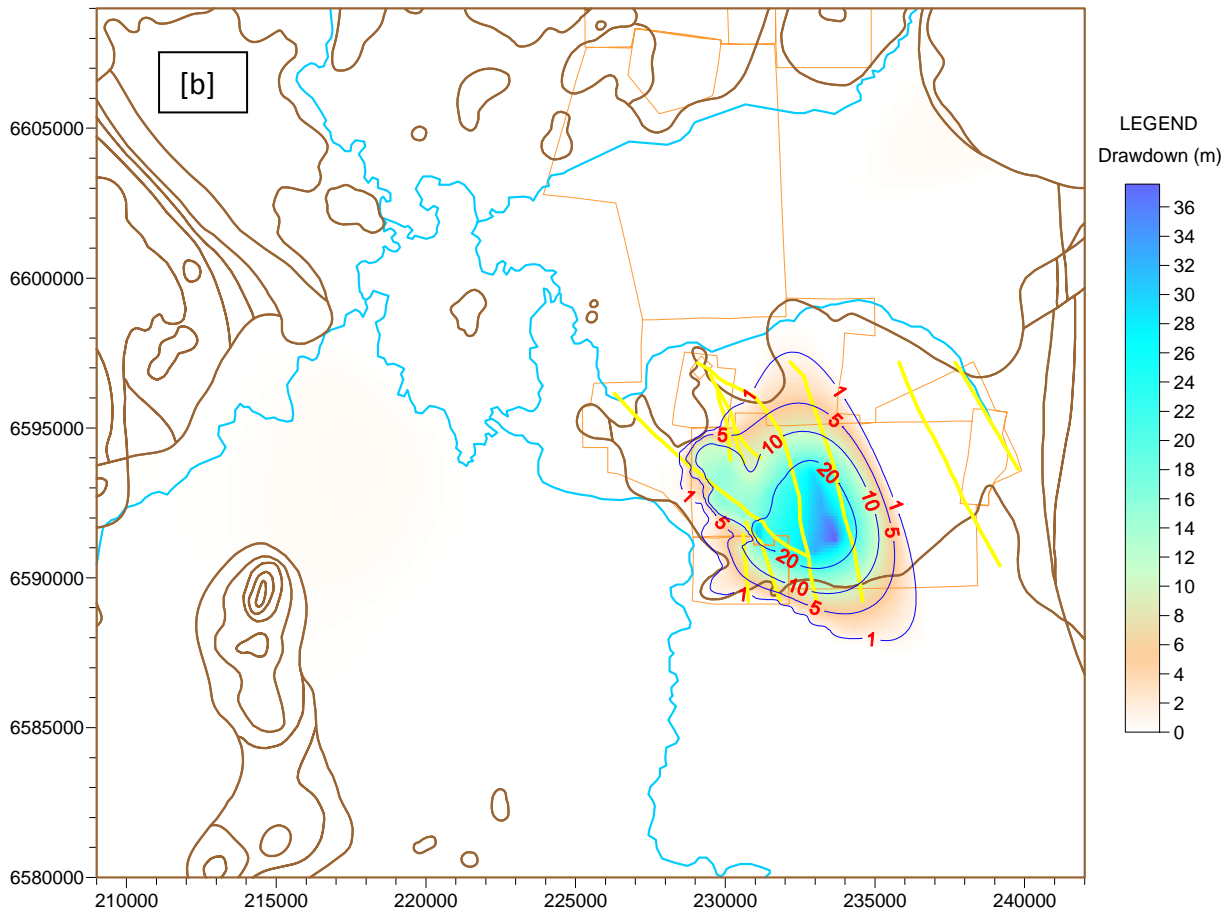
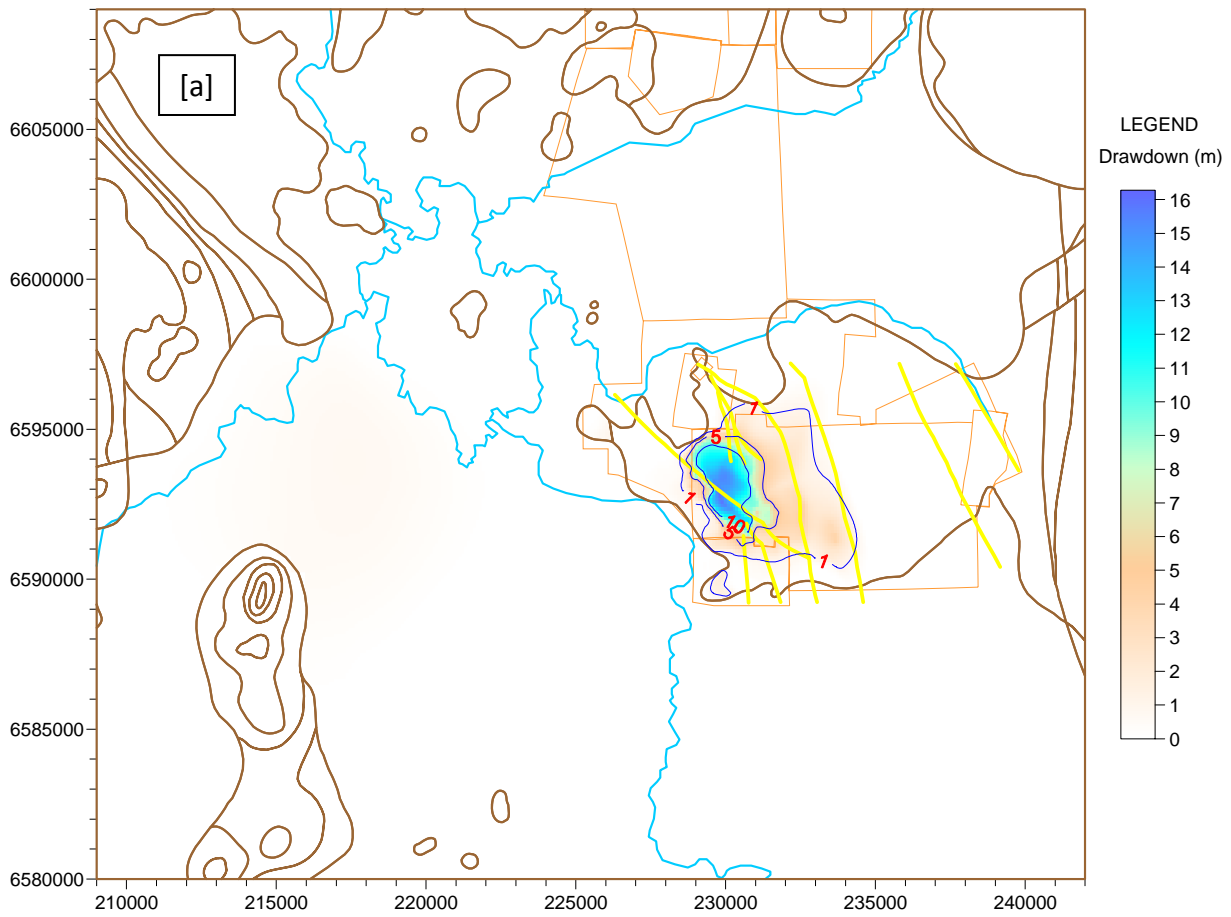
Predicted Drawdown Contours at the of End of Stress Period 15 (End of Year 2026) in Layers [a] 8 and [b] 10



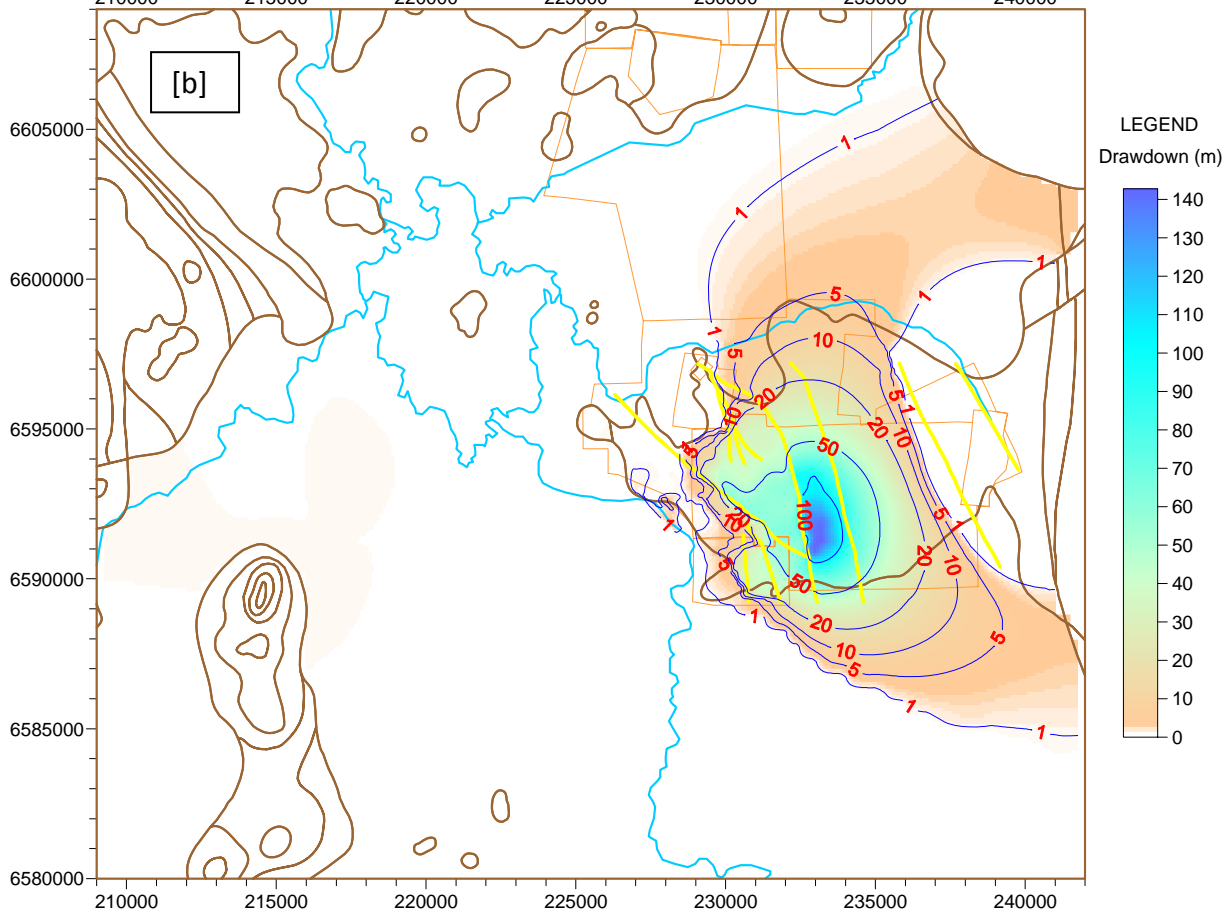
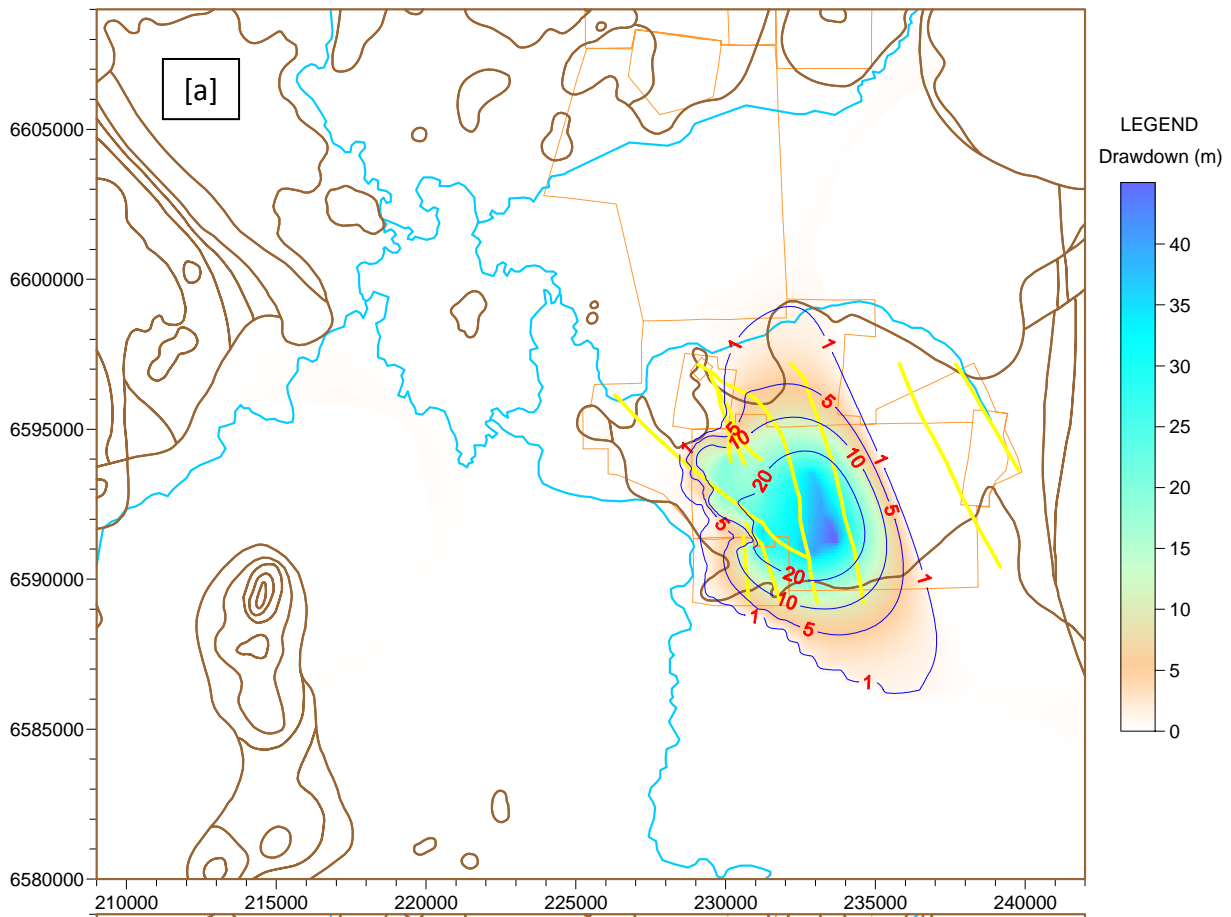
Predicted Drawdown Contours at the of End of Stress Period 15 (End of Year 2026) in Layer 12



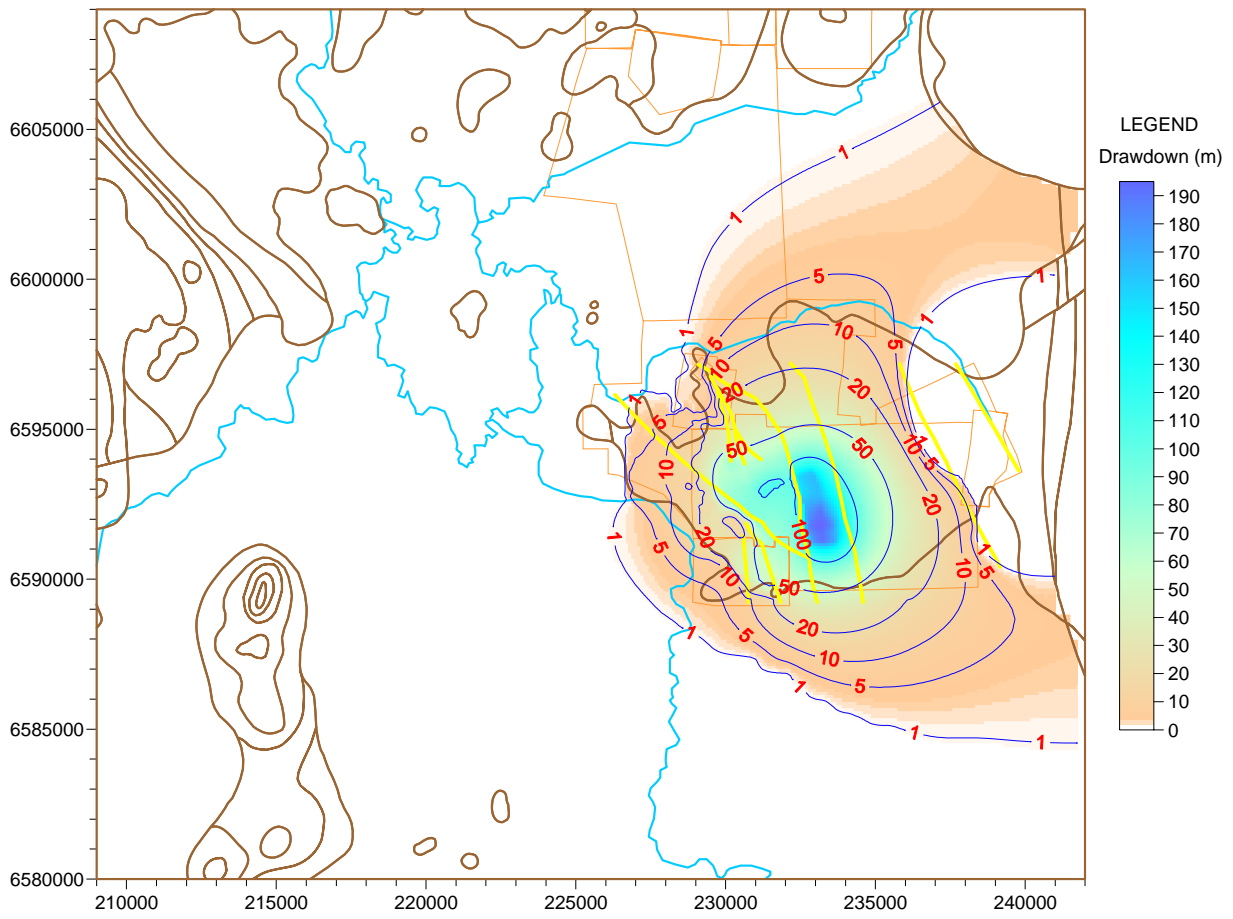
Predicted Drawdown Contours at the of End of Stress Period 20 (End of Year 2031) in Layers [a] 1 and [b] 2



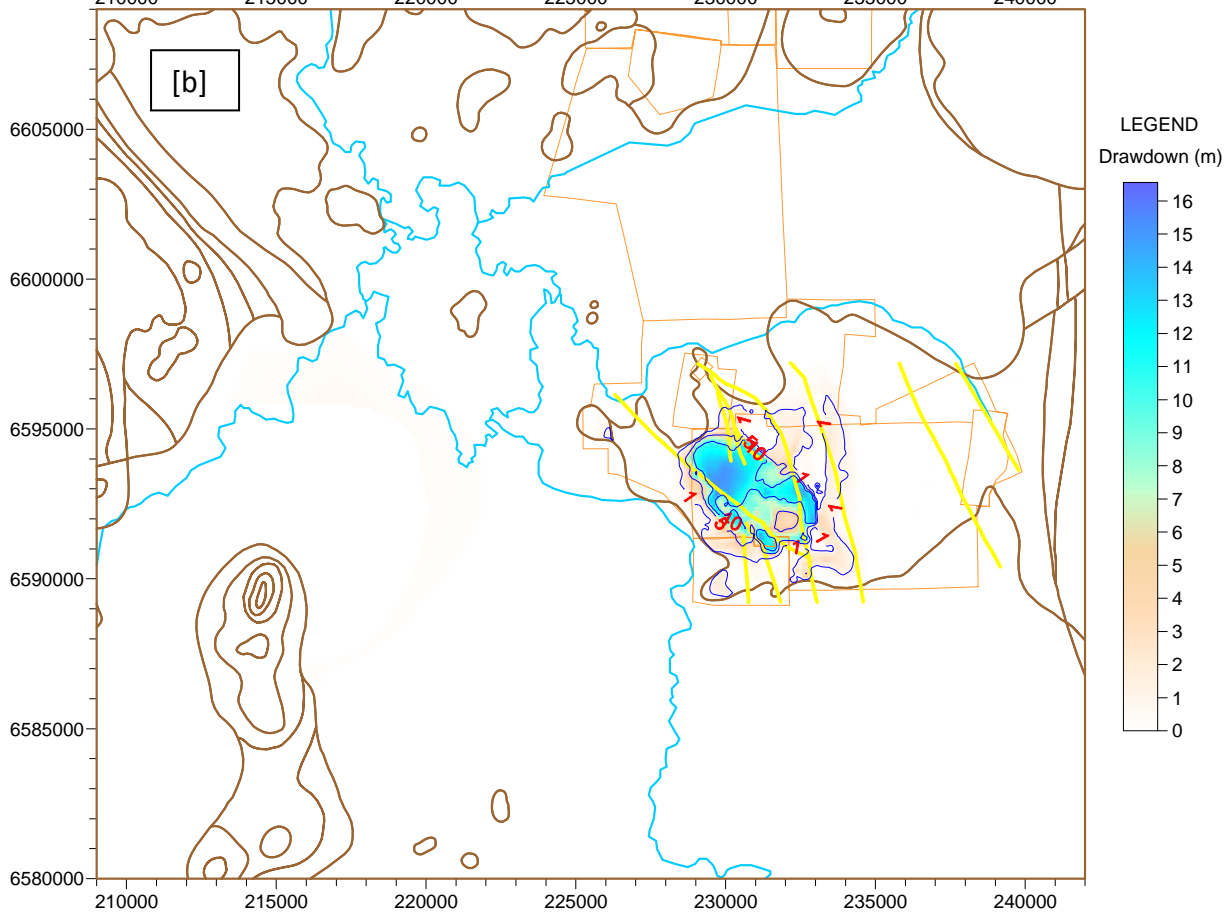
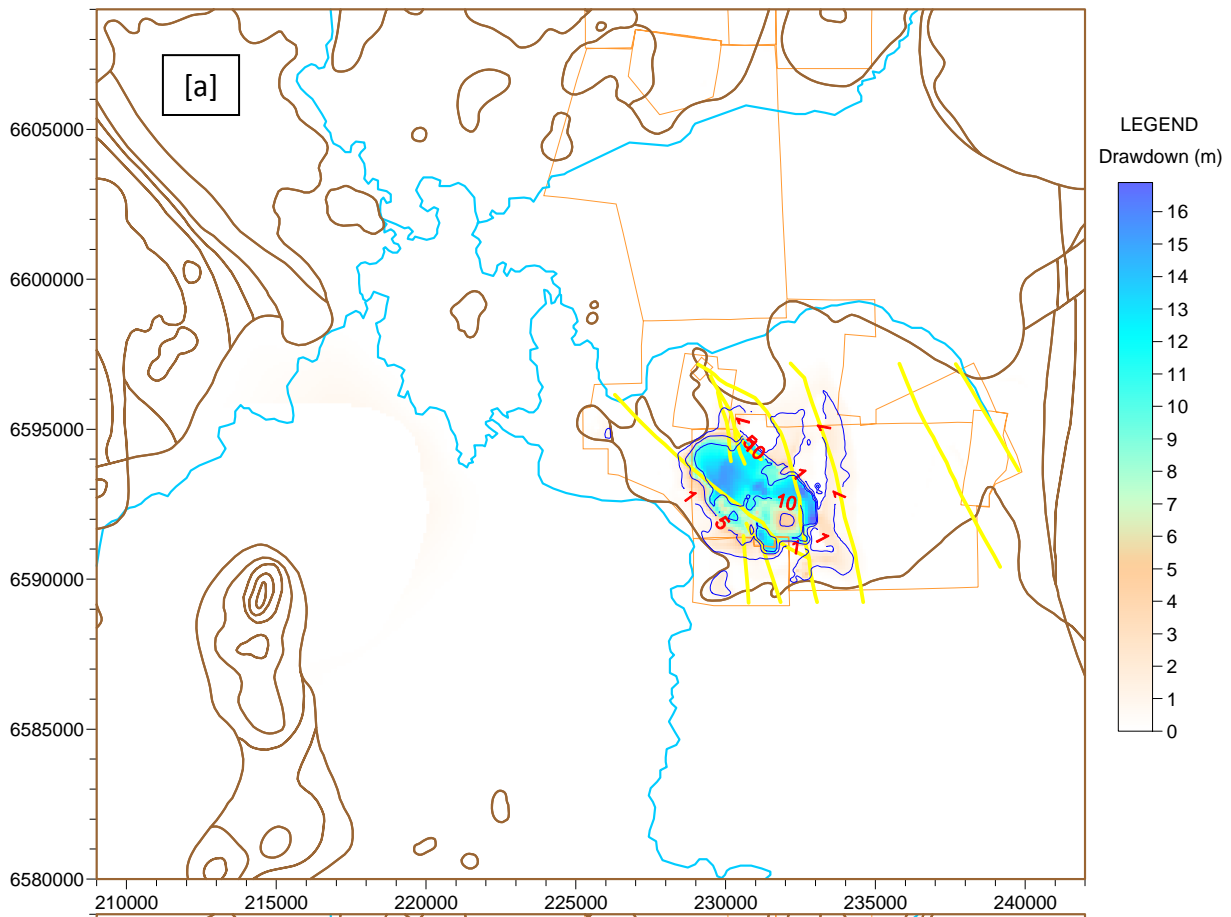
Predicted Drawdown Contours at the of End of Stress Period 20 (End of Year 2031) in Layers [a] 4 and [b] 6



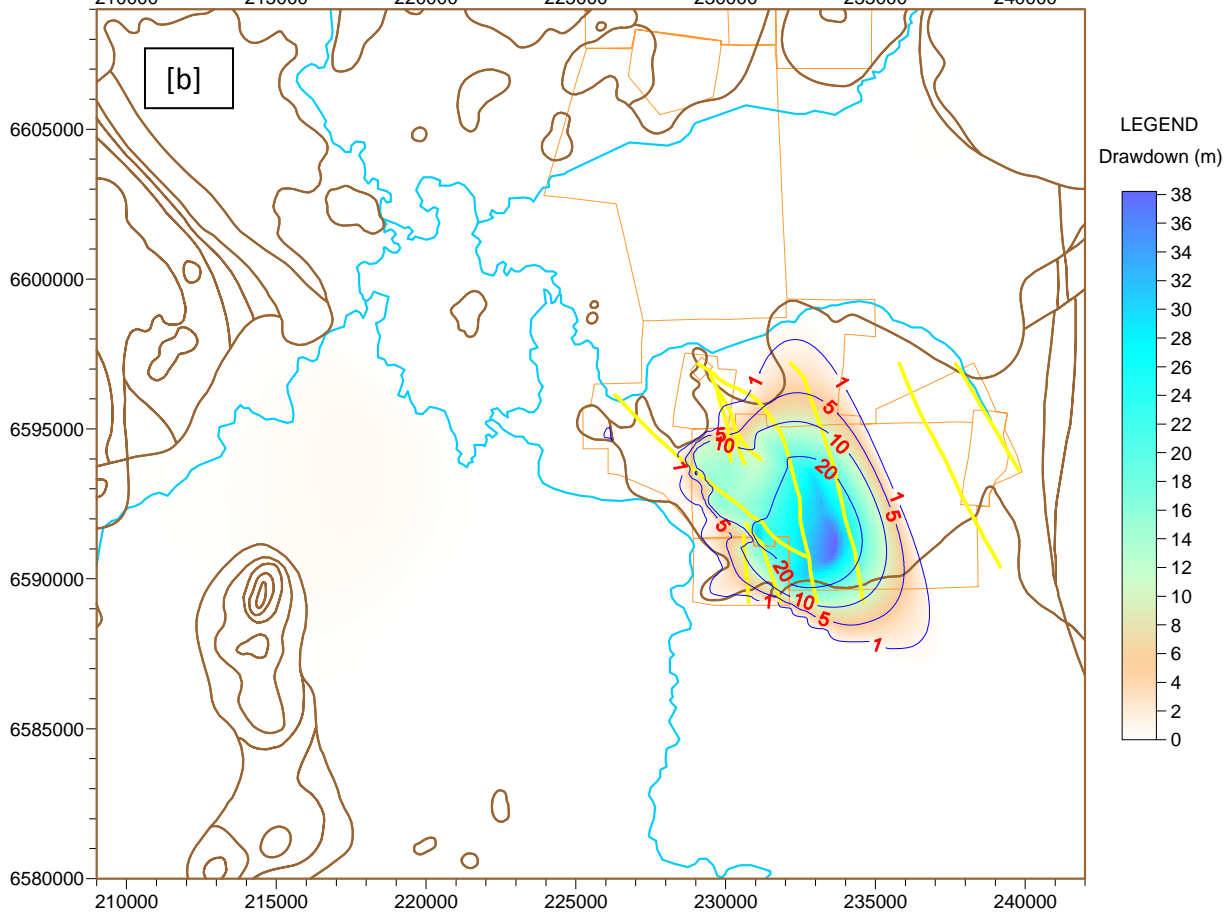
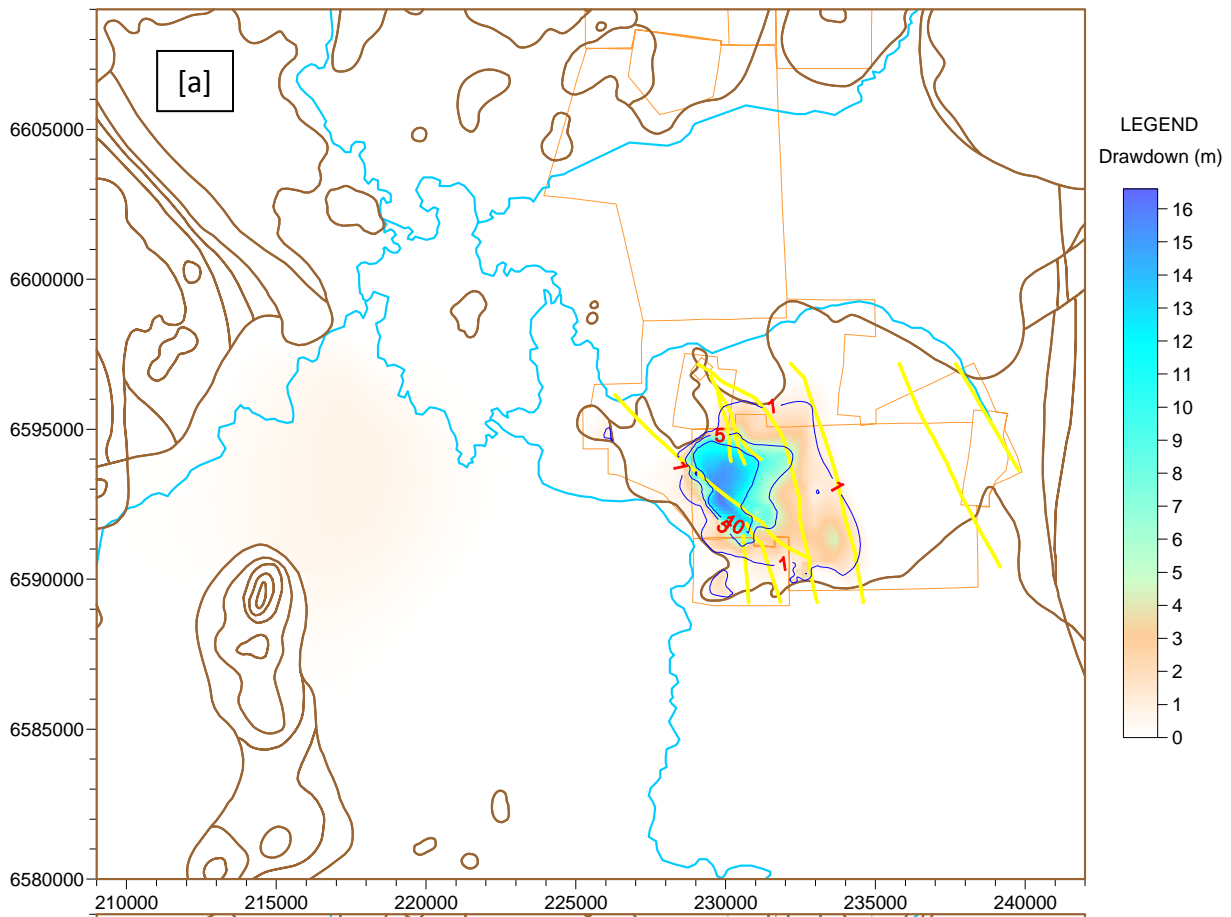
Predicted Drawdown Contours at the of End of Stress Period 20 (End of Year 2031) in Layers [a] 8 and [b] 10



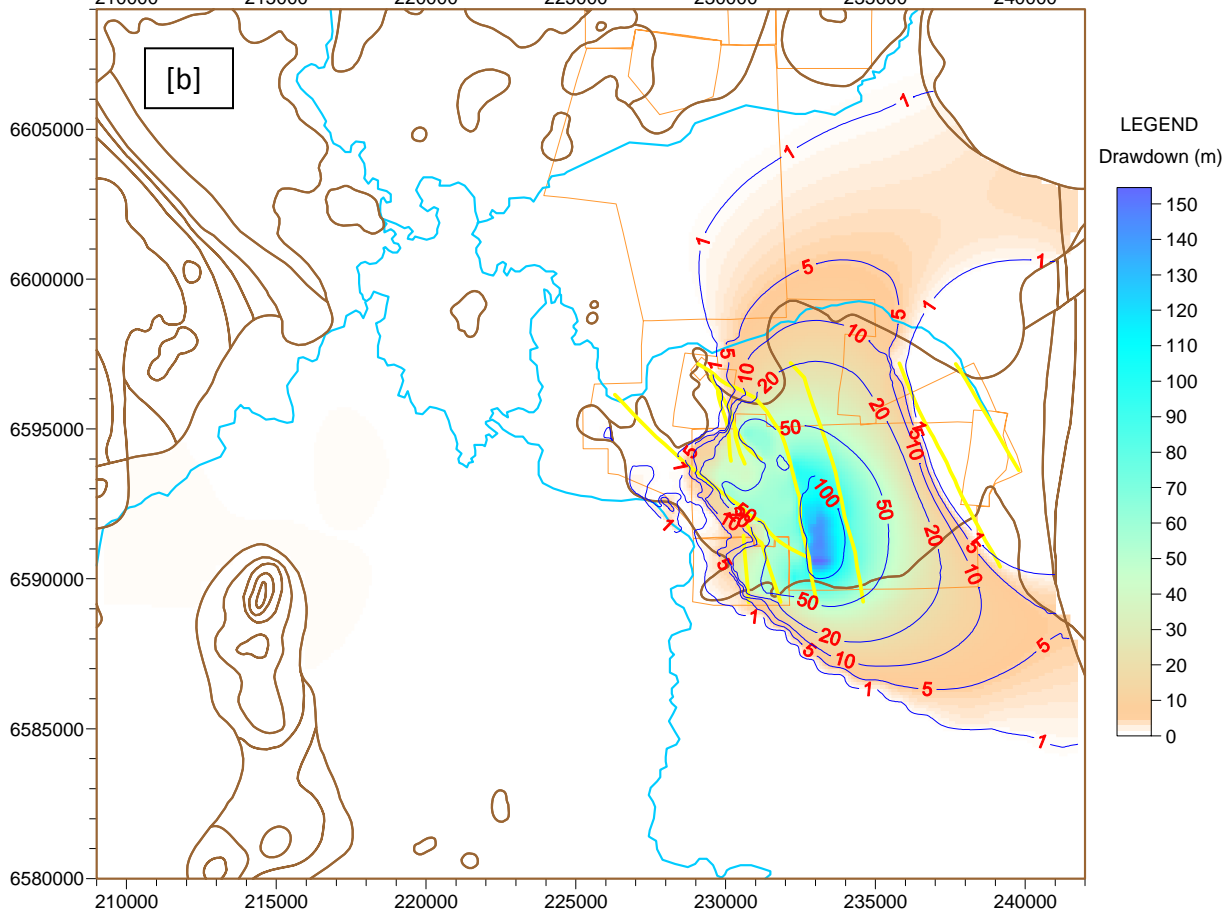
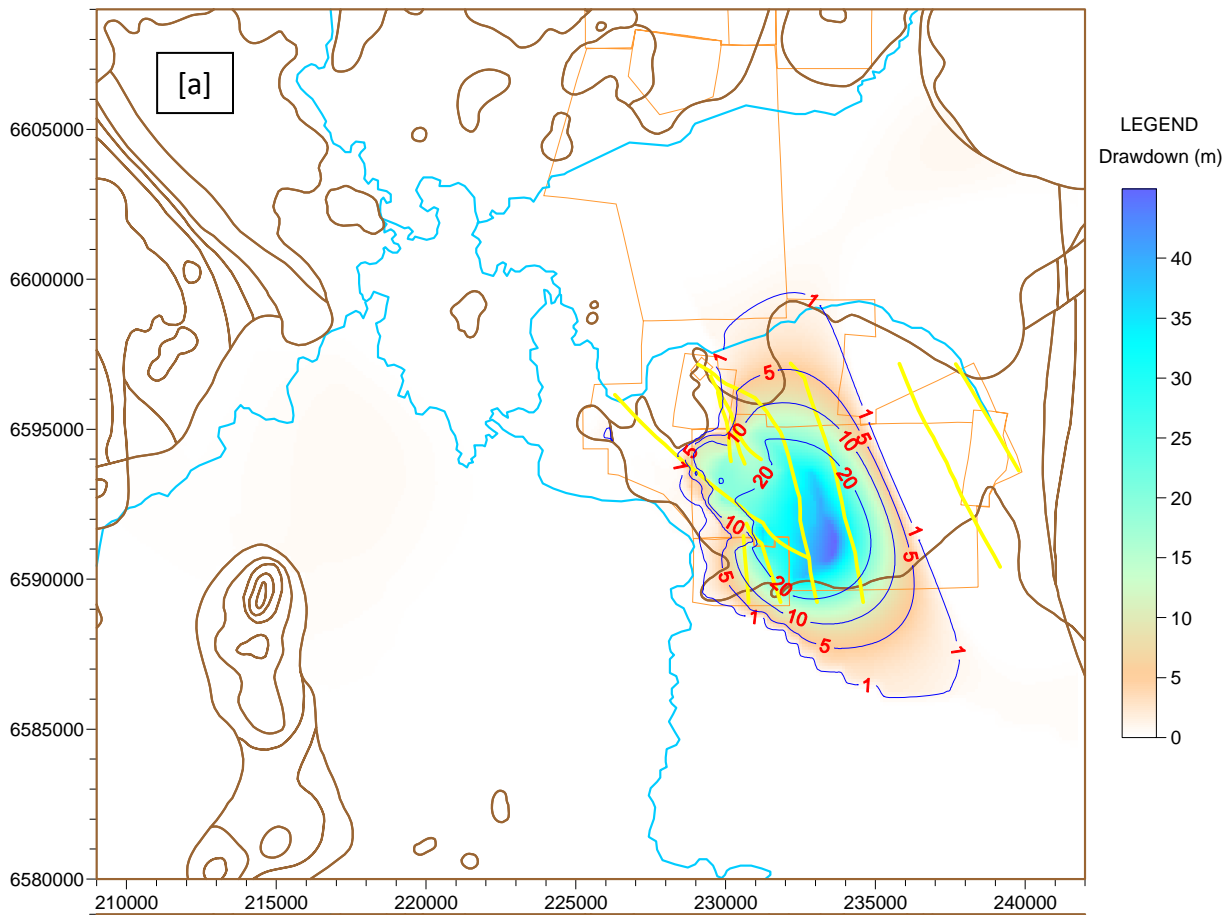
Predicted Drawdown Contours at the of End of Stress Period 20 (End of Year 2031) in Layer 12



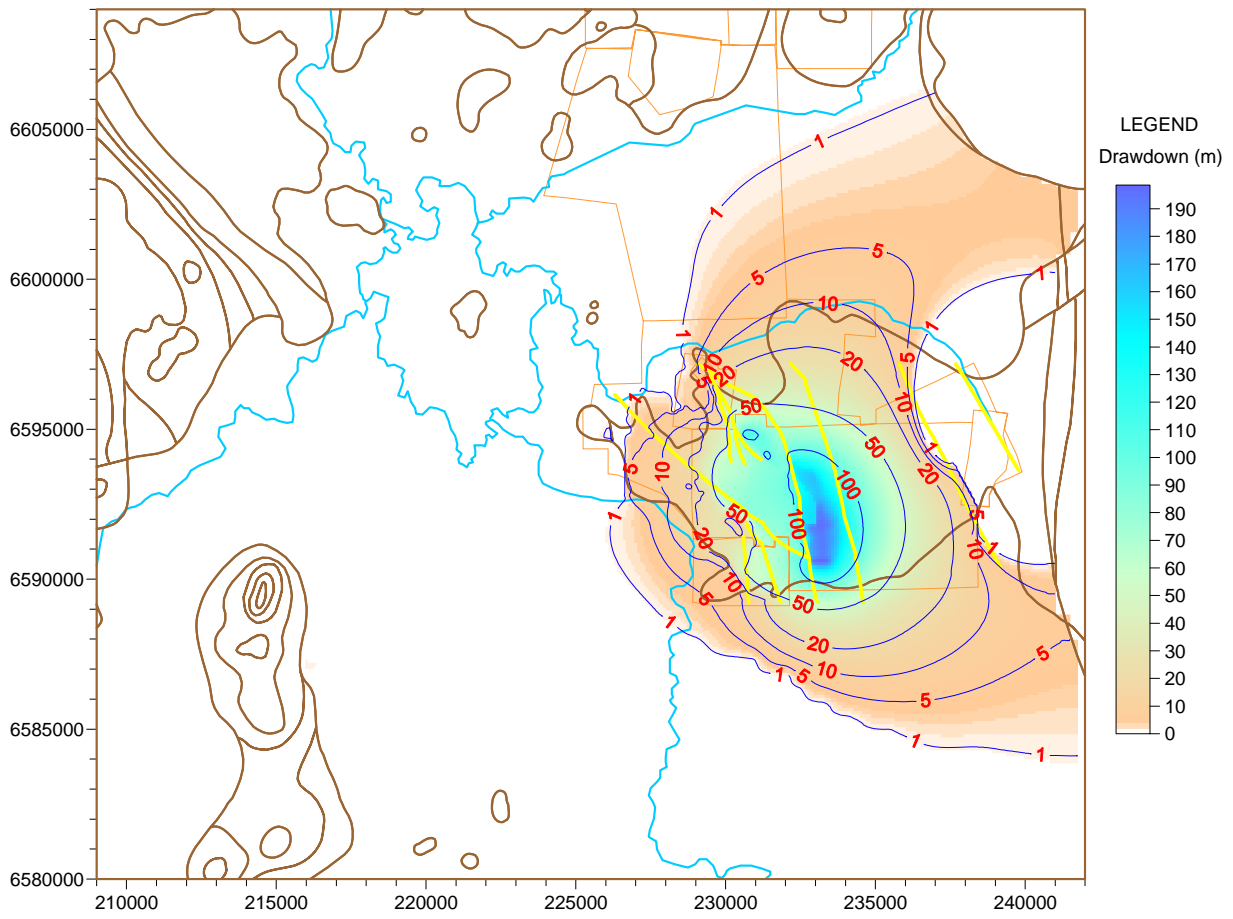
Predicted Drawdown Contours at the of End of Stress Period 25 (End of Year 2036) in Layers [a] 1 and [b] 2



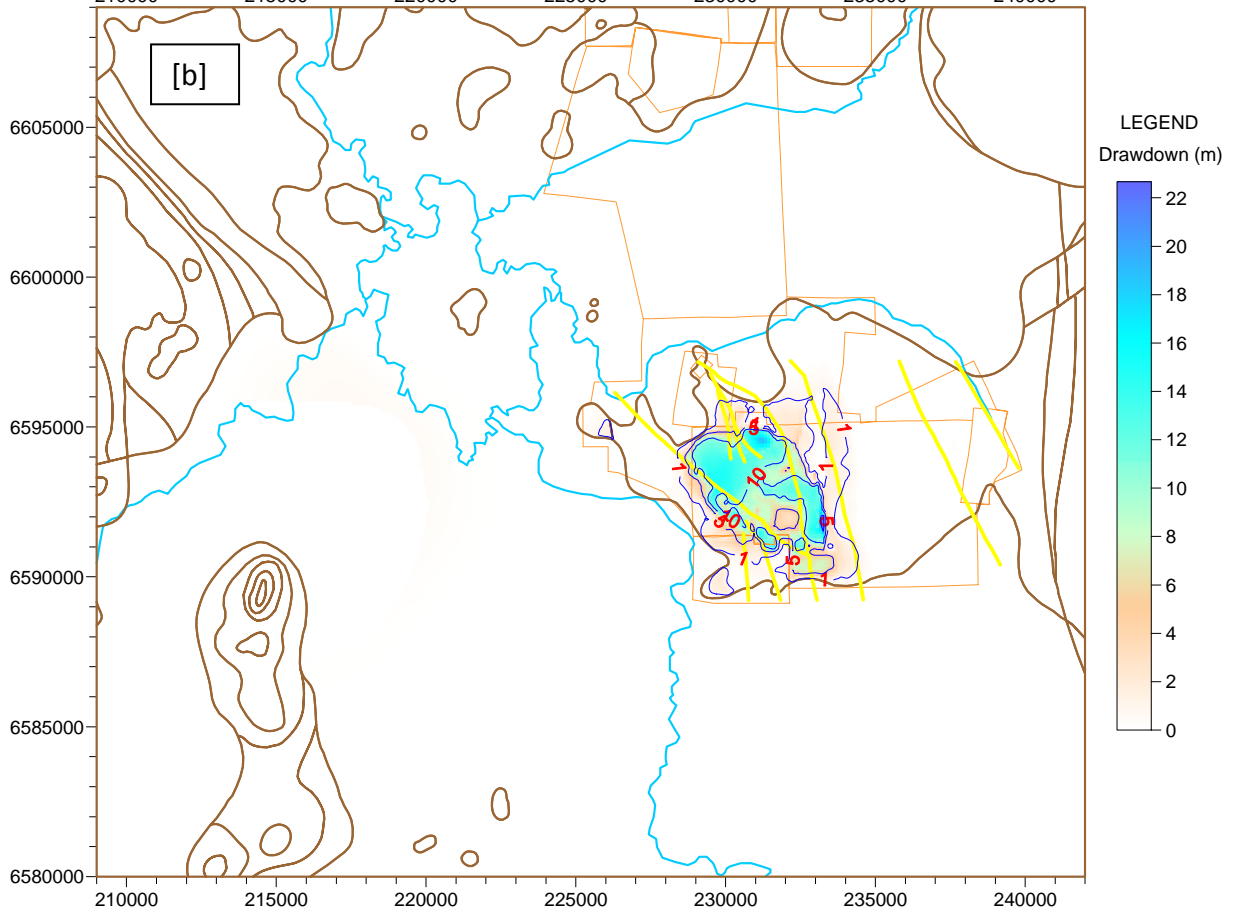
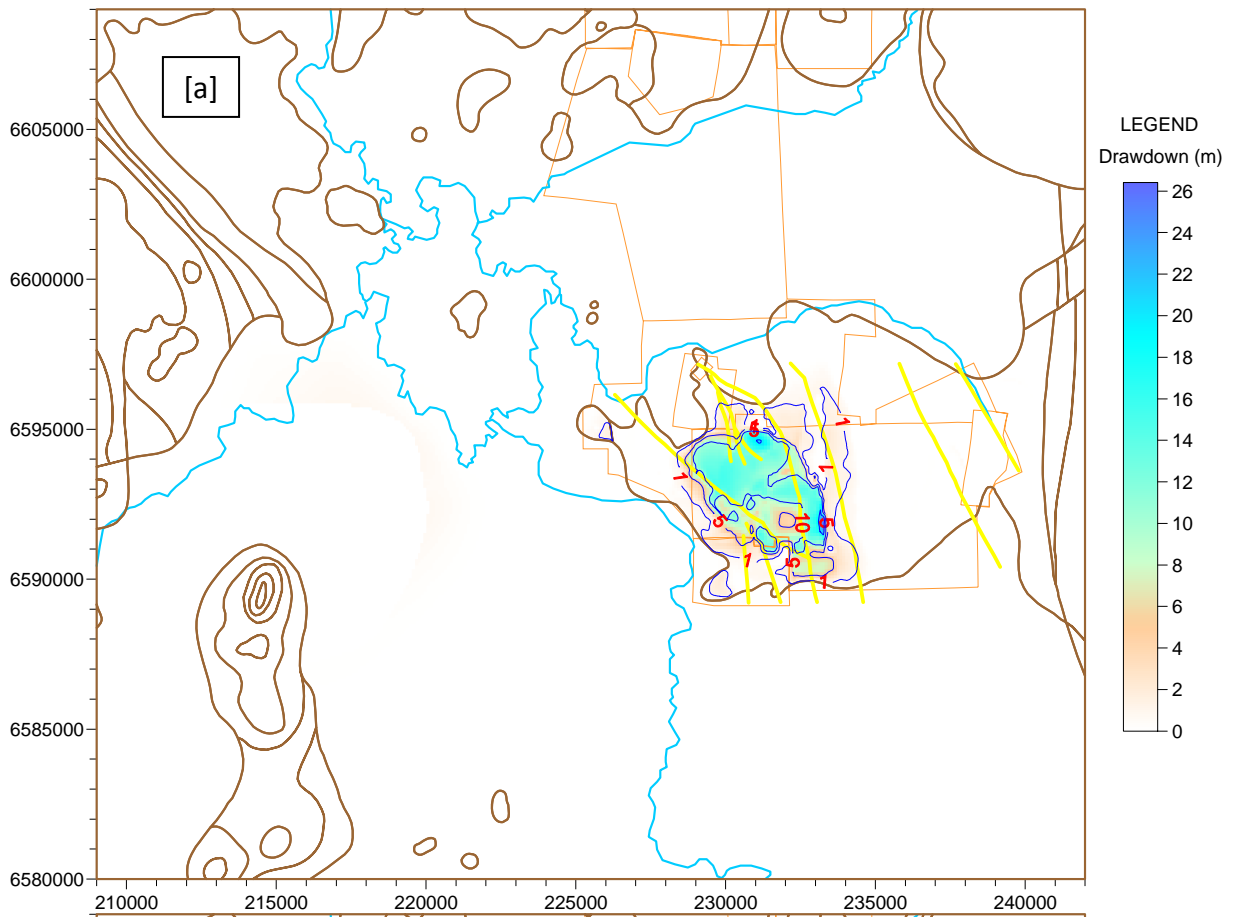
Predicted Drawdown Contours at the of End of Stress Period 25 (End of Year 2036) in Layers [a] 4 and [b] 6



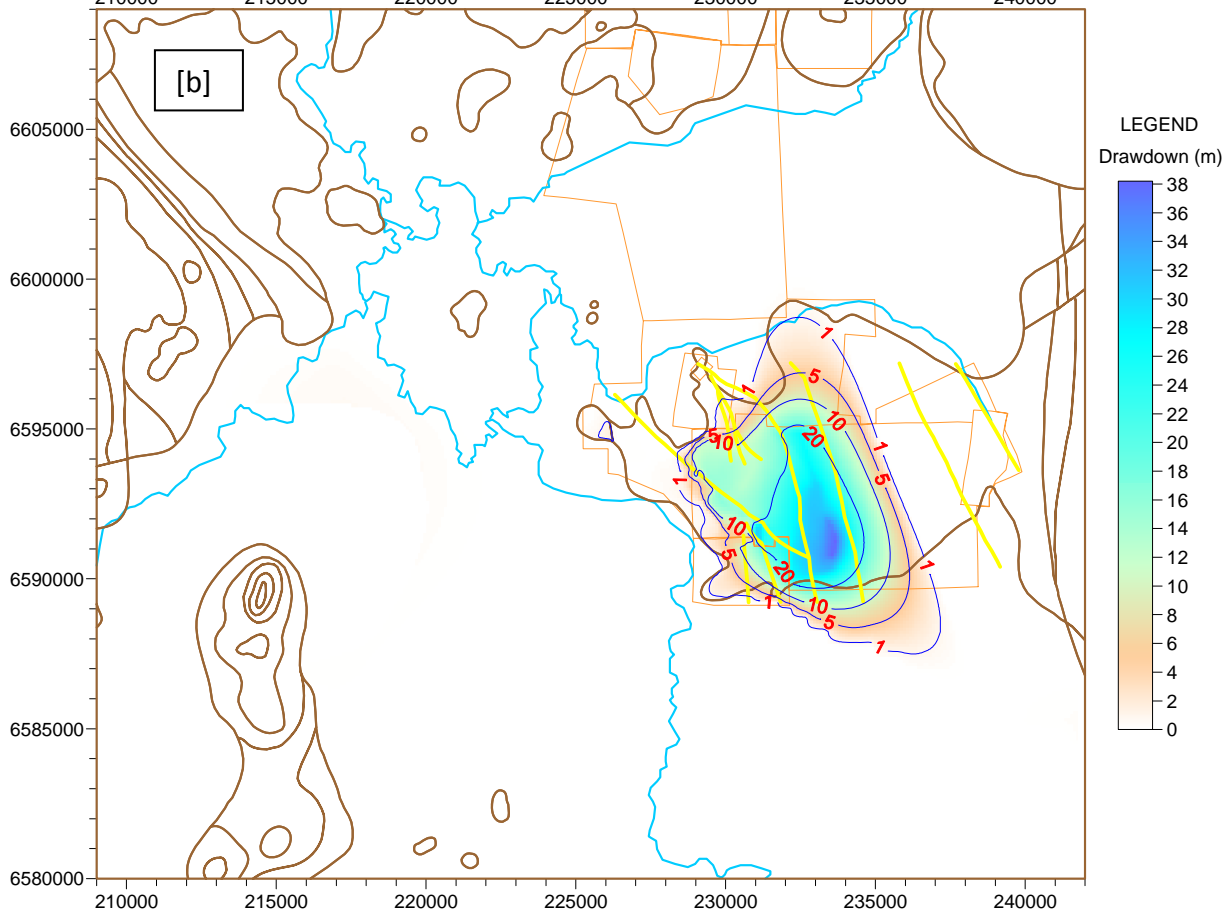
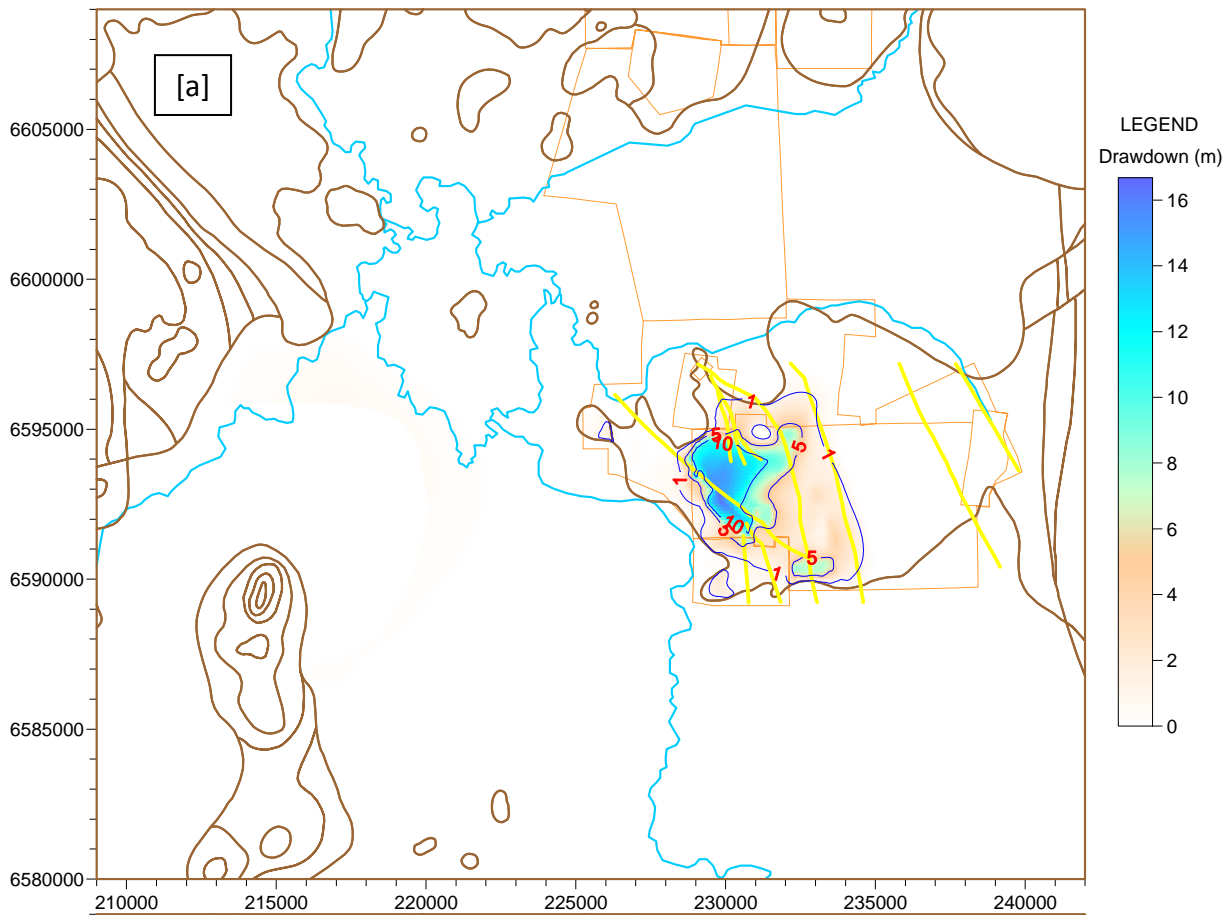
Predicted Drawdown Contours at the of End of Stress Period 25 (End of Year 2036) in Layers [a] 8 and [b] 10



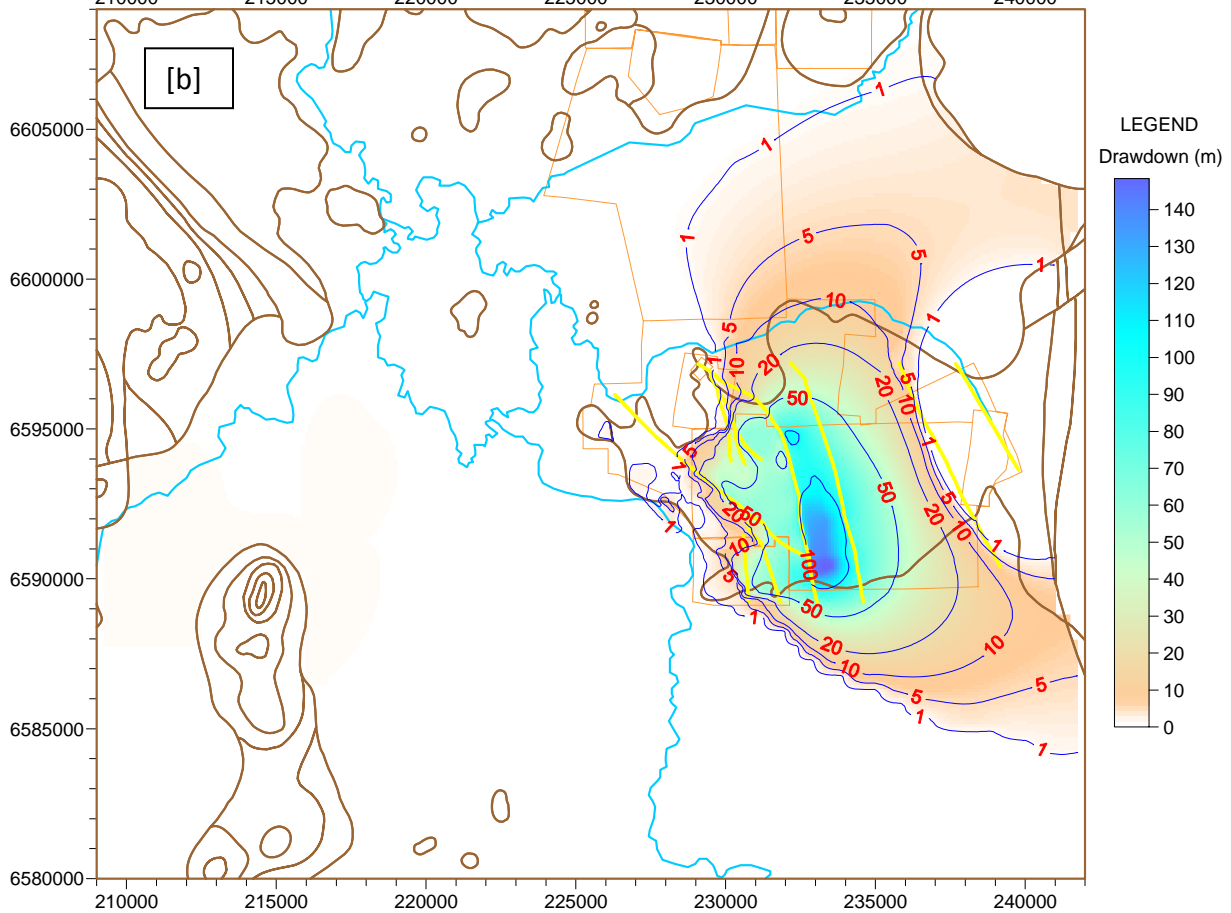
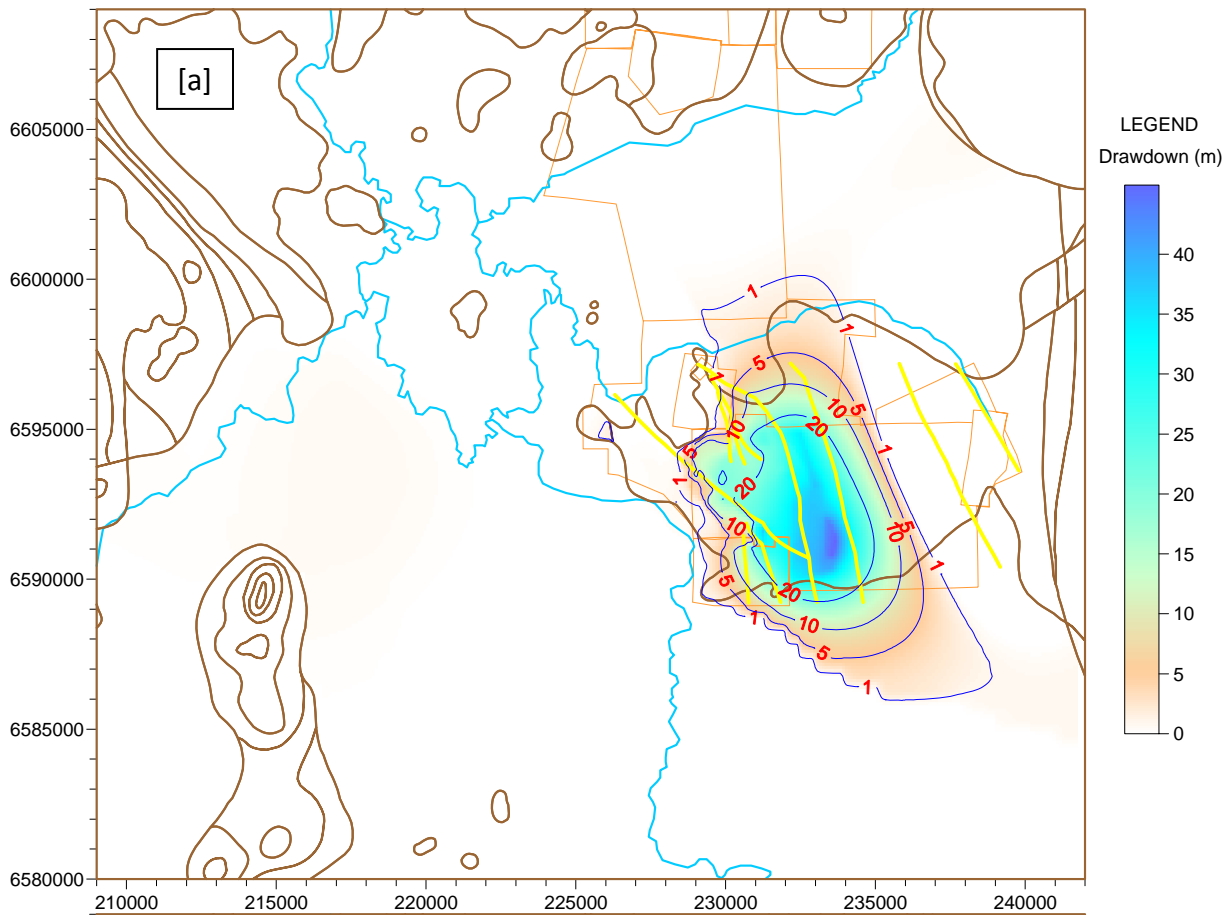
Predicted Drawdown Contours at the of End of Stress Period 25 (End of Year 2036) in Layer 12



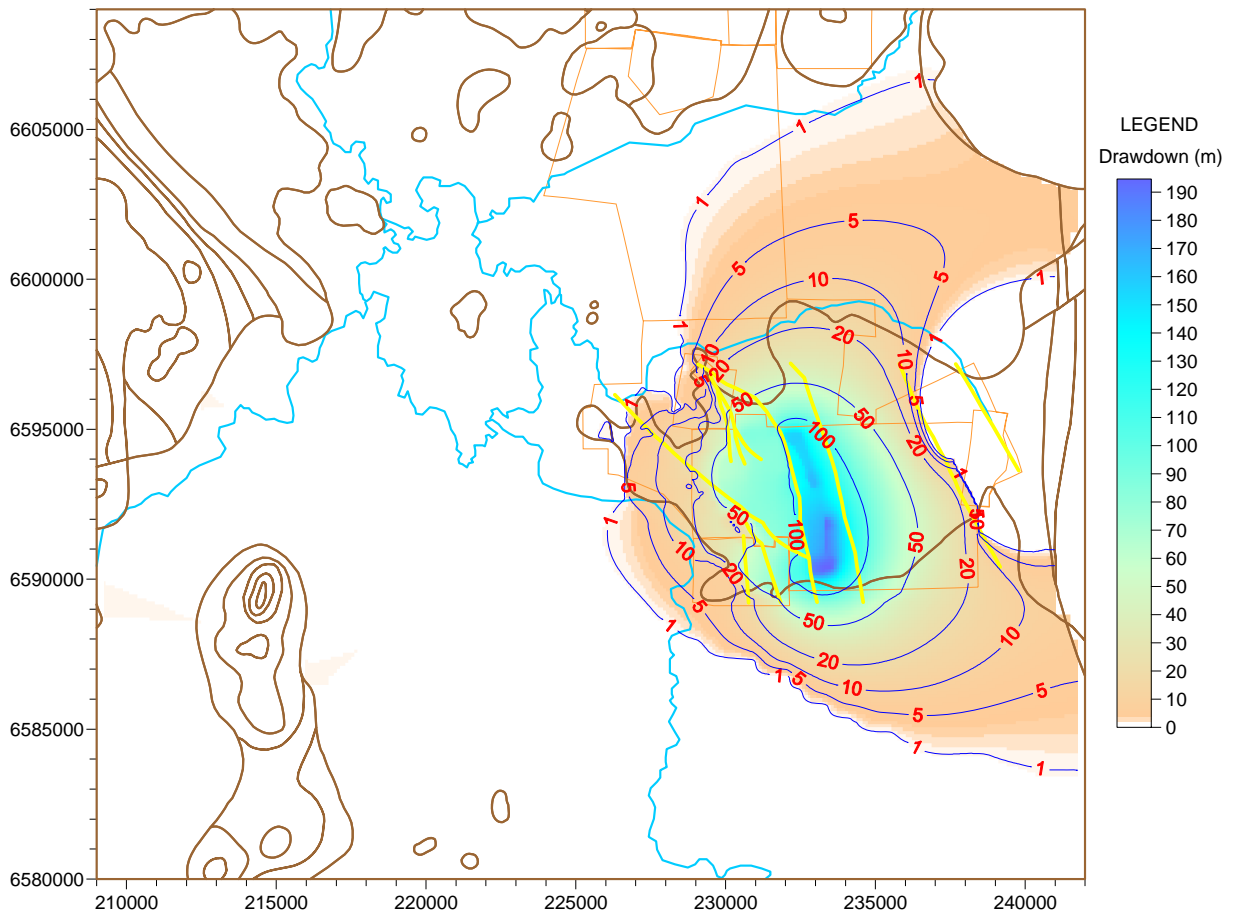
Predicted Drawdown Contours at the of End of Stress Period 31 (End of Year 2042) in Layers [a] 1 and [b] 2



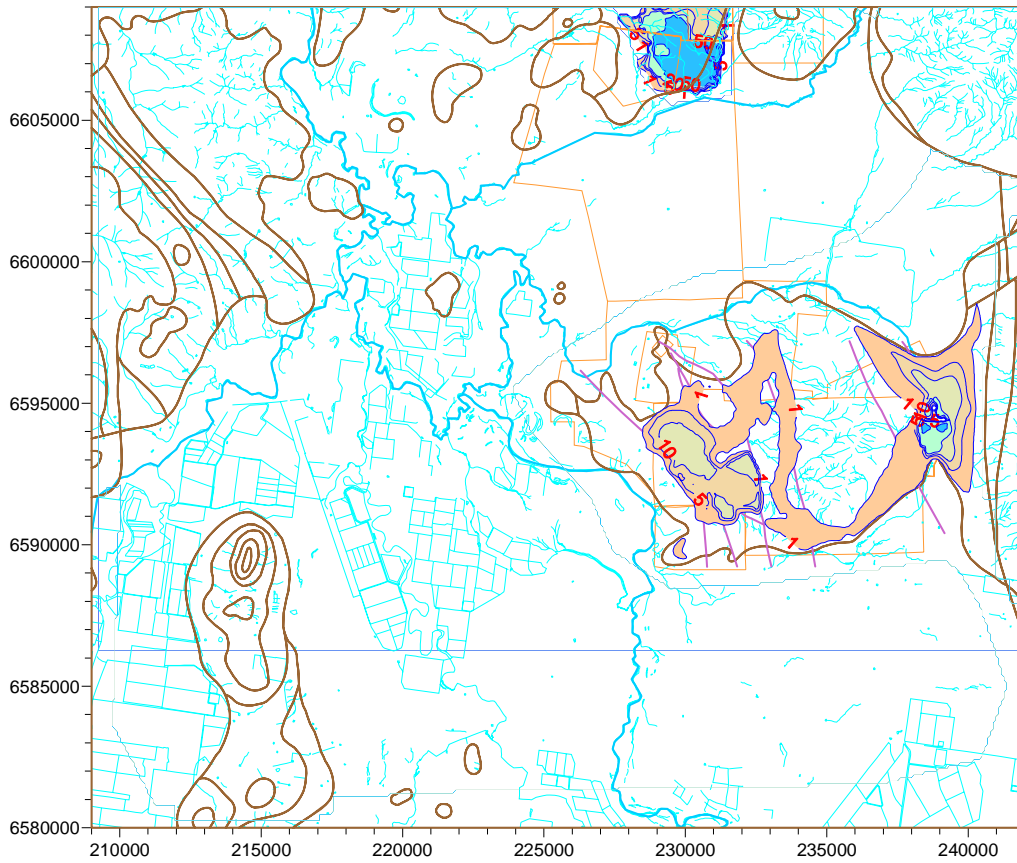
Predicted Drawdown Contours at the of End of Stress Period 31 (End of Year 2042) in Layers [a] 4 and [b] 6



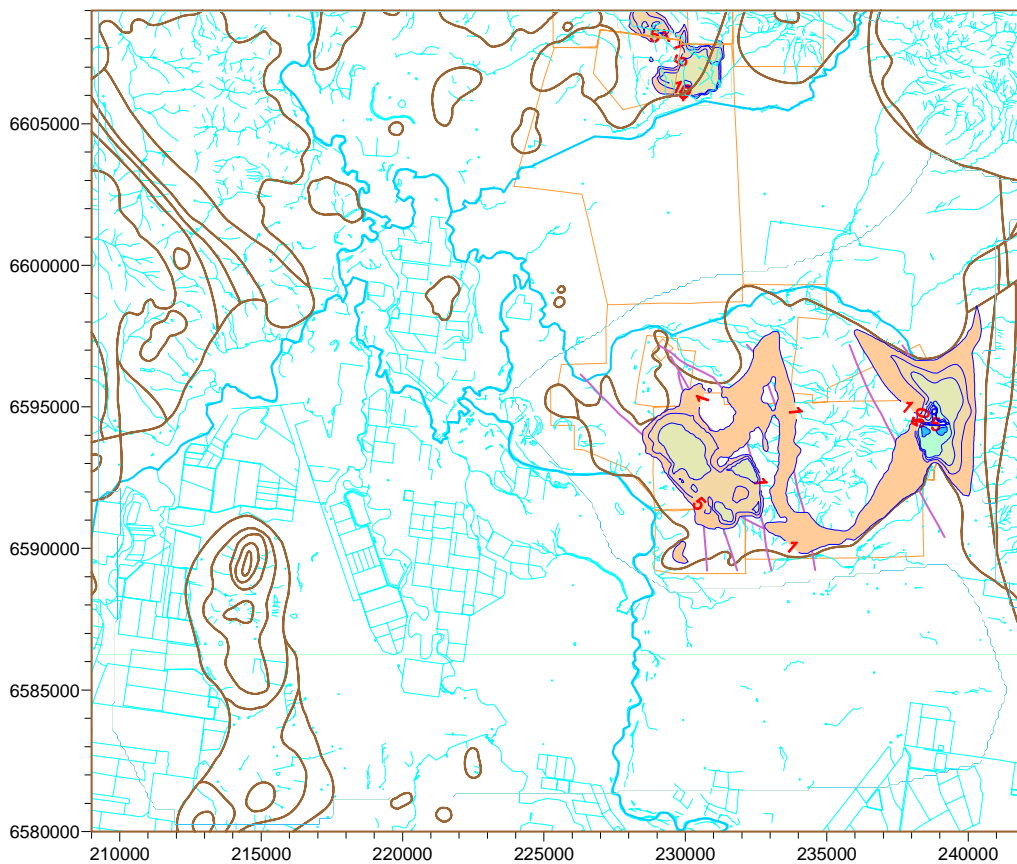
Predicted Drawdown Contours at the of End of Stress Period 31 (End of Year 2042) in Layers [a] 8 and [b] 10



Predicted Drawdown Contours at the of End of Stress Period 31 (End of Year 2042) in Layer 12



Cumulative Drawdown Contour Map for Layer 1 at the of End of Stress Period 19 (End of Year 2030)



Cumulative Drawdown Contour Map for Layer 2 at the of End of Stress Period 19 (End of Year 2030)